APPENDIX G

TRAFFIC IMPACT ANALYSIS



SUHSD #5 HIGH SCHOOL SALINAS, CALIFORNIA

TRAFFIC IMPACT ANALYSIS



Prepared For

EMC Planning Group Monterey, California

July 21, 2011



"This document has been prepared for the titled project or named part thereof and should not be relied upon or used for any other project without an independent check being carried out as to its suitability and prior written authority of Hatch Mott MacDonald being obtained. Hatch Mott MacDonald accepts no responsibility or liability for the consequences of this document being used for a purpose other than the purposes for which it was commissioned. Any person using or relying on the document for such other purposes agrees, and will by such use or reliance be taken to confirm his agreement, to indemnify Hatch Mott MacDonald for all loss or damage resulting therefrom. Hatch Mott MacDonald accepts no responsibility or liability for this document to any party other than the person by whom it was commissioned.

To the extent that this report is based on information supplied by other parties, Hatch Mott MacDonald accepts no liability for any loss or damage suffered by the client, whether contractual or tortious, stemming from any conclusions based on data supplied by parties other than Hatch Mott MacDonald and use by Hatch Mott MacDonald in preparing this report."



TABLE OF CONTENTS

1	INTR	ODUCTION	1
	1.1	Project Description	1
	1.2	Project Access	1
	1.3	Scope of Work	2
	1.4	Traffic Operation Evaluation Methodologies	3
	1.4.1	Intersection Operational Analysis	
	1.4.2	Peak Hour Factors	4
	1.4.3	Heavy Vehicle Factors	
	1.5	Modeling of Right Turns	
	1.6	Level of Service Standards and Significance Criteria	5
	1.7	Road Network Assumptions	7
2	EXIS	TING TRAFFIC CONDITIONS	9
	2.1	Existing Traffic Network	9
	2.2	Existing Transit Systems	
	2.3	Existing Bikeway and Pedestrian Facilities	. 10
	2.4	Existing Traffic Data	11
	2.5	Existing Conditions Intersection Operations	. 12
3	EXIS	TING PLUS PROJECT PHASE 1 TRAFFIC CONDITIONS	13
	3.1	Project Description and Phasing	13
	3.2	Analysis of Project Phases	
	3.3	Project Trip Generation	. 14
	3.4	Project Trip Distribution and Assignment	15
	3.5	Project Level Transit Systems	. 16
	3.6	Project Level Bikeway and Pedestrian Facilities	. 16
	3.7	Existing Plus Project Phase 1 Traffic Conditions - Intersection Operations	. 18
	3.8	Neighborhood Impacts	
	3.9	Project Access and Internal Circulation	
	3.10	Project Mitigation Measures	. 24
4	CUM	ULATIVE WITH PROJECT BUILDOUT TRAFFIC CONDITIONS	. 26
	4.1	Cumulative Traffic Conditions – Road Network	26
	4.2	Cumulative Traffic Volumes	27
	4.3	Project Description – Buildout	
	4.4	Project Trip Generation	
	4.5	Project Trip Distribution and Assignment	
	4.6	Cumulative With Project Buildout Traffic Conditions - Intersection Operations	
	4.7	Traffic Impact Fees	32



TABLE OF CONTENTS (continued)

5	ALTE	RNATIVE SITE PLANS	34
6	RECO	MMENDATIONS	36
		Recommended Improvements Under Existing Conditions	
	6.3	Recommended Improvements Under Cumulative With Project Buildout Conditions	37
		Conditions	



LIST OF EXHIBITS

1.	Project Vicinity Map
2A.	Project Site Plan
2B.	Proposed Rogge Road Improvement Plan
3.	Existing MST Routes
4A. 4B. 4B.	Existing Conditions – AM Peak Hour Volumes Existing Conditions – Midday Peak Hour Volumes Existing Conditions – PM Peak Hour Volumes
5A .	Intersection Levels of Service
5B.	Recommended Intersection Improvements
6A.	Trip Generation Rate Comparison (Area High Schools vs. ITE)
6B.	Project Trip Generation
7A.	Project Trip Distribution – Phase 1
7B.	Santa Rita Union School District Boundary
8A.	Project Phase 1 Trip Assignment – AM Peak Hour Volumes
8B.	Project Phase 1 Trip Assignment – Midday Peak Hour Volumes
8C.	Project Phase 1 Trip Assignment – PM Peak Hour Volumes
9A.	Existing Plus Project Phase 1 Conditions – AM Peak Hour Volumes
9B.	Existing Plus Project Phase 1 Conditions – Midday Peak Hour Volumes
9C.	Existing Plus Project Phase 1 Conditions – PM Peak Hour Volumes
10.	City of Salinas General Plan Road Network
11.	Project Trip Distribution – Future Growth Area
12A.	Project Buildout Trip Assignment – AM Peak Hour Volumes
12B.	Project Buildout Trip Assignment – Midday Peak Hour Volumes
12C.	Project Buildout Trip Assignment – PM Peak Hour Volumes
13A.	Cumulative With Project Buildout Conditions – AM Peak Hour Volumes
13B.	Cumulative With Project Buildout Conditions – Midday Peak Hour Volumes
13C.	Cumulative With Project Buildout Conditions – PM Peak Hour Volumes



LIST OF EXHIBITS (continued)

- 14A. Project Site Plan Alternative #1
 14B. Project Site Plan Alternative #2
- 15A. High School #5 Rogge Road Driveway Volumes Project Alternative #1
 15B. High School #5 Rogge Road Driveway Volumes Project Alternative #2
- 16. Intersection Levels of Service, Rogge Road/Project Driveway Intersections, Site Plan Alternatives 1 and 2



LIST OF APPENDICES

- A1. Level of Service Description Signalized Intersections
- A2. Level of Service Description Unsignalized Intersections All-Way Stop Control
- A3. Level of Service Description Unsignalized Intersections Two-Way Stop Control
- B. Traffic Count Dates and Sources Summary Table
- C. Intersection Level of Service Calculations Natividad Road/Rogge Road
- D. Intersection Level of Service Calculations Natividad Road/Boronda Road
- E. Intersection Level of Service Calculations San Juan Grade Road/Boronda Road
- F. Intersection Level of Service Calculations San Juan Grade Road/Russell Road
- G. Intersection Level of Service Calculations San Juan Grade Road/Penzance Street
- I. Intersection Level of Service Calculations San Juan Grade Road/Rogge Road
- J. Intersection Level of Service Calculations Rogge Road/Kelton
- K. Intersection Level of Service Calculations Rogge Road/Bollenbacher
- L. Intersection Level of Service Calculations Rogge Road/Jasper Way
- M. Intersection Level of Service Calculations Rogge Road/Jade Drive
- N. Intersection Level of Service Calculations Rogge Road/Driveway 1
- O. Intersection Level of Service Calculations Rogge Road/Driveway 2
- P. Intersection Level of Service Calculations Rogge Road/Driveway 3
- Q. Warrant Worksheets
- R. Peak Hour Factors at Study Intersections
- S. Percentage of Heavy Vehicles
- T. Traffic Operations and Safety at Schools: Recommended Guidelines
- U. Intersection Level of Service Calculations Rogge Road/El Dorado Drive



1 INTRODUCTION

The proposed Salinas Union High School District (SUHSD) #5 High School project is located on Rogge Road in the City of Salinas. The site is currently being used for agricultural use. Exhibit 1 shows the location of the study project site, while Exhibit 2 shows the proposed site plan.

The purpose of this Traffic Impact Analysis (TIA) was to determine both the potential direct and cumulative traffic impacts from the buildout of the proposed project. The TIA presents the results from a series of analyses performed to determine the existing traffic conditions, the potential traffic impacts from the study project at project buildout levels, and how traffic conditions would change with the implementation of long-range development projects in the area.

1.1 Project Description

The project would construct a new high school, including various classroom, athletic, and performing arts facilities. The new high school would have an enrollment of up to 1,500 students.

1.2 Project Access

The project site is located on the south side of Rogge Road, just east of the Bolsa Knolls residential area and will be accessed via five driveways. From west to east on Exhibit 2A, the driveways serve the following purposes:

- o Driveway 1 provides access to staff and visitor parking as well as the auto drop-off/pick-up area.
- o Driveway 2 is an inbound only driveway that provides access to student parking and the bus drop-off/pick-up area.
- o Driveway 3 is an outbound only driveway that provides egress from the student parking lot.
- o Driveway 4 is an outbound only driveway that provides egress from the bus drop-off/pick-up area. The driveway approach to Rogge Road will be designed and signed to only allow right turns from the driveway to Rogge Road.
- o Driveway 5 provides access to the special events parking lot that is located on the east side of the campus. This lot will only be used for special school events.

For the near-term, Rogge Road will be accessible from San Juan Grade Road and Natividad Road. Ultimately, the City of Salinas plans for Russell Road to be extended easterly from its current terminus at San Juan Grade Road and will be adjacent the school's southern property boundary. See Exhibit 10, City of Salinas Land Use and Circulation Policy Map. The long-range road network also includes the extension of El Dorado Drive from its current terminus at Boronda Road to Rogge Road.



1.3 Scope of Work

The scope of work for this traffic study was developed to identify the potential traffic impacts that may be associated with the study project. Intersections were selected for analysis based on the potential for the project to impact the facility.

The local streets and intersections included in this analysis were identified as potentially having the greatest impact from the project based on preliminary analysis of project trip generation and trip distribution. The boundaries of the study have been selected to include intersections that presently experience some congestion and/or may be measurably affected during the peak commute hours.

Beyond the limits of the study area, the project trips disperse onto numerous local streets. As the distance from the project increases the number of trips considered reduces and the distribution assumptions are less reliable.

This traffic study includes a traffic impact analysis of operations at 10 existing intersections during typical weekday AM and PM peak hours, as well as mid-afternoon analysis representing operations when the high school classes end for the day. The following existing intersections were analyzed in this study:

- Natividad Road/Rogge Road
- 2. Natividad Road/Boronda Road
- 3. San Juan Grade Road/Boronda Road
- 4. San Juan Grade Road/Russell Road
- 5. San Juan Grade Road/Penzance Road
- 6. San Juan Grade Road/Rogge Road
- 7. Kelton Drive/Rogge Road
- 8. Bollenbacher Drive/Rogge Road
- 9. Jasper Way/Rogge Road
- 10. Jade Drive/Rogge Road

In addition, traffic operations at four of the five high school driveway intersections were analyzed. Operations at the driveway serving the special events parking lot are not analyzed because access to this lot will be prohibited during the AM, midday and PM peak hours. The future intersection of Rogge Road and El Dorado Drive was also analyzed.

Where required, mitigation measures are recommended to fully mitigate the impacts due to the development of the study project. Traffic control warrant and channelization warrant assessments were performed at the study intersections for all traffic scenarios evaluated.

This traffic study analyzed the traffic impacts of buildout of the proposed project, along with the additional effects of traffic from the long-range cumulative projects in the area. The traffic scenarios evaluated as part of this traffic study are:



- Existing Traffic Conditions;
- Existing Plus Project Phase 1;
- Cumulative Plus Project Buildout Traffic Conditions.

Traffic conditions during the AM, mid-afternoon and PM peak hours were analyzed. Existing traffic conditions were determined based on new AM, Mid-afternoon and PM peak period traffic counts collected at the study intersections.

The trips generated by the new high school were determined on the basis of a trip generation study of five high schools in the region. The project generated trips were assigned to the local road network using a trip distribution pattern derived using the expected attendance boundary for the high school. The process provides an intersection level analysis, which is required for the environmental evaluation of project impacts.

1.4 Traffic Operation Evaluation Methodologies

The following paragraphs describe the methodologies utilized in this analysis to evaluate the operations of all of the study intersections and roadway segments. All analysis is based upon the 2000 *Highway Capacity Manual* methodologies.

1.4.1 Intersection Operational Analysis

Intersection traffic operations were evaluated based on the Level of Service (LOS) concept. LOS is a qualitative description of an intersection and roadway's operation, ranging from LOS A to LOS F. Level of service "A" represents free flow un-congested traffic conditions. Level of service "F" represents highly congested traffic conditions with what is commonly considered unacceptable delay to vehicles on the road segments and at intersections. The intermediate levels of service represent incremental levels of congestion and delay between these two extremes.

Intersection operations were evaluated using technical procedures documented in the 2000 Highway Capacity Manual (HCM). For signalized intersections, average control delay per vehicle is utilized to define intersection level of service. Delay is dependent on a number of factors including the signal cycle length, the roadway capacity (number of travel lanes) provided on each intersection approach and the traffic demand. Appendix A1 shows the relationship between vehicle delay and the signalized intersection level of service categories. The TRAFFIX software program (version 8.0) was utilized to model the traffic impact of the different development scenarios and to calculate signalized and un-signalized intersection levels of service.

For all-way (or four-way) stop intersections, average control delay per vehicle is utilized to define intersection level of service. Delay is dependent on a number of factors including the roadway capacity (number of travel lanes) provided on each intersection approach and the traffic demand. Appendix A2 shows the relationship between vehicle delay and the all-way stop intersection level of service categories.



At one- and two-way stop controlled intersections, the operating efficiency of vehicle movements that must yield to through movements are analyzed. The level of service for vehicle movement on the controlled approaches is based on the distribution of gaps in the major street traffic stream and driver judgment in selecting gaps. Appendix A3 shows the relationship between the vehicle delay and level of service for two-way stop controlled intersections. The 2000 HCM calculates the level of service of the minor street approaches. Using this data, an overall intersection level of service was calculated. Both are reported in this study because traffic on the minor street approaches has the lowest priority of right-of-way at the intersection and are the most critical in terms of delay. Generally, LOS E/F operations on the side street approach are the thresholds that warrant improvements.

Peak hour signal warrants were analyzed for the unsignalized intersections, as taken from the *California Manual on Uniform Traffic Control Devices For Streets and Highways*, (Section 4C.04, Warrant 3, Peak Hour), California Department of Transportation, January 21, 2010. In this study, the only signal warrant evaluated was the peak hour signal warrant. The decision to install a traffic signal should not be based purely on the warrants alone. Engineering judgment should be exercised on a case-by-case basis to evaluate the effect a traffic signal would have on certain types of accidents and traffic conditions at the subject intersection, as well as at adjacent intersections. Warrant worksheets can be found in Appendix Q.

1.4.2 Peak Hour Factors

Per the 2000 Highway Capacity Manual, peak hour factors are included within this analysis. A peak hour factor is utilized in a level of service analysis to simulate operations during the peak fifteen minutes of a peak hour period, thereby accounting for variations in traffic demand within that hour.

Appendix R summarizes the peak hour factors utilized within this analysis. Under Existing conditions, the analysis uses the peak hour factors calculated from the existing traffic counts. For the remaining analysis scenarios, the peak hour factor has been recalculated using a weighted average of the peak hour factors of the various components of the traffic under that scenario. For example, the primary components of the study project are a high school. Land uses such as schools experience a pronounced peaking of traffic demand just before and just after school starts, usually within a 15- to 20-minute period. Immediately before and after this period, traffic demand at the school is considerably smaller than within the period. The weighting of the peak hour factors for each intersection accounts for this short and pronounced peaking at the schools. Appendix R also contains the weighted average peak hour factor calculations at each intersection.

These peak hour factors are used in multiple ways within this analysis. The use of these peak hour factors results in the analysis representing traffic operations during the "peak of the peak," i.e. the 15 minutes prior to the start of school and the 15 minutes after school ends.



1.4.3 Heavy Vehicle Factors

At the time the intersection turning movement volume data was collected, the trucks and buses entering each study intersection was recorded. This data was used to calculate the percentage of heavy vehicles entering the intersection on each approach of each study intersection. This data was coded in the Traffix software so that the intersection level of service calculation accounts for the impact of trucks and buses. Appendix S shows the percentage of heavy vehicles for each study intersection.

1.5 Modeling of Right Turns

All of the signalized study intersections allow right turns on red (RTOR), and these right turns can have an effect on the intersection LOS calculations. However, for this study no allowance was made for RTOR, as insufficient information was available regarding the percentage of vehicles turning right on red. The results of the intersection analyses can thus be seen as reflecting a "worst case" scenario, as the effect of vehicles turning right on red on the intersection operations were not accounted for.

Right turn overlap signal phasing, whereby right turns are allowed to move unimpeded during a complementary left turn movement, is not currently in operation at any of the study intersections, but may be added as proposed improvements for at least one study intersection

Free right turns, whereby right turn movements are free flowing and not controlled by the adjacent traffic signal or stop sign, are also not present at any of the study intersections, but may be added as proposed improvements for at least one study intersection. Since these movements are unimpeded by either conflicting traffic or traffic control devices, there are no delays associated with these movements.

1.6 Level of Service Standards and Significance Criteria

The study area selected covers the jurisdiction of two public agencies, the City of Salinas and the County of Monterey. The City of Salinas and the County of Monterey have established LOS D as the general threshold for acceptable overall traffic operations for signalized, all-way stop controlled, and one- and two-way stop controlled intersections. For this study, LOS F operations on the side street approach are the thresholds that warrant improvements at one- and two-way stop controlled intersections.

The following are CEQA guidelines for the evaluation of the significance of transportation and traffic related impacts:

- 1. Would the project conflict with an applicable plan, ordinance or policy establishing measures of effectiveness for the performance of the circulation system, taking into account all modes of transportation including mass transit and non-motorized travel and relevant components of the circulation system, including but not limited to intersections, streets, highways and freeways, pedestrian and bicycle paths, and mass transit?
- 2. Conflict with an applicable congestion management program, including, but not limited to level of service standards and travel demand measures, or other standards established by the county congestion management agency for designated roads or



highways?

- 3. Result in a change in air traffic patterns, including either an increase in traffic levels or a change in location that results in substantial safety risks?
- 4. Substantially increase hazards due to a design feature (e.g., sharp curves or dangerous intersections) or incompatible uses (e.g., farm equipment)/
- 5. Result in inadequate emergency access?
- 6. Conflict with adopted policies, plans, or programs regarding public transit, bicycle, or pedestrian facilities, or otherwise decrease the performance or safety of such facilities?

The significance criteria utilized in the study is consistent with the adopted policies, regulations, goals and guidelines for the City of Salinas and County of Monterey as applicable to the facilities under their jurisdiction. Where adopted policies do not exist, significance criteria are used that is consistent with criteria applied in recently completed environmental studies. The impact criteria for this study are presented below.

Signalized Intersections

Significant impacts at signalized intersections are defined to occur when:

- The addition of project traffic causes intersection operations to degrade from an acceptable level (LOS D or better) to an unacceptable level (LOS E or worse), or
- Project traffic is added to an intersection operating at an unacceptable level (LOS E or worse).

<u>Unsignalized Intersections</u>

Significant impacts at unsignalized intersections are defined to occur when:

- The addition of project traffic to any unsignalized intersection operating at LOS F under existing conditions; or
- Any traffic signal warrant is met.

Pedestrian and Bicycle Facilities

Significant impacts to pedestrian and bicycle facilities are defined to occur when:

- The project conflicts with existing or planned pedestrian or bicycle facilities, or
- The project creates pedestrian and bicycle demand without providing adequate facilities.



Transit Facilities

Significant impacts to transit facilities are defined to occur when:

- The project conflicts with existing or planned transit facilities, or
- The project generates potential transit trips without providing adequate facilities for pedestrians and bicycles to access transit routes and stops.

1.7 Road Network Assumptions

A description of the road network assumptions incorporated into this traffic analysis under the Existing and Cumulative traffic scenarios is provided below.

Existing Conditions (Without and With Project)

The analysis of existing conditions reflects existing street network conditions. The Existing Plus Phase 1 Project analysis also reflects existing street network conditions with the addition of the school and the five driveways that serve the school.

Cumulative Conditions

The cumulative condition traffic forecasts are based on 2030 travel forecasts prepared for the supplemental transportation analysis for the proposed Future Growth Area. The traffic forecasts documented in that study used a horizon year of 2030 and include the full buildout of the Salinas Future Growth Area including the FGA road network. Road improvements assumed for the cumulative condition analysis include the following:

- 1. San Juan Road widening to 4-lanes between Boronda Road and Rogge Road.
- 2. New US 101/Harrison Road diamond interchange with local roadway improvements.
- 3. Extension of Russell Road as a 4-lane arterial between San Juan Grade Road and Old Stage Road.
- 4. Extension of McKinnon Street as a 2-lane collector between Boronda Road and Russell Road.
- 5. Extension of El Dorado Drive as a 2-lane collector between Boronda Road and Russell Road.
- 6. Extension of Independence Boulevard as a 2-lane arterial between Boronda Road and Old Stage Road.
- 7. Extension of Constitution Boulevard as a 2-lane arterial between Boronda Road and Old Stage Road.
- 8. Extension of Sanborn Road as a 2-lane arterial between Boronda Road and Old Stage Road.
- 9. Boronda Road widening to 6-lane arterial between San Juan Grade Road and Williams Road.
- 10. Natividad Road widening to a 4-lane arterial between Boronda Road and Rogge Road.



11. Addition of two east-west 2-lane collectors between San Juan Grade Road and Williams Road.

A more complete description of the cumulative condition road network is provided in Section 4.



2 EXISTING TRAFFIC CONDITIONS

This chapter presents a description of the existing road network, existing traffic volumes, intersection levels of service, and an overview of traffic flow conditions within the study area under existing traffic conditions.

2.1 Existing Traffic Network

The project site is located on the south side of Rogge Road, east of the Bolsa Knolls community. The existing road network in the vicinity of the project site is described below.

Rogge Road is a two lane collector street that connects San Juan Grade Road and Natividad Road. West of the project site, the posted speed limit is 35 miles per hour and east of the project site, the speed limit is not posted. Bolsa Knolls Middle School is located on the north side of Rogge Road, east of Bollenbacher Drive. La Joya Elementary School is located on the south side of Rogge Road, west of Bollenbacher Drive. Rogge Road is currently under County jurisdiction.

Boronda Road is a primary access route to the study area and provides access between Highway 101 on the west and Williams Road on the east. It is a six lane east-west arterial between Highway 101 and just east of Main Street. It narrows to a two lane arterial east of San Juan Grade Road. It is planned to be a six lane divided arterial in the Salinas General Plan.

El Dorado Drive is a two lane collector that extends in a north-south orientation between Alvin Drive on the south and Boronda Road on the north. It will be extended as a two lane collector street to Rogge Road.

McKinnon Street is a two lane collector that extends in a north-south orientation between Alvin Drive on the south and Boronda Road on the north. It will be extended as a two lane collector street to Russell Road near the southwest corner of the high school site.

Natividad Road is a six lane divided arterial between Laurel Drive and Boronda Road. It narrows to a two lane rural highway between Boronda Road and Old Stage Road. It is planned to be widened to a four lane divided arterial between Boronda Road and Rogge Road.

Russell Road is currently a two lane arterial between Highway 101 and San Juan Grade Road. Major portions of the Russell Road are wide enough to be a four lane arterial, although the road is currently striped as a two lane arterial due to certain sections that have not been widened to its ultimate width. It is proposed to be extended east of San Juan Grade Road across Natividad Road, along the southern boundary of the project site, to Old Stage Road. It will be a four lane arterial from Highway 101 to Old Stage Road.



San Juan Grade Road is a four lane arterial between Main Street and Boronda Road. North of Russell Road, San Juan Grade is two-lanes wide. It extends north of Russell Road to Crazy Horse Canyon Road and continues north to Highway 156 near San Juan Bautista as a two-lane road. It is planned to be a four lane divided arterial from Rogge Road to Main Street.

San Juan Grade Road north of Russell Road, and Rogge Road including the Rogge Road/Natividad Road intersection and the streets within the Bolsa Knolls community, are under the jurisdiction of the County of Monterey. San Juan Grade Road south of Russell Road including the San Juan Grade Road/Russell Road intersection, Natividad Road south of the Natividad Road/Rogge Road intersection and Boronda Road are under the jurisdiction of the City of Salinas.

2.2 Existing Transit Systems

The largest single public transit provider in Monterey County is the Monterey-Salinas Transit (MST). The Monterey-Salinas Transit operates from five key transit centers, the Monterey Transit Plaza, Salinas Transit Center, Watsonville Transit Center, Edgewater Transit Exchange in Seaside/Sand City, and Marina Transit Exchange. Each of these centers operates on a time-transfer "pulse" schedule providing easy connections and quick transfers to multiple routings.

The project site is not currently served by MST. The closest service to the site is Route 45 that serves Russell Road, Van Buren Avenue, San Juan Grade Road south of Van Buren Avenue and Boronda Road. Exhibit 3 shows the existing MST transit routes in the vicinity of the project site.

2.3 Existing Bikeway and Pedestrian Facilities

Bikeways

There are three basic types of bicycle facilities. Each type is described below:

- Bike path (Class I) A completely separate right-of-way designed for the exclusive use of cyclists and pedestrians, with minimal crossings for motorists. These paths should have a minimum width of 8 feet when two-way travel is required and 5 feet in width to accommodate one-way movement.
- Bike lane (Class II) A lane on a regular roadway, separated from the motorized vehicle right-of-way by paint striping, designated for the exclusive or semi-exclusive use of bicycles. Bike lanes allow one-way bike travel. A minimum width of 5 feet should be provided and adjacent curbside parking avoided where feasible; where curbside parking is allowed adjoining a bike lane, the combined width of the parking and adjacent bike lane should be not less than 13 feet.
- Bike route (Class III) Provides shared use of the roadway, designated by signs or permanent markings and shared with motorists.



Existing Bike facilities in project vicinity

Except for a short section of Rogge Road between San Juan Grade Road and La Joya Elementary School, bike lanes are not currently provided on Rogge Road between San Juan Grade Road and Natividad Road. San Juan Grade Road and Natividad Road in the vicinity of the project also do not have bike lanes. The Monterey County Bikeways Plan includes plans for Class II bike lanes for San Juan Grade Road (between Van Buren Avenue and Crazy Horse Canyon Road, Rogge Road (between Natividad Road and San Juan Grade Road) and Natividad Road (between Boronda Road and Old Stage Road.

Pedestrian facilities

A sidewalk is currently provided on the south side of Rogge Road between San Juan Grade Road and the westerly property line. A sidewalk is also provided on the north side of Rogge Road between San Juan Grade Road and the high school site, except along two parcels located between Jasper Way and Jade Drive. Along the project frontage as well as east of the project to Natividad Road, sidewalks are not provided as this portion of Rogge Road fronts agricultural land. The residential streets located within the Bolsa Knolls community are improved with curb, gutter and sidewalks.

At the San Juan Grade Road/Rogge Road intersection, a marked crosswalk is provided across the east intersection leg (Rogge Road) and across the south intersection leg (San Juan Grade Road). A marked crosswalk is also provided on Rogge Road on the east leg of the Rogge Road/Rogge Village Drive intersection. This crosswalk is located about 200 feet east of the Rogge Road/Kelton Street intersection. No other marked crosswalks are provided on Rogge Road. California Vehicle Code, Section 275 defines a crosswalk as the extension of sidewalks through intersections, or the portion of a roadway distinctly indicated by lines or other markings on the surface. Therefore, there are unmarked crosswalks at every street intersecting Rogge Road that require approaching motorists to yield to pedestrians as if they are in a marked crosswalk.

2.4 Existing Traffic Data

To establish existing traffic flow conditions, new traffic counts were conducted at most of the study intersections during the weekday AM (i.e. 7:00-9:00 am), midday (i.e. 2:00-4:00 PM) and PM (i.e. 4:00-6:00 pm) peak hours. The intersection volumes were collected by Hatch Mott MacDonald in March, April and May 2011. A table summarizing the collection dates of the existing traffic volumes is presented in Appendix B.

From the peak period traffic counts, the AM, midday, and PM peak hour turning movement volumes were identified. Each intersection was analyzed at its individual peak hour. Because all of the counts were not collected on the same day, the counts did not necessarily balance between intersections. The intersection traffic volumes were therefore balanced between adjacent intersections along the arterial corridors, to account for variations in the counts. Along each corridor, the intersection with the highest approach volume was selected as the controlling volume and volumes at the other



intersections along the corridor were balanced between intersections to the controlling volume, regardless of when the count was collected. This provides a reasonable worst-case analysis as the highest volume of traffic observed over the count period was used for the study. The existing peak hour traffic volumes are presented on Exhibits 4A, 4B, and 4C.

The following discussion provides an evaluation of operating conditions for the study intersections under existing traffic conditions.

2.5 Existing Conditions Intersection Operations

Existing conditions AM, midday, and PM intersection levels of service are summarized on Exhibit 5A. The LOS calculation sheets for existing traffic conditions are contained in Appendix C through Appendix N. The traffic control warrant and channelization warrant worksheets are included in Appendix M. Recommended intersection improvements are indicated on Exhibit 5B.

Three of the existing study intersections are currently signalized, one is all-way stop controlled and six are one-way or two-way stop-controlled, one is effectively an "L" intersection (with no conflicting movements). Please reference Exhibit 5A for a list of the study intersections and detail of the traffic control at each intersection.

All of the ten study intersections analyzed under existing conditions operate at or better than the LOS D standard. No improvements are required at any of these study intersections under existing conditions based on the intersection operations analysis.

The left turn lane on northbound Natividad Road at Rogge Road is designed with a storage bay of about 100 feet and a bay taper of 120 feet. The design of the left turn lane is not adequate based on design guidelines published by Caltrans. Left turn lanes provide storage for vehicles turning left and also vehicle deceleration for vehicles entering the left turn lane. It is desirable that deceleration takes place entirely off the through lanes. Caltrans allows 10 to 20 miles per hour of deceleration to occur in the left turn lane under certain conditions. Based on a 60 mph design speed, but allowing for 20 mph of deceleration to occur in the left turn lane, the left turn lane would need to be 315 feet in length just for vehicle deceleration. Based on Caltrans standards, 325 feet of additional space should be provided for vehicle storage. This will serve the peak demand during the morning peak hour. The under-design of the left turn lane is a safety issue as inadequate storage and deceleration lengths are currently provided. Vehicles slowing in the northbound Natividad Road through lane before entering the left turn lane present a hazard to through traffic on Natividad Road. Vehicle spillback from the left turn lane into the northbound through lane also is a safety hazard for motorists in the left turn lane and motorists travelling on northbound Natividad Road.



3 EXISTING PLUS PROJECT PHASE 1 TRAFFIC CONDITIONS

This section describes the analysis results of the study intersection and roadway segment operations under Existing Plus Project Phase 1 traffic conditions. This traffic scenario includes only Phase 1 of the proposed project.

3.1 Project Description and Phasing

The proposed Salinas #5 High School project is located in the City of Salinas. The project site is located on the south side of Rogge Road, immediately east of Bolsa Knolls community. The site is currently being used for agricultural uses.

The project has been split into two separate phases. Phase 1 would develop a high school with an enrollment of 900 students. The campus would be built-out to include classrooms, support facilities and athletic fields. The school is being built to support 900 students in existing neighborhoods served by the Santa Rita Union School District (elementary schools). The SUHSD expects to be able to accommodate 600 students from the Future Growth Area. Because it is not known if and when development within the Future Growth Area will proceed, the traffic impacts of full enrollment was analyzed in the Cumulative scenario.

The project will be accessed from Rogge Road via five driveways. For reference, the driveways are 1 through 5 from west to east as shown on Exhibit 2A. From west to east on Exhibit 2A, the driveways serve the following purposes:

- o Driveway 1 provides access to staff and visitor parking as well as the auto drop-off/pick-up area.
- o Driveway 2 is an inbound only driveway that provides access to student parking and the bus drop-off/pick-up area.
- o Driveway 3 is an outbound only driveway that provides egress from the student parking lot.
- o Driveway 4 is an outbound only driveway that provides egress from the bus drop-off/pick-up area. The driveway approach to Rogge Road will designed and signed to only allow right turns from the driveway to Rogge Road.
- o Driveway 5 provides access to the special events parking lot that is located on the east side of the campus. This lot will only be used for special school events.

3.2 Analysis of Project Phases

This chapter (Chapter 3) focuses on traffic operations under Phase 1 of the project, while Chapter 5 describes the analysis of Phase 2. Phase 1 is analyzed in conjunction with existing traffic volumes and the existing street network, while Buildout is analyzed with the trips from both cumulative long-range development projects in the greater Salinas area that includes development of the Future Growth Area land uses and proposed street network changes.



3.3 Project Trip Generation

Exhibit 6B contains the trip generation estimate for Phase 1 of the study project. The assumptions utilized in deriving the trip generation estimates for each component are described below.

The standard method for estimating trip generation for a high school is via the use of trip rates published in the Institute of Transportation Engineers' (ITE) *Trip Generation*, 8th Edition, 2008. However, these trip rates were derived from surveys conducted across the United States over a four-decade period (from the late 1960s to the 2000s). In more recent times, traffic activity at schools has increased dramatically, as fewer students walk or bike to school. The United States Federal Highway Administration has documented this trend in the following two studies:

- 1. Transportation Characteristics of School Children, Report No. 4, Nationwide Personal Transportation Study, Federal Highway Administration, Washington, DC, July 1972; and
- 2. 2001 National Household Travel Survey, conducted by Federal Highway Administration.

According to these studies, in 1969, nearly half of all students walked or biked to school. By the 2001, fewer than 15% of students walked or biked. In recognition of this trend, the use of the ITE trip rates was rejected in favor of the derivation of more localized trip generation rates, derived from surveys of high schools in the greater Monterey Bay Area and southern Santa Clara County.

Traffic counts were conducted by Hatch Mott MacDonald (formerly Higgins Associates) staff in March 2008 during a seven-day period at three area high schools – Seaside High School in Seaside, Pajaro Valley High School in Watsonville, and Everett Alvarez High School in Salinas. Entering and exiting volumes at the driveways to each of these high schools were collected, and compiled on a daily and peak hour basis. The traffic counts between successive school days were averaged together to create average daily and peak hour volumes for each of the three schools. This data was supplemented with previous traffic counts performed at two other area high schools – Gilroy High School in Gilroy, and Aptos High School in Aptos. The Gilroy High School data was taken from the report Christopher High School Traffic Analysis, Fehr & Peers Transportation Consultants, May 30, 2006, while the Aptos High School data was taken from the report Aptos High School Access and Circulation Evaluation, Higgins Associates, May 7, 2004. Appendix L contains the traffic volumes collected at the three high schools in March 2008. The March 2008 counts at Everett Alvarez High School were supplemented with additional counts at the school driveways by Hatch Mott MacDonald (formerly Higgins Associates) in January 2008.

Exhibit 6A contains a summary of the traffic activity at each of the five area high schools, along with their student populations at the time of the data collection. From this data, trip generation rates were derived on a daily basis, as well as for the AM peak hour,



PM school peak hour (i.e. midday peak hour, when school lets out for the day), and PM street peak hour (i.e. PM peak hour, the evening rush hour). These rates were derived by performing a weighted average of the trip activity at the five schools, weighted based upon the student population at each school. These rates were then utilized in the trip generation estimate for the high school component of Phase 1 of the project.

Exhibit 6A also compares the derived trip generation rates to those from ITE for a standard public high school. The combined trip rate from the five area high schools are higher than those published by ITE. The daily trip generation rate derived from the school survey is 8% higher than the ITE rate and the AM, midday and PM peak hour rates from the school survey range from 22% to 33% higher than the ITE published trip rates. Therefore, the utilization of trip rates derived from traffic counts at area schools would constitute a more conservative (high-side) analysis than use of the ITE rates.

Exhibit 6B contains the estimated total project trip generation. Phase 1 of the project would generate a net 1,661 daily trips, with 495 trips during the AM peak hour, 306 trips during the midday peak hour, and 162 trips during the PM street peak hour.

3.4 Project Trip Distribution and Assignment

Trip distribution defines the origins and destinations of all trips to and from a project site. Sixty percent of the enrollment for the new high school will be located within the Santa Rita Union School District and this represents the anticipated enrollment for Phase 1 of the project. Forty percent of the ultimate enrollment will be located within the Future Growth Area and this represents the remainder of the project buildout enrollment. Trip distribution patterns were developed separately for each of the areas to account for the different spatial orientation of each with respect to the new high school.

Exhibit 7 shows the spatial distribution pattern for students located within the Santa Rita Union School District. The trip distribution pattern was developed using the enrollment figures at the elementary schools located within the school district as a surrogate for the spatial distribution pattern of population within the district. The following enrollment figures were used in the analysis for each school:

La Joya Elementary School	459 students
McKinnon Elementary School	540 students
New Republic Elementary School	410 students
Santa Rita Elementary School	515 students

The trip distribution was further refined to determine the percentage arriving and departing to/from the west versus the percentage arriving and departing to/from the east on Rogge Road. The results are as follows:

	To/From the West	To/From the East
Santa Rita Union District Students	80%	20%



The Phase 1 assignment of AM, Midday and PM peak hour high school trips to the study intersections is shown on Exhibits 8A, 8B and 8C, respectively.

No credit was given for the trips generated by existing high school students that would be transferred to the new school. All of the project-generated traffic was assumed to be new traffic on the local road network. As a result, the analysis of traffic impacts is conservative. Further, regionally important benefits that will occur as a result of the project are not documented in this report. For example, school commute related trip lengths for the 900 students that would attend the new school in Phase 1 of the project could be less, on average, with the school developed versus without the school. That is because these students now attend other high schools in Salinas and the new high school will be located a shorter distance from home than the other high schools. The shorter average trip lengths will reduce overall vehicle miles of school commute travel and will reduce air quality and green house gas impacts associated with school commute trips.

The start and end times for the high school was assumed to occur at or near the start and end times of the existing elementary school and middle school located on Rogge Road. Consistent with the starting and ending times of other high schools in the Salinas Union High School District, the new high school was assumed to begin the school day at or near the 8:00 AM hour and end the school day at or near the 3:00 PM hour. La Joya Elementary School begins the day at 8:00 AM and ends at 2:40 PM and the Bolsa Knolls Middle school begins school at 8:15 AM and ends at 3:10 PM.

3.5 Project Level Transit Systems

The Monterey-Salinas Transit (MST) does not provide service to Rogge Road. However, the school district operates a school busing program. Students living outside of $2\frac{1}{2}$ miles of the high school would be offered bus transportation to the school. The exact number of students that would be bused to and from the school is uncertain. For purposes of this analysis, it was assumed the school would be served by six buses during the morning arrival period and afternoon departure period. The proposed project would not create significant impacts to transit facilities.

3.6 Project Level Bikeway and Pedestrian Facilities

The project will generate pedestrian and bicycle trips. Based on the statistics cited in Section 3.3, Trip Generation, approximately 135 students during Phase 1 and 225 students at buildout would bike or walk to school. This is a rough approximation of walking and bicycling demand generated by the school based on national statistics. Actual usage will depend on housing density located with reasonable walking and biking distances, the provision of pedestrian and bicycling facilities and other factors.

The predominant pedestrian and bicycle demand will be initially generated from west of the site. Future pedestrian and bicycle demand from east and south of the site will be generated when the Future Growth Area is developed.



Pedestrians and bicyclists would travel primarily from the Bolsa Knolls neighborhood using Penzance Street, Bollenbacher Drive, Jasper Way and Jade Street to access Rogge Road. Additional walking and bicycle trips to and from the site would be generated by residential housing located on the north side of Rogge Road at San Juan Grade Road and on the west side of San Juan Grade Road at Rogge Road.

The continuous sidewalk located on the south side of Rogge Road between San Juan Grade Road and the project site would serve pedestrians originating from residential development located south of Rogge Road and west of the project site. A continuous sidewalk is not currently provided on the north side of Rogge Road west of the project site to serve pedestrians originating from residential development on the north side of Rogge Road. In addition, walking trips are anticipated between the middle school located on the north side of Rogge Road west of Jasper Way and the high school. The lack of a continuous sidewalk on the north side of Rogge Road west of the project site is a significant impact considering the pedestrian traffic that the project will generate. To mitigate this impact, the project should complete the missing sections of sidewalk on Rogge Road west of the project site.

A Class II bike lane currently exists on Rogge Road between San Juan Grade Road and La Joya Elementary School, but not along the remaining sections on Rogge Road west of the project site. This analysis assumes that some of the high school students will bike to school along Rogge Road. Without Class II bike lanes along the entire portion of Rogge Road west of the project site, bicycling along Rogge Road could be dangerous and result in injury or death to students. This would be a significant safety impact. To mitigate this impact, the project should provide Class II bike lanes along both sides Rogge Road consistent with the Monterey County Bikeways Plan. This would require prohibiting parking on Rogge Road from La Joya Elementary School to the project site. In addition, minor widening of Rogge Road would be required on the north side of Rogge Road at locations that have not been improved with curb gutter and sidewalk. A crosswalk would then need to be provided at the Rogge Road/Jade Street intersection, allowing bicyclists and pedestrians to safely cross the road.

The project will provide a 10 foot wide meandering shared pedestrian/bicycle path on the south side of Rogge Road along the project frontage. The shared facility will be constructed in lieu of Class II bike lanes on Rogge Road. The shared path will continue to the west of Driveway #1 and will terminate at the sidewalk on the south side of Rogge Road that terminates at the westerly property boundary. The existing Rogge Road sidewalk at this location is only 4 feet in width, which is not wide enough to serve as a shared path facility. The planned facilities for pedestrians and bicyclists between the project's western boundary and Jade Drive will not be adequate for the pedestrian and bicycle demand anticipated for this section of roadway. This could result in collisions between pedestrians and bicyclists potentially resulting in injury. This is considered a significant safety impact. To mitigate this impact, the project should widen the existing sidewalk located on the south side of Rogge Road between Jade Drive and the school property boundary to 10 feet. In addition, as identified above, a crosswalk should be provided across Rogge Road at the Jade Drive intersection, allowing bicyclists and



pedestrians to safely cross the road. The crosswalk should be yellow in color and include additional signing and striping as required by the California Manual on Uniform Traffic Control Devices (MUTCD).

A sidewalk will be constructed on the west side of Driveway #1 that will connect to the sidewalk on Rogge Road that terminates at the westerly property boundary. This sidewalk will provide a hard surface connection between the school and the Rogge Road sidewalk that will allow pedestrians to avoid conflicting with drop-off/pick-up traffic.

A bicycle parking area should be provided on the school site.

3.7 Existing Plus Project Phase 1 Traffic Conditions - Intersection Operations

The traffic that would be generated by Phase 1 of the study project was combined with the existing traffic to achieve Existing Plus Project Phase 1 condition traffic volumes. Existing Plus Project Phase 1 morning, midday, and evening peak hour turning volumes are illustrated on Exhibits 9A, 9B and 9C. Exhibit 5A tabulates corresponding morning, midday, and evening peak hour levels of service. The level of service worksheets are presented in Appendices C through N.

The following three intersections would not operate at acceptable operations under the Existing Plus Project Phase 1 Condition:

Intersection #5 – San Juan Grade Road/Penzance Street Intersection #6 – San Juan Grade Road/Rogge Road Intersection #11 – Rogge Road/High School Driveway #1

In addition, the left turn storage lane on northbound Natividad Road at Rogge Road is not adequate to serve the Existing Plus Project Phase 1 travel demand at the intersection.

Traffic operations at each of these intersections under existing plus project conditions are described below.

<u>Intersection #1 – Natividad Road/Rogge Road:</u> This intersection will operate at satisfactory levels of service under Project Phase 1 conditions. However, the project will add trips to the northbound Natividad Road left turn lane. As described in the Existing Conditions section, this left turn lane is not adequately designed for current conditions. The addition of project trips to the intersection represents a significant impact. To mitigate the impact, the left turn lane on northbound Natividad Road at Rogge Road should be lengthened to provide for 400 feet for vehicle storage plus additional length for vehicle deceleration.

<u>Intersection #5 – San Juan Grade Road/Penzance Street</u>: This intersection will operate at an overall LOS D, but the westbound Penzance Street approach will operate at LOS F during the AM peak hour. Note that a small amount of project trips were assigned to this intersection on the Penzance Street approaches to account for the possibility that some high school traffic would use Penzance Street to circulate between Rogge Road and San



Juan Grade Road. Even if zero project trips were added to the Penzance Street approaches, the Penzance Street approaches to San Juan Grade Road would operate at LOS F during the AM peak hour due to the amount of project traffic added to San Juan Grade Road.

The San Juan Grade Road/Penzance Street intersection operates at an overall LOS A and the westbound Penzance Street approach operates at LOS E during the AM peak hour. By degrading the westbound intersection approach from LOS E, which is an acceptable operation, to LOS F, which is not acceptable, the project impact to this intersection is considered significant.

The Caltrans Peak Hour Signal Warrant is met at the San Juan Grade Road/Penzance Street intersection for the Existing Plus Project Phase 1 Condition. With signalization and the addition of a left turn on the northbound San Juan Grade approach to Penzance Street, the San Juan Grade Road/Penzance Street would operate at LOS C during each of the three study peak hours (AM, Midday and PM). The recommended intersection improvement is summarized on Exhibit 5B.

<u>Intersection #6 – San Juan Grade Road/Rogge Road</u>: This intersection is currently an allway stop controlled intersection and it would operate at LOS F during the AM peak hour with the project developed. The intersection currently operates at LOS C during the AM peak hour. The project impact to this intersection would be significant.

The following improvements would improve the operation of the intersection to acceptable levels:

- 1. Add a traffic signal.
- 2. Modify the northbound San Juan Grade approach to provide one left turn lane, one northbound through lane and one right turn lane.
- 3. Modify the southbound approach to provide one left turn lane and one shared through/right turn lane.
- 4. Modify the westbound approach to provide one exclusive left turn lane and one shared left/through/right lane.
- 5. Operate the eastbound and westbound approaches with "split" signal phasing.

With these improvements, the intersection would operate at LOS D during the AM peak hour and LOS C during the Midday and PM peak hours.

<u>Intersection #11 – Rogge Road/High School Driveway #1</u>: This intersection would operate at an overall LOS B, but the northbound driveway would operate at LOS F during the AM peak hour. This is an unacceptable operating condition. Because this driveway serves the drop-off/pick-up area, significant vehicle queuing through the drop-off/pick-up area would occur in this situation. Potentially, the queue could extend from the northbound approach (exit) to Rogge Road, through the drop-off/pick-up area and to the inbound lane (entrance) of the driveway at Rogge Road. This situation could cause gridlock that would extend onto Rogge Road. Under these conditions, motorists



dropping students off in the morning could choose to avoid entering the drop-off area in the morning and would drop their student(s) at other locations near the school, including on Rogge Road and on Jade Drive.

Signalization of the Rogge Road/Project Driveway #1 intersection is not recommended for Existing Plus Project Phase 1 conditions with the project site plan depicted on Exhibit 2A. The peak hour signalization warrant would not be met, except for the brief 20 minute period prior to the start of school under Existing Plus Project Phase 1 conditions. In addition, signalization is not necessarily the best alternative considering the high inbound and outbound vehicle demand that would occur during the morning drop-off period and the relatively short distance between Rogge Road and the parent drop-off area for both the inbound and outbound directions. Signalization of the Rogge Road intersection with the driveway serving the drop-off/pick-up area could create gridlock in the drop-off/pick-up area during periods when the signal indication for the driveway exit approach was red.

It is recommended that the school access plan be modified to achieve acceptable traffic operations at the Rogge Road intersections with the project driveways. This could necessitate separating inbound movements to the parent loading area and outbound movements from the parent loading area to two separate driveways.

Alternative Mitigation

As an alternative to the improvements described for to mitigate impacts at San Juan Grade Road/Penzance Road (Intersection 5) and San Juan Grade Road/Rogge Road (Intersection 6), the starting time of the high school could be shifted to avoid the starting and ending times of the elementary and middle schools located on Rogge Road. La Joya Elementary School begins the day at 8:00 AM and ends at 2:40 PM and the Bolsa Knolls Middle school begins school at 8:15 AM and ends at 3:10 PM. Generally, peak traffic conditions occur in the 20 minute period prior to the beginning of school and dissipate soon after the scheduled beginning of school. By scheduling the high school to begin the first period prior to 7:45 AM or after 8:30 AM, impacts identified at the San Juan Grade Road/Penzance Road and San Juan Grade Road/Rogge Road would be reduced and the two intersections would operate at satisfactory levels of service with existing intersection geometrics and traffic control. Impacts to Intersections 5 and 6 would not be significant and mitigation improvements would not be necessary.

Impacts to Intersection 1 (Natividad Road/Rogge Road) would remain significant if the starting time for the high school was shifted to before 7:45 AM or after 8:30 AM and improving the design of the left turn lane would be warranted.

3.8 Neighborhood Impacts

The high school site is located immediately east of the Bolsa Knolls residential community and the Bolsa Knolls street network provides an alternative route to Rogge Road and San Juan Grade Road for circulation between the high school and the San Juan Grade Road at Russell Road. Some high school traffic could use Penzance Street and



Jade Drive for access to avoid San Juan Grade Road north of Penzance Street and Rogge Road between San Juan Grade Road and Jade Drive. Currently, Penzance Street between San Juan Grade Road and Bollenbacher Drive and Bollenbacher Drive between Penzance Street and Rogge Road is used by parents as an access route to La Joya Elementary School.

Whether Penzance Street is used to bypass San Juan Grade Road and Rogge Road will depend on travel time differential between the two alternative routes. The Penzance Street-Jade Drive route is subject to 25 mile per hour travel speeds on these residential streets and additional delay at two all-way stop controlled intersections on Penzance Street, at Dexter Drive and Pingree Way. Additional delay is incurred turning onto and off of Rogge Road and San Juan Grade Road. The San Juan Grade Road-Rogge Road route is subject to 35 mile per hour speed limits and additional delay at the all-way stop controlled San Juan Grade/Rogge Road intersection.

It is important that the capacity of the San Juan Grade Road – Rogge Road route and the San Juan Grade Road/Rogge Road intersection in particular, be improved to serve the projected demand unless the alternative mitigation (i.e., non-conflicting school bell schedule) is implemented. If the capacity of the route is not improved to meet the demand, traffic will seek alternatives, which is in this case, is Penzance Street and Jade Drive.

Travel times between the San Juan Grade Road/Penzance Street intersection and the Rogge Road/Jade Street intersection using the two alternative routes are compared in the table below.

Travel Time Comparison Between
San Juan Grade Road/Penzance Street and Rogge Road/Jade Drive Intersections Using Alternative Routes
Existing Plus Project Conditions - AM Peak Hour

	Inbound			Outbound		
	SJG Rd	Penzance	Time	SJG Rd	Penzance St.	Time
	Rogge Rd	Jade Dr.	Differential	Rogge Rd	Jade Dr.	Differential
	(seconds)	(seconds)	(seconds)	(seconds)	(seconds)	(seconds)
No Mitigation	286	135	151	263	374	-111
With Mitigation	101	157	-56	114	178	-64

Notes:

- 1. Segment travel time based on speed limit and travel distance. Intersection delay based on Existing Plus Project intersection delay calculations using the Traffix software.
- Travel times for the no mitigation condition based on existing intersection geometrics and traffic control.
- Travel times for the with mitigation condition based on intersection geometrics and traffic control recommended on Exhibit 5B that would improve intersection operations to acceptable conditions.



Under Existing Plus Project Conditions and with no improvements, the Penzance Street – Jade Drive route would provide faster travel time from San Juan Grade Road compared to the San Juan Grade Road – Rogge Road route in the inbound direction during the AM peak hour. With signalization of the San Juan Grade Road/Rogge Road and San Juan Grade/Penzance Street intersections, the San Juan Grade Road-Rogge Road route would become the faster route to the school in the morning. In the outbound direction during the AM peak hour, the San Juan Grade Road – Penzance Street route is faster than the Penzance Street – Jade Drive route with or without the recommended improvements. The comparison of travel times indicates the importance of providing the recommended improvements for the Existing Plus Project condition. Without the recommended improvements or implementation of the alternative mitigation, it is very likely that high school traffic will utilize the Penzance Street – Jade Street route to access the high school to avoid congestion at the San Juan Grade Road/Rogge Road intersection.

3.9 Project Access and Internal Circulation

The operations of the project accesses, internal site circulation, pedestrian and bicycle circulation on and near the project site are analyzed in this section.

Project Access Operations

As previously discussed, it is recommended that project access be modified. The access plan should be designed to provide acceptable operations at the Rogge Road intersections with the project driveways.

Internal Circulation and Layout

The project site plan depicts four parking areas – 1) staff parking is provided on the westerly boundary of the project site, 2) a drop-off/pick-up and visitor parking area is provided on the Rogge Road frontage on the west portion of the site, 3) the student area is provided on the Rogge Road frontage at the center of the site, and a special events parking area is located on the easterly boundary of the project site. All driveways will be accessed from Rogge Road under Phase 1 Conditions. A connection between the visitor/drop-off/pick-up area and the main student parking lot is suggested to allow for circulation between these areas. This would reduce the potential for the use of Rogge Road for internal site circulation.

The special events parking lot will have a single access to Rogge Road and will not connect to other parking lots on site. This parking lot will only be used during special events. Eighty-one parking spaces will be provided in this parking lot. Police traffic control at the driveway intersection with Rogge Road is recommended when the parking lot is used for special events because left turn channelization is not provided on Rogge Road at this driveway.

Drop-Off/Pick-Up Areas

All drop-off and pick-up activity should be performed on-site to avoid impacting the adjacent residential neighborhood and traffic operations on Rogge Road. The site plan includes one drop-off/pick-up area for students and a second area for buses. The bus drop off area for the high school would be accessed via the student parking lot driveway



and a separate exit driveway for the buses will be provided. The proposed plan will provide satisfactory access, circulation and parking for buses.

It is recommended that this drop-off/pick-up area be managed by high school staff during both the start of school and end of the school day, to minimize conflicts between students and vehicles.

The report *Traffic Operations and Safety at Schools: Recommended Guidelines*, by Scott A. Cooner, Kay Fitzpatrick, Mark D. Wooldridge, and Garry L. Ford of the Texas Transportation Institute, dated October 2003, is a recommended guide for layout of the drop-off/pick-up areas on both school campuses. This report includes recommended drop-off/pick-up area lengths utilized by various states. A copy of this report is included within this traffic report at Appendix T.

The driveway length for vehicle stacking and curb space required for drop-off/pick-up is dependent on the volume of drop-off/pick-up vehicles generated by the school. Based on the vehicle generation observed at the five high schools surveyed for the trip generation analysis, it is estimated that during the morning peak hour, 330 vehicles would drop-off students and during the mid-day peak hour, 204 vehicles would arrive to pick-up students at project buildout. For Phase 1, 198 vehicles are estimated to drop students off in the morning and 122 vehicles would arrive in the afternoon to pick up students. Note that the trip generation characteristics of Everett Alvarez High School approximate the average of the five high school survey sample. Therefore, the drop-off/pick-up estimates provided above approximate the conditions at Everett Alvarez High School factored to the planned High School No. 5 school enrolment of 1,500 students. A higher rate of walking and bicycling might be expected at the new high school compared to Everett Alvarez High School because the new high school will be located within the Santa Rita Union School District area, which is currently primarily served by Everett Alvarez High School. Therefore, the projections of drop-off/pick-up activity cited above represent worst-case (high side) estimates of drop-off/pick-up demand.

Several studies have developed curb drop-off/pick-up stacking capacity as a function of enrolment. Guidelines published by the South Carolina Department of Transportation and Texas Transportation Institute recommend a stacking area of 1,200 feet to 1,500 feet be provided for a school with an enrolment of between 800 and 2,500 students. Based on these statistics, a stacking area of 1,325 feet should be provided for buildout of the project and 1,220 feet for Phase 1 of the project. Providing these stacking areas would reduce the potential for drop-off and pick-up activity to occur off-site and reduce the potential for on-site vehicle queuing from the drop-off/pick-up area to extend to Rogge Road.

The width of Rogge Road along the project frontage will not allow for on-street parking or loading/unloading. Rogge Road along the project frontage should be marked for no stopping at anytime.



Summary of Project Access and Internal Circulation

The efficiency of traffic operations at the Rogge Road intersections with the project access driveways is dependent on the efficiency of traffic operations on-site. Vehicle queues that extend from the drop-off/pick-up area to Rogge Road would impact traffic operations at the Rogge Road intersection with the project driveway. Providing adequate stacking area for drop-off/pick-up operations on-site will ensure vehicle queues do not extend to Rogge Road from the loading area.

To reduce project impacts related to site access and internal circulation to insignificant levels, the school access plan should be modified to achieve the following:

- 1. Acceptable intersection operations (i.e., intersection levels of service) at all Rogge Road intersections with project driveways.
- 2. Separation of the physical routes for the various transportation modes (buses, cars, pedestrians/bicycles and service vehicles) as much as possible from each other.
- 3. Adequate loading/unloading space with an adequate driveway length for queuing vehicles on site.

3.10 Project Mitigation Measures

The following improvements are recommended to mitigate the impact of project impacts:

- 1. Signalize the San Juan Grade Road/Penzance Street intersection and add a left turn lane on the northbound San Juan Grade Road approach to Penzance Street.
- 2. Construct the following improvements to the San Juan Grade Road/Rogge Road intersections:
 - a. Signalize.
 - b. Modify the northbound San Juan Grade approach to provide one left turn lane, one northbound through lane and one right turn lane.
 - c. Modify the southbound approach to provide one left turn lane and one shared through/right turn lane.
 - d. Modify the westbound approach to provide one exclusive left turn lane and one shared left/through/right lane.
 - e. Operate the eastbound and westbound approaches with "split" signal phasing.
- 3. As an alternative to the improvements described in recommendations 1 and 2 above, adjust the starting time of the high school to avoid the starting and ending times of the elementary and middle schools located on Rogge Road. Based on the current schedules for the elementary and middle schools, the high school should begin prior to 7:45 AM or after 8:30 AM.
- 4. Lengthen the left turn lane on the northbound Natividad Road approach to Rogge Road.
- 5. Redesign the access plan to achieve the following:
 - a. Acceptable intersection operations (i.e., intersection levels of service) at all Rogge Road intersections with project driveways.



- b. Separation of the physical routes for the various transportation modes (buses, cars, pedestrians/bicycles and service vehicles) as much as possible from each other.
- c. Adequate loading/unloading space with an adequate driveway length for queuing vehicles on site.
- 6. A connection between the drop-off/pick-up area and the student parking area is suggested to limit the use of Rogge Road for internal trips.
- 7. Police traffic control should be provided at the Rogge Road driveway to the special events parking lot to expedite vehicles into and out of the driveway.
- 8. Construct the missing sections of sidewalk on the north side of Rogge Road, west of the project site.
- 9. Construct bike lanes on Rogge Road west of the project site. This would require prohibiting parking on Rogge Road from La Joya Elementary School to the project site. In addition, minor widening of Rogge Road would be required on the north side of Rogge Road at locations that have not been improved with curb gutter and sidewalk.
- 10. Widen the existing sidewalk located on the south side of Rogge Road between Jade Drive and the school property boundary to 10 feet.
- 11. Provide a crosswalk across Rogge Road at the Jade Drive intersection.
- 12. Provide a bicycle parking area on the high school site.
- 13. Rogge Road along the project frontage should be marked for no stopping at anytime.



4 CUMULATIVE WITH PROJECT BUILDOUT TRAFFIC CONDITIONS

This section describes the analysis results of the study intersection and roadway segment operations under Cumulative With Project Buildout traffic conditions with the study project developed. The cumulative condition traffic forecasts are primarily based on 2030 travel forecasts prepared for the supplemental transportation analysis for the proposed Sphere of Influence (SOI) Amendment and Annexation development north of East Boronda Road in the City of Salinas. The development area is commonly referred to as the Future Growth Area and consists of three distinct plan areas (West, Central and East). A specific plan for each of the areas has not been submitted to the City of Salinas for processing and adoption. The traffic analysis for the SOI Amendment and Annexation is documented in a 2007 study entitled Salinas Sphere of Influence Amendment and Annexation Supplemental TIA that was prepared in 2007. The traffic forecasts documented in that study used a horizon year of 2030 and include the full buildout of the Salinas Future Growth Area.

4.1 Cumulative Traffic Conditions – Road Network

Exhibit 10 shows the City of Salinas *Land Use and Circulation Policy Map*. The map shows the configuration of the planned long-range road network for the City of Salinas. In the vicinity of the project site, the extension of Russell Road, McKinnon Street and El Dorado Drive are key new road links that will affect traffic access opportunities for the project.

As previously stated, the traffic projections for the cumulative scenario are primarily based on 2030 travel forecasts prepared for the supplemental transportation analysis that was prepared for the Future Growth Area. The traffic forecasting for that study included several road improvements that are anticipated to be built by 2030. The traffic forecasts for the cumulative 2030 scenario assume implementation of the following road improvements:

- 1. Blanco Road widening to 4-lanes between Alisal Street and Davis Road.
- 2. Laurel Drive widening to 6-lanes between Natividad Road and Constitution Boulevard with left turn channelization east of Constitution Boulevard.
- 3. Davis Road widening to 4-lanes between Market Street and Reservation Road.
- 4. Reservation Road widening to 4-lanes between Blanco Road and Davis Road.
- 5. San Juan Road widening to 4-lanes between Boronda Road and Rogge Road.
- 6. New interchange at US 101 and Crazy Horse Canyon Road.
- 7. New US 101/Harrison Road diamond interchange with local roadway improvements.
- 8. Eastside Road between Intergarrison Road and Giggling Road.
- 9. Intergarrision Road widening to 4-lanes between Reservation Road and Eastside Road
- 10. Sanborn Road widening to 6-lanes between John Street and Abbott Street.
- 11. General Jim Moore Boulevard widening to 4-lanes between McClure Road and South Boundary Road. This improvement has been completed.



- 12. Alisal Street widening to 4-lanes between Williams Road and Alisal Road.
- 13. Extension of Russell Road as a 4-lane arterial between San Juan Grade Road and Old Stage Road.
- 14. Extension of McKinnon Street as a 2-lane collector between Boronda Road and Russell Road.
- 15. Extension of El Dorado Drive as a 2-lane collector between Boronda Road and Russell Road.
- 16. Extension of Independence Boulevard as a 2-lane arterial between Boronda Road and Old Stage Road.
- 17. Extension of Constitution Boulevard as a 2-lane arterial between Boronda Road and Old Stage Road.
- 18. Extension of Sanborn Road as a 2-lane arterial between Boronda Road and Old Stage Road.
- 19. Boronda Road widening to 6-lane arterial between San Juan Grade Road and Williams Road.
- 20. Natividad Road widening to a 4-lane arterial between Boronda Road and Rogge Road.
- 21. Addition of two east-west 2-lane collectors between San Juan Grade Road and Williams Road.

Road network improvements that are included in the Salinas traffic Improvement Program that were not included in traffic forecast modeling for the Future Growth Area traffic study and are not considered to be included in the cumulative condition forecasts for this study are as follows:

- 1. Prunedale Bypass
- 2. Western Bypass
- 3. Eastern Bypass
- 4. US 101 widening through Salinas
- 5. Alisal Road extension
- 6. Moffett Street extension
- 7. Main Street widening to 6-lanes between Bernal Street and Market Street
- 8. Roadway extensions of Bernal Street and Constitution Boulevard into Carr Lake
- 9. Alvin Drive extension as a 4-lane arterial to include Westridge Parkway extension
- 10. US 101/Laurel interchange widening to 6-lanes between Davis Road and Adams street
- 11. Williams Road widening to 4-lanes between Freedom Parkway and Boronda Road
- 12. Espinosa Road widening to 4-lanes between US 101 and SR 183
- 13. Blanco Road extension as a 4-lane arterial between Reservation Road and Imjin Road

4.2 Cumulative Traffic Volumes

The 2030 cumulative condition volumes are primarily based upon the 2030 travel forecasts estimated using the association of Monterey Bay Area Governments (AMBAG) Regional Travel Forecasting Model that are documented in the SOI Supplemental TIA.



The forecasts in the SOI Supplemental TIA were supplemented with travel forecasts developed for the City of Salinas General Plan Circulation Study, the Rogge Road High School Traffic Analysis Report prepared in 2006 and the Creekbridge II Transportation Planning Report prepared in 2008. Traffic volumes consistent with the average daily traffic volumes (ADT's) developed from the SOI Supplemental TIA and other referenced reports were used to estimate intersection turning volumes at the study intersections.

4.3 Project Description – Buildout

At buildout, the capacity of the school would be 1,500 students. Access to the school as described for Phase 1 would be maintained with access provided via five driveways to Rogge Road. The easterly most driveway would only be utilized during special school events.

4.4 Project Trip Generation

Exhibit 6B contains the trip generation estimate for buildout of the study project. This trip generation estimate is a combination of the previous trip generation under Phase 1, plus the additional trips that would be generated by the additional students originating in the Future Growth Area.

The high school at full buildout would generate 2,768 trips per day, with 825 trips (495 in, 330 out) during the AM peak hour, 510 trips (204 in, 306 out) during the midday peak hour and 270 trips (119 in, 151 out) during the PM street peak hour.

4.5 Project Trip Distribution and Assignment

A project trip assignment for the buildout of the school was developed that included Phase 1 trips and the trips that will be generated by the enrollment that would be generated within the Future Growth Area. The Phase 1 project trip distribution origins remain unchanged from the Existing Plus Project Phase 1 analysis previously described. However, the assignment of Phase 1 trips was revised to account for the new roadways that would be constructed in conjunction with the development of the Future Growth Area. Exhibit 11 shows the trip distribution pattern for the high school trips that would be generated within the Future Growth Area.

The assignment of AM, Midday and PM peak hour high school trips to the study intersections for the buildout condition is shown on Exhibits 12A, 12B and 12C, respectively.

The construction of the Future Growth Area road network that includes the extensions of Russell Road, McKinnon Street and El Dorado Drive would alter travel patterns for motorists traveling to and from the school. For the buildout condition, the arrival/distribution pattern for the school shifts from a predominate westerly orientation under Phase 1 to a predominant easterly orientation with buildout. Under the Phase 1 scenario, 80% of the high school trips were estimated to arrive and depart to and from the west. With buildout, 78% of the high school trips were estimated to arrive and depart to and from the east.



4.6 Cumulative With Project Buildout Traffic Conditions - Intersection Operations

Cumulative With Project Buildout morning, midday, and evening peak hour turning volumes are illustrated on Exhibits 13A, 13B, and 13C. Exhibit 5A tabulates corresponding morning, midday, and evening peak hour levels of service, the details of which are presented in Appendix C through N.

Cumulative conditions were evaluated based on existing intersection geometrics and traffic control. The exception was the San Juan Grade Road/Russell Road extension. Because this Russell Road will be extended to the east, which will add a fourth leg to the intersection, the initial intersection level of service calculation was based on the intersection lane configuration documented in the *City of Salinas Transportation Improvement Program*, 2010 Update.

Subsequent to the initial intersection calculation, intersection geometrics documented in the *City of Salinas Transportation Improvement Program*, 2010 Update were analyzed.

Based on existing intersection geometrics, the following five intersections would not operate at acceptable levels of service under Cumulative With Project Buildout Conditions:

- 1. Natividad Road/Rogge Road
- 2. Natividad Road/Boronda Road
- 3. San Juan Grade Road/Boronda Road
- 4. San Juan Grade Road/Penzance Street
- 5. San Juan Grade Road/Rogge Road

In addition, the San Juan Grade Road/Russell Road intersection will not operate at a satisfactory level of service with the intersection improvements described in the Salinas TIP. The intersection of Rogge Road and project driveway #1 will also not operate at a satisfactory level of service during the AM peak hour under cumulative conditions.

The Rogge Road/El Dorado Drive intersection would operate at LOS D during the AM peak hour and LOS C during the midday and PM peak hours with signal traffic control and the following lane configuration:

- o Westbound one left turn lane and one through lane,
- o Eastbound one through lane and one right turn lane,
- o Northbound one left turn lane and one right turn lane.

A discussion of the traffic operations for each intersection with operational deficiencies under Cumulative With Project Buildout Conditions is provided below.



Intersection #1 – Natividad Road/Rogge Road

This intersection is currently stop controlled on the eastbound Rogge Road approach to Natividad Road. The eastbound approach is projected to operate at LOS F during the AM and Midday peak hours.

Signalization of the intersection is warranted under Cumulative With Project Buildout conditions. In addition, widening of Natividad Road between Boronda Road and Rogge Road to a four-lane road is planned in conjunction with the development of the Future Growth Area. It is recommended that the eastbound right turn lane be designed as a free right turn movement that becomes the second southbound right turn lane on Natividad Road south of Rogge Road. With these improvements the intersection would operate at LOS C during the AM peak hour and LOS B during the Midday and PM peak hours.

Intersection #2 – Natividad Road/Boronda Road

The Natividad Road/Boronda Road intersection would operate at LOS F during the AM, Midday and PM peak hours under Cumulative With Project Buildout conditions with existing geometrics. Boronda Road and Natividad Road are planned for widening in conjunction with the development of the Future Growth Area. The Salinas TIP indicates that the Natividad Road/Boronda Road intersection approaches will be widened to provide the following intersection design:

- o Northbound: Two left turn lanes, two through lanes and one right turn lane.
- o Southbound: Two left turn lanes, two through lanes and one right turn lane.
- o Eastbound: Two left turn lanes, three through lanes and one right turn lane.
- o Westbound: Two left turn lanes, three through lanes and one right turn lane.

With the intersection improved as described above, the intersection would operate at LOS E during the Midday peak hours. To achieve acceptable operations, a fourth westbound through lane would be required. Alternatively, the traffic forecasts do not reflect the addition of the Alvin Drive extension over Highway 101. This improvement would divert traffic from Boronda Road and improve operations at the Natividad Road/Boronda Road intersection.

Intersection #3 – San Juan Grade Road/Boronda Road

The San Juan Grade Road/Boronda Road intersection would operate at LOS F during the Midday and PM peak hours under Cumulative With Project Buildout conditions with existing geometrics. Boronda Road and San Juan Grade Road are planned for widening in conjunction with the development of the Future Growth Area. The north, south and east legs of the intersection have been widened to their ultimate widths as described in the Salinas TIP. The Salinas TIP indicates that the west leg will be widened to provide 2 left turn lanes, 3 through lanes and one right turn lane. With the intersection improved as planned in the Salinas TIP, the intersection would operate at LOS D during the AM and PM peak hours, but at an unacceptable LOS E during the Midday peak hour. To achieve an acceptable level of service, the northbound San Juan Grade right turn lane would need to be continued onto eastbound Boronda Road as a free right turn and the southbound



right turn operated with an overlap phase. Provision of the third eastbound through lane as well as the northbound to eastbound free right turn is problematic given that the existing development located in the southwest and southeast quadrant of the intersection.

The analysis road network that was modeled for the Cumulative With Project Buildout Condition does not include the Alvin Street extension. The extension of Alvin Street over U.S. 101 to N. Davis Street will reduce traffic on Boronda Road. This improvement would improve traffic operations on Boronda Road and at the San Juan Grade Road/Boronda Road intersection.

<u>Intersection #4 – San Juan Grade Road/Russell Road</u>

This intersection would operate at LOS E during the AM and mid-day peak hours under cumulative conditions. The improvements described in the Salinas TIP for the San Juan Grade Road/Russell Road intersection provide the following lane configuration on the intersection approaches:

- o Northbound: Two left turn lanes, one through lane and one shared/right turn lane.
- o Southbound: Two left turn lanes, two through lanes and one right turn lane.
- o Eastbound: Two left turn lanes, two through lanes and one right turn lane.
- o Westbound: Two left turn lanes, two through lanes and one right turn lane.

Providing a right turn lane operated with an overlap phase on the northbound San Juan Grade approach would achieve LOS D operations during the AM peak hour and LOS C during the mid-day peak hours. The northbound approach would be configured to provide two left turn lanes, two through lanes and one right turn lane.

Intersection #5 – San Juan Grade Road/Penzance Street

The westbound Penzance Street approach to San Juan Grade Road would operate at LOS F during the AM peak hour. With signalization and the addition of a northbound left turn lane, the intersection would operate at LOS B during the AM peak hour under cumulative conditions. Intersection levels of service with the recommended improvements are summarized on Exhibit 5A.

<u>Intersection #6 – San Juan Grade Road/Rogge Road</u>

As an all-way stop controlled intersection, the San Juan Grade Road/Rogge Road intersection would operate at LOS E during the AM peak hour under cumulative conditions. The peak hour signal warrant is met for the AM peak hour condition. Signalization of the intersection and modification of the intersection as described under Existing Plus Project Phase 1 conditions would improve intersection operations to LOS C during the AM peak hour. The following improvements would improve the operation of the intersection to acceptable levels:

- 1. Add a traffic signal.
- 2. Modify the northbound San Juan Grade approach to provide one left turn lane, one northbound through lane and one right turn lane.



- 3. Modify the southbound approach to provide one left turn lane and one shared through/right turn lane.
- 4. Modify the westbound approach to provide one exclusive left turn lane and one shared left/through/right lane.
- 5. Operate the eastbound and westbound approaches with "split "signal phasing.

Intersection #11 – Rogge Road/High School Driveway #1: This intersection would operate at an overall LOS F and the northbound driveway would operate at LOS F during the AM peak hour. This is an unacceptable operating condition.

As with Existing Plus Project Phase 1 Conditions, signalization of the Rogge Road/Project Driveway #1 intersection is not recommended for Buildout Conditions. To improve driveway operations, the access plan for the school be modified as described for the Existing Plus Project Condition.

4.7 Traffic Impact Fees

The City of Salinas administers a traffic impact fee to fund planned improvements to the City's road network. The improvements funded by the traffic impact fee include the following improvements in the immediate vicinity of the project:

- TIP 8 Russell Road Extension east of San Juan Grade Road
- TIP 9 Natividad Road widening between Boronda Road and Rogge Road
- TIP 10 El Dorado Drive Extension between Boronda Road and Rogge Road
- TIP 11 McKinnon Street Extension between Boronda Road and Russell Road
- TIP 12- Russell Road widening between Main Street and San Juan Grade Road
- TIP 13- San Juan Grade widening between Boronda Road and Rogge Road
- TIP 49 San Juan Grade/Russell Road intersection improvements
- TIP 50 San Juan Grade/Boronda Road intersection improvements
- TIP 51 Boronda Road/Natividad Road intersection improvement
- TIP 58 Natividad Road / Russell Road intersection improvements

Signalization of the Natividad Road/Rogge Road intersection is not included in the traffic impact fee program and the project should provide a pro-rata contribution based on the percentage of trips at the intersection that are generated by the new high school. The right turn lane recommended for the northbound San Juan Grade approach to Russell Road for the Buildout Condition should be added to the Salinas Traffic Improvement Program. Signalization of the San Juan Grade/Penzance and San Juan Grade/Rogge Road intersections were recommended to mitigate project impacts under the Existing Plus Project Phase 1 scenario. The results of the analysis also indicate that the Alvin Street extension is required for the long-term buildout development condition. The Alvin Street extension will divert traffic from Boronda Road resulting in better operations than forecast in this study.

The improvements listed above will mitigate cumulative project impacts. With the specific exception of improvements recommended for the Natividad Road/Rogge Road, San Juan Grade Road/Penzance Street and San Juan Grade Road/Rogge Road



intersections, the City of Salinas is collecting fees for the construction of the improvements described above via the Traffic Impact Fee program. New development, including new development in the Future Growth Area, will pay traffic impact fees to the City of Salinas.



5 ALTERNATIVE SITE PLANS

Two alternative site plans for the high school are displayed on Exhibits 14A and 14B. These site plans maintain access to the project from Rogge Road, but relocate the driveways to the easterly portion of the project site. Peak hour traffic volume projections for the Project Phase 1 and Buildout Conditions are shown on Exhibits 15A and 15B and level of service calculations for the Rogge Road intersections with the project driveways are shown on Exhibit 16.

Both alternative site plans locate an access driveway adjacent to the easterly property boundary. In the event that site plan Alternative 1 or Alternative 2 is adopted and a collector street is ultimately constructed directly adjacent to the school's eastern property boundary, it is recommended that access to the easterly high school driveway be relocated from Rogge Road to the new north-south collector street. This would leave one access driveway connection to Rogge Road and one access driveway to the north-south collector. Overall, this configuration, if achievable, would reduce high school related traffic on Rogge Road as it is expected that when the Future Growth Area develops high school traffic will access the site from the south and east to a greater degree than during Phase 1 of the project. Alternatively, access could also be provided via a connection to the Russell Road extension.

By locating the driveways to the easterly portion of the site, vehicular access is separated from the primary pedestrian and bicycle activity, which will be primarily to and from the west, at least for Phase 1. The plans for the project alternatives do not show a shared path along the project frontage on Rogge Road. This should be included in any alternative plan for the project. In addition, left turn channelization on Rogge Road at the project driveways is not show on the alternative plans. Left turn channelization should be included on Rogge Road at the project driveways for the project alternatives.

The access plan for Site Plan Alternative 1 is very similar to the access plan for the preferred project except the number of driveways to Rogge Road have been consolidated from five to two and an internal connection is provided between the student parking lot and the visitor parking lot. Both of these changes are positive improvements to the access plan. However, with access Alternative 1, student parking lot access would be primarily limited to the westerly driveway (Driveway #1) and visitor/staff/drop-off/pick-off access would be primarily concentrated at the easterly driveway (Driveway #2). Because of this, intersection operations at the Rogge Road/Driveway #2 intersection are unacceptable during the AM peak hour for both the Phase 1 and Buildout Conditions. Given the long driveways provided on-site between Rogge Road and the drop-off/pick-up area, signalization of the Rogge Road/Driveway #2 intersection is recommended for the Project Phase 1 and Buildout Conditions. In addition, a free right turn movement from northbound Driveway 2 to eastbound Rogge Road would be required for the buildout condition, assuming access is not reconfigured to a new north-south collector street built adjacent to the school's eastern property line. Both driveway approaches to Rogge Road should be designed to provide separate left and right turn lanes. These improvements would improve traffic operations to acceptable levels.

The access plan for Site Plan Alternative 2 provides two driveways to Rogge Road and an internal circulation plan that would balance turning volumes at the two driveways. For this



reason, acceptable driveway operations are projected for the Phase 1 access plan for Alternative 2. Left turn channelization would be required on Rogge Road at both intersection driveways and both driveways should be designed to provide separate left and right turn lanes on the approach to Rogge Road.

Under Buildout Conditions, signalization of the easterly most driveway is recommended and a free right turn should be provided from the northbound driveway approach to eastbound Rogge Road at the same driveway. This assumes access is not relocated to a new north-south collector street built on the school's eastern property boundary.

In terms of access from Rogge Road, Site Plan Alternative 2 provides a better access plan than Alternative 1 because the internal connections between parking areas on-site will allow for the distribution of inbound and outbound traffic between the two driveways. In terms of access from Rogge Road, Alternative 2 is at least equal if not better than the project plan shown on Exhibit 2A.

Both Alternatives locate the parent loading area to the interior of the site, which will provide onsite queuing area on the approach to the drop-off/pick-up area and on the exiting approach to Rogge Road that is not provided with the project plan. With regards to on-site vehicle queuing for parent loading, Alternatives 1 and 2 are preferred over the project plan shown on Exhibit 2A. However, the area provided for drop-offs is not adequate for Alternatives 1 and 2.



6 RECOMMENDATIONS

The following sections summarize the recommended improvements within this report.

6.1 Recommended Improvements Under Existing Conditions

The following improvement is recommended for Existing Conditions:

1. The left turn lane on northbound Natividad Road at Rogge Road should be lengthened to provide for vehicle deceleration and vehicle storage.

6.2 Recommended Improvements Under Existing Plus Project Phase 1 Conditions

The following improvements are recommended under Existing Plus Project Phase 1 conditions:

- 2. Signalize the San Juan Grade Road/Penzance Street intersection and add a left turn lane on the northbound San Juan Grade Road approach to Penzance Street.
- 3. Construct the following improvements to the San Juan Grade Road/Rogge Road intersections:
 - a. Signalize
 - b. Modify the northbound San Juan Grade approach to provide one left turn lane, one northbound through lane and one right turn lane.
 - c. Modify the southbound approach to provide on left turn lane and one shared through/right turn lane.
 - d. Modify the westbound approach to provide one exclusive left turn lane and one shared left/through/right lane.
 - e. Operate the eastbound and westbound approaches with "split" signal phasing.
- 4. As an alternative to the improvements described in recommendations 1 and 2 above, adjust the starting time of the high school to avoid the starting and ending times of the elementary and middle schools located on Rogge Road.
- 5. Lengthen the left turn lane on the northbound Natividad Road approach to Rogge Road.
- 6. Redesign the access plan to achieve the following:
 - a. Acceptable intersection operations (i.e., intersection levels of service) at all Rogge Road intersections with project driveways.
 - b. Separation of the physical routes for the various transportation modes (buses, cars, pedestrians/bicycles and service vehicles) as much as possible from each other.
 - Adequate loading/unloading space with an adequate driveway length for queuing vehicles on site.
- 7. A connection between the parent drop-off/pick-up area and student parking area is suggested to limit the use of Rogge Road for internal trips.



- 8. Police traffic control should be provided at the Rogge Road driveway to the special events parking lot to expedite vehicles into and out of the driveway.
- 9. Construct the missing sections of sidewalk on the north side of Rogge Road, west of the project site.
- 10. Construct bike lanes on Rogge Road west of the project site. This would require prohibiting parking on Rogge Road from La Joya Elementary School to the project site. In addition, minor widening of Rogge Road would be required on the north side of Rogge Road at locations that have not been improved with curb gutter and sidewalk.
- 11. Widen the existing sidewalk located on the south side of Rogge Road between Jade Drive and the school property boundary to 10 feet.
- 12. Provide a crosswalk across Rogge Road at the Jade Drive intersection.
- 13. Provide a bicycle parking area on the high school site.
- 14. Rogge Road along the project frontage should be marked for no stopping at anytime.

6.3 Recommended Improvements Under Cumulative With Project Buildout Conditions

The following improvements are recommended under Cumulative With Project Buildout conditions:

- Signalize the Natividad Road/Rogge Road intersection and widen Natividad Road between Boronda Road and Rogge Road to a four-lane road as planned in conjunction with the development of the Future Growth Area. The eastbound right turn lane from Rogge Road to southbound Natividad Road should be designed as a free right turn movement that becomes the second southbound right turn lane on Natividad Road south of Rogge Road.
- 2. Widen the Natividad Road/Boronda Road intersection as described in the Salinas Transportation Improvement Program as follows:
 - o Northbound: Two left turn lanes, two through lanes and one right turn lane.
 - o Southbound: Two left turn lanes, two through lanes and one right turn lane.
 - o Eastbound: Two left turn lanes, three through lanes and one right turn lane.
 - o Westbound: Two left turn lanes, three through lanes and one right turn lane.
 - In addition to these improvements, a fourth westbound through lane would be required. In lieu of the fourth westbound through lane, construct the Alvin Street extension to Westridge Parkway.
- 3. Widen the west leg of the San Juan Grade Road/Boronda Road intersection as described in the Salinas Transportation Improvement Program to provide two left turn lanes, three through lanes and one right turn lane. Also, convert the northbound San Juan Grade right turn lane to a free right turn lane. In lieu of these improvements, construct the Alvin Street extension to Westridge Parkway.
- 4. In addition to the improvements described in the Salinas TIP for the San Juan Grade Road/Russell Road intersection, a right turn lane should be added on the northbound San Juan Grade Road approach to Russell Road and the right turn movement should be operated with a right turn overlap phase.



- 5. Signalize the San Juan Grade Road/Penzance Street intersection and add a left turn lane on the northbound San Juan Grade Road approach to Penzance Street.
- 6. Construct the following improvements to the San Juan Grade Road/Rogge Road intersections:
 - a. Signalize
 - b. Modify the northbound San Juan Grade approach to provide one left turn lane, one northbound through lane and one right turn lane.
 - c. Modify the southbound approach to provide on left turn lane and one shared through/right turn lane.
 - d. Modify the westbound approach to provide one exclusive left turn lane and one shared left/through/right lane.
 - e. Operate the eastbound and westbound approaches with "split" signal phasing.
- 7. If a collector street is located on the immediate boundary of the school property, access to the special events parking area should be relocated to the collector street.
- 8. The project access should be modified as described for Existing Plus Project Conditions.

The right turn lane recommended for the northbound San Juan Grade approach to Russell Road for the Buildout Condition should be added to the Salinas Traffic Improvement Program. The project should provide a pro-rata contribution to the signalization of the Natividad Road/Rogge Road intersection (Improvement #1). Signalization of the San Juan Grade/Penzance and San Juan Grade/Rogge Road intersections were recommended to mitigate project impacts under the Existing Plus Project Phase 1 scenario.

APPENDIX A LEVEL OF SERVICE DESCRIPTIONS

APPENDIX A1

LEVEL OF SERVICE (LOS) DESCRIPTION SIGNALIZED INTERSECTIONS

The capacity of an urban street is related primarily to the signal timing and the geometric characteristics of the facility as well as to the composition of traffic on the facility. Geometrics are a fixed characteristic of a facility. Thus, while traffic composition may vary somewhat over time, the capacity of a facility is generally a stable value that can be significantly improved only by initiating geometric improvements. A traffic signal essentially allocates time among conflicting traffic movements that seek to use the same space. The way in which time is allocated significantly affects the operation and the capacity of the intersection and its approaches.

The methodology for signalized intersection is designed to consider individual intersection approaches and individual lane groups within approaches. A lane group consists of one or more lanes on an intersection approach. The outputs from application of the method described in the HCM 2000 are reported on the basis of each lane. For a given lane group at a signalized intersection, three indications are displayed: green, yellow and red. The red indication may include a short period during which all indications are red, referred to as an all-red interval and the yellow indication forms the change and clearance interval between two green phases.

The methodology for analyzing the capacity and level of service must consider a wide variety of prevailing conditions, including the amount and distribution of traffic movements, traffic composition, geometric characteristics, and details of intersection signalization. The methodology addresses the capacity, LOS, and other performance measures for lane groups and the intersection approaches and the LOS for the intersection as a whole.

Capacity is evaluated in terms of the ratio of demand flow rate to capacity (v/c ratio), whereas LOS is evaluated on the basis of control delay per vehicle (in seconds per vehicle). The methodology does not take into account the potential impact of downstream congestion on intersection operation, nor does the methodology detect and adjust for the impacts of turn-pocket overflows on through traffic and intersection operation.

LEVEL OF SERVICE (LOS) CRITERIA FOR SIGNALIZED INTERSECTIONS (Reference Highway Capacity Manual 2000)

 Level of Service
 Control Delay (seconds / vehicle)

 A
 <10</td>

 B
 >10 - 20

 C
 >20 - 35

 D
 >35 - 55

 E
 >55 - 80

 F
 >80

APPENDIX A2

LEVEL OF SERVICE (LOS) DESCRIPTION UNSIGNALIZED INTERSECTIONS WITH ALL-WAY STOP CONTROL (AWSC)

AWSC intersections require every vehicle to stop at the intersection before proceeding. Since each driver must stop, the judgement as to whether to proceed into the intersection is a function of traffic conditions on the other approaches. While giving priority to the driver on the right is a recognized rule in some areas, it is not a good descriptor of actual intersection operations. What happens is the development of a consensus of right-of-way that alternates between the drivers on the intersection approaches, a consensus that depends primarily on the intersection geometry and the arrival patterns at the stop line.

If no traffic is present on the other approaches, a driver can proceed immediately after the stop is made. If there is traffic on one or more of the other approaches, a driver proceeds only after determining that there are no vehicles currently in the intersection and that it is the driver's turn to proceed. Since no traffic signal controls the stream movement or allocates the right-of-way to each conflicting stream, the rate of departure is controlled by the interaction between the traffic streams themselves.

For AWSC intersections, the average control delay (in seconds per vehicle) is used as the primary measure of performance. Control delay is the increased time of travel for a vehicle approaching and passing through an AWSC intersection, compared with a free-flow vehicle if it were not required to slow down or stop at the intersection.

The criteria for AWSC intersections have different threshold values than do those for signalized intersections, primarily because drivers expect different levels of performance from different kinds of traffic control devices (i.e traffic signals, two way stop or all way stop, etc.). The expectation is that a signalized intersection is designed to carry higher traffic volumes than an AWSC intersection and a higher level of control delay is acceptable at a signalized intersection for the same LOS.

For AWSC analysis using the HCM 2000 method, the LOS shown reflects the weighted average of the delay on each of the approaches.

LEVEL OF SERVICE (LOS) CRITERIA FOR AWSC INTERSECTIONS (Reference Highway Capacity Manual 2000)

Level of Service	Control Delay (seconds / vehicle)	
A	0 - 10	
В	>10 - 15	
C	>15 - 25	
D	>25 - 35	
E	>35 - 50	
F	>50	

APPENDIX A3

LEVEL OF SERVICE (LOS) DESCRIPTION UNSIGNALIZED INTERSECTIONS WITH TWO-WAY STOP CONTROL (TWSC)

TWSC intersections are widely used and stop signs are used to control vehicle movements at such intersections. At TWSC intersections, the stop-controlled approaches are referred to as the minor street approaches; they can be either public streets or private driveways. The intersection approaches that are not controlled by stop signs are referred to as the major street approaches. A three-leg intersection is considered to be a standard type of TWSC intersection if the single minor street approach (i.e. the stem of the T configuration) is controlled by a stop sign. Three-leg intersections where two of the three approaches are controlled by stop signs are a special form of unsignalized intersection control.

At TWSC intersections, drivers on the controlled approaches are required to select gaps in the major street flow through which to execute crossing or turning maneuvers on the basis of judgement. In the presence of a queue, each driver on the controlled approach must use some time to move into the front-of-queue position and prepare to evaluate gaps in the major street flow. Capacity analysis at TWSC intersections depends on a clear description and understanding of the interaction of drivers on the minor or stop-controlled approach with drivers on the major street. Both gap acceptance and empirical models have been developed to describe this interaction.

Thus, the capacity of the controlled legs is based on three factors:

- the distribution of gaps in the major street traffic stream;
- driver judgement in selecting gaps through which to execute the desired maneuvers; and
- the follow-up time required by each driver in a queue.

The delay experienced by a motorist is made up of a number of factors that relate to control, geometrics, traffic and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions, in the absence of incident, control, traffic or geometric delay. Average control delay for any particular minor movement is a function of the capacity of the approach and the degree of saturation and referred to as level of service.

LEVEL OF SERVICE (LOS) CRITERIA FOR TWSC INTERSECTIONS (Reference Highway Capacity Manual 2000)

Level of Service	Control Delay (seconds / vehicle)	
A	0 - 10	
В	>10 - 15	
C	>15 - 25	
D	>25 - 35	
E	>35 - 50	
F	>50	

APPENDIX B INTERSECTION TURNING MOVEMENT VOLUME COUNT DATES

SALINAS #5 HIGH SCHOOL TRAFFIC ANALYSIS INTERSECTION TURNING MOVEMENT VOLUME COUNT DATE

	Intersection		AM Peak Period	Midday Peak Period	PM Peak Period
	North-South Street	East-West Street	AMFORKEONOG	Wildday Feak Fellod	FINI FORK PORTOG
1	Natividad Road	Rogge Road	Wed, March 2, 2011	Wed, March 2, 2011	Wed, March 2, 2011
2	Natividad Road	Boronda Road	Wed, May 11, 2011	Wed, March 2, 2011	Wed, May 11, 2011
3	San Juan Grade Road	Boronda Road	Thur, May 12, 2011	Thur, March 3, 2011	Thur, May 12, 2011
4	San Juan Grade Road	Russell Road	Tue, March 1, 2011	Tue, March 1, 2011	Tue, March 1, 2011
5	San Juan Grade Road	Penzance Street	Tue, April 26, 2011	Tue, April 26, 2011	Tue, April 26, 2011
6	San Juan Grade Road	Rogge Road	Tue, March 1, 2011	Tue, March 1, 2011	Tue, March 1, 2011
7	Keiton Drive	Rogge Road	Tue, May 3, 2011	Tue, May 3, 2011	Tue, May 3, 2011
8	Bollenbacher Drive	Rogge Road	Wed, April 27, 2011	Wed, April 27, 2011	Wed, April 27, 2011
9	Jasper Way	Rogge Road	Thur, April 28, 2011	Thur, April 28, 2011	Thur, April 28, 2011
10	Jade Drive	Rogge Road	Thur, April 28, 2011	Thur, April 28, 2011	Thur, April 28, 2011

APPENDIX C INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

Natividad Road/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************************** Intersection #1 Natividad / Rogge ************************ Average Delay (sec/veh): 7.5 Worst Case Level Of Service: B[15.0] ************************* Natividad Street Name: Approach: Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Movement: -----|-----|------| Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Channel Include Include 1 0 1 0 0 0 0 1 0 1 0 0 0 1 0 0 0 0 -----|----|-----|------| Volume Module: Base Vol: 235 172 0 0 139 .34 49 0 270 Initial Bse: 235 172 0 0 139 34 49 0 270 0 PHF Volume: 305 223 0 181 44 351 0 64 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 305 223 0 181 0 44 64 0 351 0 -----|----|-----|------| Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxxx 6.4 xxxx 6.2 xxxxx xxxx xxxxx -----|----||------||------| Capacity Module: Cnflict Vol: 225 xxxx xxxxx xxxx xxxx xxxx 1036 xxxx 203 xxxx xxxx xxxxx Potent Cap.: 1344 xxxx xxxxx xxxx xxxx xxxx 255 xxxx 836 xxxx xxxx xxxxx Move Cap.: 1344 xxxx xxxxx xxxx xxxx xxxx 211 xxxx 836 xxxx xxxx xxxxx Volume/Cap: 0.23 xxxx xxxx xxxx xxxx xxxx 0.30 xxxx 0.42 xxxx xxxx xxxx -----|----|-----|------| Level Of Service Module: Control Del: 8.5 xxxx xxxxx xxxxx xxxxx 29.3 xxxx 12.4 xxxxx xxxx xxxxx LOS by Move: A * * * * * D * B * * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT ApproachDel: XXXXXX XXXXXX 15.0 XXXXXX ApproachLOS: В ******************* Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) *********************************** Intersection #1 Natividad / Rogge Average Delay (sec/veh): Worst Case Level Of Service: B[14.8] ******************************** Natividad Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RControl: Uncontrolled Uncontrolled Stop Sign Rights: Include Include Channel Stop Sign Include Include Lanes: 1 0 1 0 0 0 0 0 1 0 1 0 0 0 0 0 0 -----| Volume Module: 0 172 198 313 0 Base Vol: 37 0 245 48 Initial Bse: 198 313 0 0 172 48 37 0 245 0 215 PHF Volume: 248 391 0 60 46 0 306 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 60 FinalVolume: 248 391 0 215 46 0 306 0 0 -----| Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxx 6.4 xxxx 6.3 xxxxx xxxx xxxxx FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxx 3.5 xxxx 3.3 xxxxx xxxx xxxxx -----| Capacity Module: Cnflict Vol: 275 xxxx xxxxx xxxx xxxx xxxx 1131 xxxx 245 xxxx xxxx xxxxx Potent Cap: 1271 xxxx xxxxx xxxx xxxx xxxx 222 xxxx 786 xxxx xxxx xxxxx Move Cap.: 1271 xxxx xxxxx xxxx xxxx xxxx 189 xxxx 786 xxxx xxxx xxxx Volume/Cap: 0.19 xxxx xxxx xxxx xxxx xxxx 0.25 xxxx 0.39 xxxx xxxx xxxx Level Of Service Module: 2Way95thQ: 0.7 xxxx xxxxx xxxx xxxx xxxxx 0.9 xxxx 1.9 xxxx xxxx xxxxx Control Del: 8.5 xxxx xxxxx xxxxx xxxx xxxxx 30.2 xxxx 12.5 xxxxx xxxx xxxxx В LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT ApproachDel: xxxxxx 14.8 XXXXXX XXXXXX ApproachLOS: В ************************************ Note: Queue reported is the number of cars per lane. *************************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************************* Intersection #1 Natividad / Rogge ************************** Average Delay (sec/veh): 4.3 Worst Case Level Of Service: B[11.7] ***** Natividad Street Name: Rogge North Bound South Bound Approach: East Bound West Bound North Bound South Bound East Bound L - T - R L - T - R L - T - R Movement: Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Channel Include Rights: Include Include Channel Include Lanes: 1 0 1 0 0 0 0 1 0 1 0 0 0 1 0 0 0 Volume Module: 144 138 0 Base Vol: 0 219 50 27 0 135 50 Initial Bse: 144 138 0 0 219 27 0 135 0 0 PHF Volume: 160 153 0 0 243 56 30 0 150 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 .0 0 FinalVolume: 160 153 0 0 243 0 0 150 56 30 0 Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxxx 6.4 xxxx 6.2 XXXXX XXXX XXXX Capacity Module: Cnflict Vol: 299 xxxx xxxxx xxxx xxxx xxxx 744 xxxx 271 xxxx xxxx xxxxx 770 xxxx xxxx xxxx Potent Cap.: 1262 xxxx xxxxx xxxx xxxx xxxx 383 xxxx Move Cap.: 1262 xxxx xxxxx xxxx xxxx xxxxx 346 xxxx 770 xxxx xxxx xxxxx Volume/Cap: 0.13 xxxx xxxx xxxx xxxx xxxx 0.09 xxxx 0.19 xxxx xxxx xxxx -----||-----||------| Level Of Service Module: 2Way95thQ: 0.4 xxxx xxxxx xxxx xxxx xxxx 0.3 xxxx 0.7 xxxx xxxx xxxx Control Del: 8.3 xxxx xxxxx xxxxx xxxx xxxxx 16.4 xxxx 10.8 xxxxx xxxx xxxxx C * B * * * LOS by Move: A * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shared LOS: * * * * * * * * * * 11.7 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ************************************ Note: Queue reported is the number of cars per lane.

```
Level Of Service Computation Report
      2000 HCM Unsignalized Method (Base Volume Alternative)
*************************
Intersection #1 Natividad / Rogge
Average Delay (sec/veh): 10.1
                    Worst Case Level Of Service: C[ 20.8]
*****************************
Street Name:
           Natividad
                              Rogge
Approach:
       North Bound South Bound
      North Bound South Bound East Bound West Bound L - T - R L - T - R
                        East Bound West Bound
Movement:
-----|-----|------|
Control: Uncontrolled Uncontrolled Stop Sign
                               Stop Sign
Rights: Include Include Channel Include Lanes: 1 0 1 0 0 0 0 1 0 1 0 0 0 1 0 0 0 0
-----||-----|----|
Volume Module:
     289 172 0 0 139 40
Base Vol:
                       53 0 306
                                0 0 0
Initial Bse: 289 172 0
              0 139
                    40
                       53 0 306
56 75 0 431
0 0 0 0
PHF Volume: 407 242 0 0 196
                                0 0
                                       0
Reduct Vol: 0 0
            0
                0 0
                                   0
                                 0
                                       0
FinalVolume: 407 242 0
                     56
                        75 0 431
                0 196
                                 0 0
                                       Ω
Critical Gap Module:
Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx 6.4 xxxx 6.2 xxxxx xxxx xxxxx
-----||-----|----||------|
Capacity Module:
Cnflict Vol: 252 xxxx xxxxx xxxx xxxx xxxxx 1280 xxxx
                             224 xxxx xxxx xxxxx
Potent Cap.: 1313 xxxx xxxxx xxxx xxxxx 182 xxxx 813 xxxx xxxx xxxxx
Volume/Cap: 0.31 xxxx xxxx xxxx xxxx xxxx 0.54 xxxx 0.53 xxxx xxxx xxxx
-----|
Level Of Service Module:
2Way95thQ: 1.3 xxxx xxxxx xxxx xxxx 2.6 xxxx 3.2 xxxx xxxx xxxx
Control Del: 9.0 xxxx xxxxx xxxxx xxxxx 58.0 xxxx 14.3 xxxxx xxxx xxxxx
LOS by Move: A * * * * * F * B * * *
Movement: LT - LTR - RT
              LT - LTR - RT
                       LT - LTR - RT
                                LT - LTR - RT
ApproachDel: xxxxxx
                          20.8
               XXXXXX
                                 XXXXXX
       *
ApproachLOS:
                          C
****************************
Note: Queue reported is the number of cars per lane.
*****************************
```

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************************** Intersection #1 Natividad / Rogge ************************* Average Delay (sec/veh): 6.7 Worst Case Level Of Service: C[16.5] ***** Natividad Street Name: Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: _____| Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Channel Include Lanes: 1 0 1 0 0 0 0 0 1 0 1 0 0 0 0 0 0 _____ Volume Module: Base Vol: 220 313 0 0 172 50 41 0 277 0 0 Initial Bse: 220 313 0 0 172 50 41 0 277 0 0 Λ PHF Volume: 282 401 0 0 221 64 53 0 355 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 221 FinalVolume: 282 401 64 53 0 355 0 0 Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxxx 6.4 xxxx 6.3 xxxxx xxxx xxxxx -----| Capacity Module: Cnflict Vol: 285 xxxx xxxxx xxxx xxxx 1218 xxxx 253 xxxx xxxx xxxxx Volume/Cap: 0.22 xxxx xxxx xxxx xxxx xxxx 0.32 xxxx 0.46 xxxx xxxx xxxx Level Of Service Module: 2Way95thQ: 0.9 xxxx xxxxx xxxx xxxx 1.3 xxxx 2.4 xxxx xxxx xxxxx Control Del: 8.7 xxxx xxxxx xxxxx xxxxx xxxxx 37.3 xxxx 13.4 xxxxx xxxx xxxxx LOS by Move: A * * * * * E * B * * * Movement: LT - LTR - RT Shared LOS: * * * * * * * * * * * * ApproachDel: xxxxx XXXXXX 16.5 XXXXXX ApproachLOS: C ************************************* Note: Queue reported is the number of cars per lane. *******************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************ Intersection #1 Natividad / Rogge Average Delay (sec/veh): 4.7 Worst Case Level Of Service: B[12.3] ************************* Street Name: Natividad Approach: Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Movement: L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Channel Include Lanes: 1 0 1 0 0 0 0 1 0 1 0 0 0 0 0 0 Volume Module: Base Vol: 157 138 0 0 219 51 29 0 151 0 0 Initial Bse: 157 138 0 0 219 51 29 0 151 0 0 PHF Volume: 183 160 0 0 255 59 34 0 176 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 FinalVolume: 183 160 0 0 255 34 0 176 59 0 0 0 -----| Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxxx 6.4 xxxx 6.2 xxxxx xxxx xxxxx Capacity Module: Cnflict Vol: 314 xxxx xxxxx xxxx xxxx xxxx 810 xxxx 284 xxxx xxxx xxxxx Potent Cap:: 1246 xxxx xxxxx xxxx xxxx xxxxx 351 xxxx 757 xxxx xxxx xxxxx Move Cap:: 1246 xxxx xxxxx xxxx xxxx xxxxx 311 xxxx 757 xxxx xxxx xxxxx Volume/Cap: 0.15 xxxx xxxx xxxx xxxx 0.11 xxxx 0.23 xxxx xxxx xxxx Level Of Service Module: 2Way95thQ: 0.5 xxxx xxxxx xxxx xxxxx 0.4 xxxx 0.9 xxxx xxxx xxxxx Control Del: 8.4 xxxx xxxxx xxxxx xxxx xxxxx 18.0 xxxx 11.2 xxxxx xxxx xxxxx LOS by Move: A * * * * * C * B * * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT ApproachDel: xxxxxx 12.3 XXXXXX · * ApproachLOS: Note: Queue reported is the number of cars per lane. *******************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************************** Intersection #1 Natividad / Rogge ***************************** Average Delay (sec/veh): 88.7 Worst Case Level Of Service: F[274.6] ********************* Natividad Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RUncontrolled Uncontrolled Stop Sign Stop Sign Include Include Channel Include Control: Rights: 1 0 1 0 0 0 0 0 1 0 1 0 0 0 1 0 0 0 0 Lanes: Volume Module: Base Vol: 290 360 0 290 0 120 170 0 320 Initial Bse: 290 360 0 0 290 120 170 0 320 0 0 User Adj: PHF Adj: PHF Volume: 358 444 0 0 358 148 210 0 395 0 0 . 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 FinalVolume: 358 444 0 0 358 148 0 210 395 0 ~-----||-----||-----||------| Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxx 6.4 xxxx 6.2 xxxxx xxxx xxxxx FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxx 3.5 xxxx 3.3 xxxxx xxxx xxxxx Capacity Module: Cnflict Vol: 506 xxxx xxxxx xxxx xxxx xxxx 1593 xxxx 432 xxxx xxxx xxxxx Potent Cap.: 1059 xxxx xxxxx xxxx xxxx xxxx 117 xxxx 621 xxxx xxxx xxxxx Move Cap.: 1059 xxxx xxxxx xxxx xxxx xxxx 86 xxxx 621 xxxx xxxx xxxxx Volume/Cap: 0.34 xxxx xxxx xxxx xxxx xxxx 2.43 xxxx 0.64 xxxx xxxx xxxx Level Of Service Module: 2Way95thQ: 1.5 xxxx xxxxx xxxx xxxx xxxxx 19.5 xxxx 4.5 xxxx xxxx xxxxx Control Del: 10.1 xxxx xxxxx xxxxx xxxx xxxxx 753.0 xxxx 20.4 xxxxx xxxx xxxxx B * * * * * F * LOS by Move: С LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT * * * Shared LOS: * * * * * * ApproachDel: 274.6 XXXXXX XXXXXX XXXXXX ApproachLOS: F ******************************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************************* Intersection #1 Natividad / Rogge ******************************** Average Delay (sec/veh): 57.9 Worst Case Level Of Service: F[224.9] **************************** Natividad Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RUncontrolled Uncontrolled Stop Sign Stop Sign Include Include Channel Include Control: Rights: Lanes: 1 0 1 0 0 0 0 0 1 0 1 0 0 0 1 0 0 0 0 _____| Volume Module: Base Vol: 240 575 0 0 315 180 140 300 0 1.00 1.00 1.00 Initial Bse: 240 575 0 0 315 140 0 300 180 0 0 User Adj: PHF Adj: PHF Volume: 282 676 0 0 371 212 165 0 353 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 282 676 0 0 371 212 165 353 0 Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxx 6.4 xxxx 6.3 xxxxx xxxx xxxxx FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxx 3.5 xxxx 3.3 xxxxx xxxx xxxxx Capacity Module: Cnflict Vol: 582 xxxx xxxxx xxxx xxxx xxxx 1718 xxxx 476 XXXX XXXX XXXXX Potent Cap.: 977 xxxx xxxxx xxxx xxxx xxxx 97 xxxx
Move Cap.: 977 xxxx xxxxx xxxx xxxx 75 xxxx 582 xxxx xxxx xxxxx 582 xxxx xxxx xxxxx Volume/Cap: 0.29 xxxx xxxx xxxx xxxx xxxx 2.19 xxxx 0.61 xxxx xxxx xxxx Level Of Service Module: 2Way95thQ: 1.2 xxxx xxxxx xxxx xxxx xxxxx 15.2 xxxx 4.0 xxxx xxxx xxxxx Control Del: 10.2 xxxx xxxxx xxxxx xxxx xxxxx 663.5 xxxx 20.2 xxxxx xxxx xxxxx B * * * * F * LOS by Move: С Movement: LT - LTR - RT Shared LOS: * * * * * * . * * * 224.9 ApproachDel: XXXXXX. XXXXXX XXXXXX ApproachLOS: ******************** Note: Queue reported is the number of cars per lane. **********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************ Intersection #1 Natividad / Rogge *********************************** Average Delay (sec/veh): 13.0 Worst Case Level Of Service: F[59.1] ******************************* Natividad Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-R-----|----|-----| Uncontrolled Uncontrolled Stop Sign Stop Sign Control: Rights: Include Include Channel Include 1 0 1 0 0 0 0 0 1 0 1 0 0 0 1 Lanes: 0 0 0 0 0 -----||-----||-----| Volume Module: Base Vol: 190 300 0 0 460 200 170 110 0 1.00 1.00 1.00 Initial Bse: 190 300 0 0 460 200 110 0 170 0 0 User Adj: 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 PHF Adj: 0.91 0.91 PHF Volume: 209 330 0 0 505 220 121 0 187 0 0 Reduct Vol: 0 0 0 0 . 0 0 0 0 0 = FinalVolume: 209 330 0 0 505 220 0 121 187 0 -----|-----|------| Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx xxxxx xxxx xxxx 6.4 xxxx 6.3 xxxxx xxxx xxxxx FollowUpTim: 2.2 xxxx xxxxx xxxxx xxxx xxxxx 3.5 xxxx 3.3 xxxxx xxxx xxxxx -----|----|-----|-----||------||------| Capacity Module: Cnflict Vol: 725 xxxx xxxxx xxxx xxxx xxxxx 1363 xxxx 615 XXXX XXXX XXXXX Potent Cap.: 864 xxxx xxxxx xxxx xxxx 161 xxxx 485 xxxx xxxx xxxxx Move Cap.: 864 xxxx xxxxx xxxx xxxx xxxx 131 xxxx 485 xxxx xxxx xxxxx Volume/Cap: 0.24 xxxx xxxx xxxx xxxx xxxx 0.93 xxxx 0.38 xxxx xxxx xxxx ------||-----||-----||------| Level Of Service Module: 2Way95thQ: 0.9 xxxx xxxxx xxxx xxxx xxxxx 6.1 xxxx 1.8 xxxx xxxx xxxxx Control Del: 10.5 xxxx xxxxx xxxxx xxxx xxxxx 124.2 xxxx 17.0 xxxxx xxxx xxxxx B * * * * * F * LOS by Move: С Movement: LT - LTR - RT Shared LOS: * * * * * * * * * * * ApproachDel: XXXXXX XXXXXX 59.1 XXXXXX ApproachLOS: * F ******************* Note: Queue reported is the number of cars per lane. ***********************************

APPENDIX D INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

Natividad Road/Boronda Road

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************ Intersection #2 Natividad / Boronda *********************************** Cycle (sec): 100 Critical Vol./Cap.(X): 12 67 Loss Time (sec): Average Delay (sec/veh): 34.0 Optimal Cycle: Level Of Service: Natividad Street Name: Boronda Approach: North Bound South Bound East Bound West Bound L-T-R L-T-R L-T-R Movement: -----|----|-----| Control: Protected Protected Protected Protected Rights: Ovl Include Ovl Ovl 6 6 6 6 6 6 Min. Green: 6 6 4 6 6 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 Lanes: 1 0 1 0 1 1 0 0 1 0 1 0 1 0 1 1 0 0 1 0 -----|----|-----|------| Volume Module: 97 174 Base Vol: 256 164 207 38 32 359 76 184 431 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 97 174 256 164 207 38 32 359 76 184 431 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.86 0.86 0.86 0.86 0.86 PHF Adj: 0.86 0.86 0.86 0.86 0.86 PHF Volume: 113 202 298 191 241 44 37 417 88 214 501 234 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 113 202 298 191 241 37 417 44 88 214 501 234 PCE Adj: 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 113 202 298 191 241 44 37 417 88 214 501 -----| Saturation Flow Module: Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Adjustment: 0.91 0.96 0.82 0.90 0.92 0.92 0.90 0.95 0.81 0.93 0.93 0.93 1.00 1.00 1.00 1.00 0.84 0.16 1.00 1.00 1.00 1.00 0.68 0.32 Lanes: Final Sat.: 1736 1828 1554 1702 1479 272 1718 1809 1537 1769 1209 -----|----|-----||------| Capacity Analysis Module: Vol/Sat: 0.06 0.11 0.19 0.11 0.16 0.16 0.02 0.23 0.06 0.12 0.41 *** Crit Moves: **** *** Green/Cycle: 0.08 0.14 0.35 0.14 0.20 0.20 0.06 0.39 0.47 0.20 0.53 0.68 Volume/Cap: 0.79 0.78 0.55 0.78 0.79 0.79 0.36 0.59 0.12 0.59 0.78 Uniform Del: 45.1 41.3 26.4 41.2 37.8 37.8 45.2 24.2 14.8 36.0 18.6 IncremntDel: 25.8 13.7 1.3 14.5 11.6 11.6 2.2 1.4 0.1 2.6 4.1 0.9 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Delay/Veh: 70.9 55.1 27.7 55.7 49.4 49.4 47.3 25.6 14.9 38.7 22.7 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 70.9 55.1 27.7 55.7 49.4 49.4 47.3 25.6 14.9 38.7 22.7 LOS by Move: E E C E D D D C В D С Α 5 8 8 HCM2kAvgQ: 8 10 10 2 11 2 7 ************************************** Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) *********************************** Intersection #2 Natividad / Boronda ****************************** 100 Cycle (sec): Critical Vol./Cap.(X): 12 90 Loss Time (sec): Average Delay (sec/veh): 40.4 Optimal Cycle: Level Of Service: ******************************* Natividad Street Name: Boronda North Bound South Bound Approach: East Bound West Bound $L - T - R \quad L - T - R$ L - T - R Movement: L - T - R -----| Control: Protected Protected Protected Protected Rights: Ovl Ovl Include Ovl 6 6 Min. Green: 6 6 6 6 6 6 4 6 6 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 1 0 1 0 1 1 0 0 1 0 1 0 1 0 1 Lanes: 1 0 0 1 0 Volume Module: 140 125 399 201 187 30 58 553 101 Base Vol: 254 606 130 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 140 125 399 201 187 30 58 553 101 254 606 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.89 0.89 0.89 0.89 0.89 PHF Adj: 0.89 0.89 0.89 0.89 0.89 0.89 PHF Volume: 157 140 448 226 210 34 65 621 113 285 681 8: O Reduct Vol: 0 0 0 0 0 0 0 0 0 0 O Reduced Vol: 157 140 448 226 210 34 65 621 285 681 113 146 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adi: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 448 FinalVolume: 157 140 226 210 34 65 621 113 285 681 Saturation Flow Module: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Sat/Lane: Adjustment: 0.93 0.98 0.83 0.91 0.94 0.94 0.93 0.98 0.83 0.93 0.95 0.95 1.00 1.00 . 1.00 1.00 0.86 0.14 1.00 1.00 0.82 Lanes: 1.00 1.00 0.18 Final Sat.: 1769 1862 1583 1736 1542 247 1769 1862 1583 1769 1492 Capacity Analysis Module: 0.09 0.08 0.28 0.13 0.14 0.04 0.33 0.07 Vol/Sat: 0.14 0.16 0.46 0.46 **** **** *** Crit Moves: Green/Cycle: 0.12 0.14 0.33 0.15 0.18 0.06 0.40 0.51 0.18 0.19 0.53 0.68 Volume/Cap: 0.77 0.53 0.85 0.86 0.77 0.77 0.61 0.84 0.14 0.84 0.86 0.67 39.3 Uniform Del: 43.0 39.9 31.0 41.5 39.3 45.9 27.3 12.8 38.9 20.5 9.5 IncremntDel: 16.5 2.1 12.5 24.4 11.2 10.3 8.6 11.2 0.1 17.0 8.2 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Delay Adj: 1.00 1.00 1.00 50.4 56.1 35.9 Delay/Veh: 59.5 42.0 43.6 65.9 50.4 12.9 55.9 28.7 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 43.6 65.9 50.4 AdjDel/Veh: 59.5 42.0 50.4 56.1 35.9 12.9 55.9 28.7 11.0 D LOS by Move: E D D \mathbf{E} D E В E C D В 7 5 9 3 16 10 9 2 HCM2kAvqQ: 20 11 ****************************** Note: Queue reported is the number of cars per lane. ****************************** ______

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) Intersection #2 Natividad / Boronda 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 12 Optimal Cycle: 81 Average Delay (sec/veh): 40.1 Level Of Service: *********************************** Natividad Street Name: Boronda Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R L - T - R Movement: L - T - R Control: Protected Protected Protected Protected Rights: Ovl Include Ovl Ovl 6 6 6 6 6 6 6 6 6 6 Min. Green: 4 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 1 0 1 0 1 Lanes: 1 0 1 0 1 1 0 0 1 0 1 0 0 1 0 -----| Volume Module: Base Vol: 109 124 474 247 139 30 13 569 108 273 608 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 109 124 474 247 139 13 569 30 108 273 608 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 0.98 0.98 0.98 0.98 0.98 PHF Adj: 0.98 0.98 0.98 0.98 0.98 0.98 484 PHF Volume: 111 127 252 142 31 13 581 110 279 620 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 n Reduced Vol: 111 127 484 252 142 31 13 581 110 279 620 148 PCE Adj: 1.00 MLF Adj: 484 110 FinalVolume: 111 127 252 142 31 13 581 279 620 -----| Saturation Flow Module: . Adjustment: 0.94 0.99 0.84 0.92 0.94 0.94 0.93 0.98 0.83 0.94 0.96 0.96 1.00 1.00 1.00 1.00 0.82 0.18 1.00 1.00 1.00 1.00 0.81 0.19 Lanes: Final Sat.: 1787 1881 1599 1753 1476 319 1769 1862 1583 1787 1475 Capacity Analysis Module: Vol/Sat: 0.06 0.07 0.30 0.14 0.10 0.10 0.01 0.31 0.07 0.16 0.42 0.42 **** *** Crit Moves: **** Green/Cycle: 0.13 0.17 0.35 0.17 0.20 0.20 0.06 0.36 0.50 0.18 0.49 0.65 0.87 0.47 0.47 0.12 0.86 0.14 Volume/Cap: 0.47 0.40 0.86 0.86 0.87 44.5 29.4 13.7 Uniform Del: 40.2 37.0 30.2 40.6 35.1 35.1 39.7 22.9 10.5 IncremntDel: 1.5 0.8 13.0 23.0 1.0 1.0 0.5 10.6 0.1 19.8 9.1 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adi: 41.7 37.8 43.2 63.6 36.1 36.1 45.0 40.1 13.8 59.4 32.0 11.8 Delay/Veh: User DelAdj: 1.00 1.00 1.00 AdjDel/Veh: 41.7 37.8 43.2 63.6 36.1 36.1 45.0 40.1 13.8 59.4 32.0 11.8 D D В LOS by Move: D D D - E D D E С □ B 4 17 5 4 11 5 0 19 2 HCM2kAvqQ: 11 *********************************** Note: Queue reported is the number of cars per lane. *************************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************************ Intersection #2 Natividad / Boronda 100 Cycle (sec): Critical Vol./Cap.(X): 12 85 Loss Time (sec): Average Delay (sec/veh): 37.5 Optimal Cycle: Level Of Service: ************************************* Natividad Street Name: Boronda North Bound South Bound East Bound Approach: West Bound L-T-R L-T-R L-T-R Movement: -----|----|-----|------| Protected Control: Protected Protected Protected Rights: Ovl Include Ovl Ovl 6 6 6 6 Min. Green: 6 6 6 6 4 6 6 6 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 Lanes: 1 0 1 0 1 1 0 0 1 0 1 0 1 0 1 1 0 0 1 0 -----| Volume Module: Base Vol: 97 204 256 164 227 54 56 359 76 184 431 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 97 204 164 227 256 56 359 54 76 184 431 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 0.83 PHF Volume: 117 246 308 198 273 65 67 433 92 222 519 242 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 117 246 308 198 273 65 67 433 92 222 519 242 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 198 273 65 FinalVolume: 117 246 308 67 433 92 222 519 -----|----|-----|-----| Saturation Flow Module: Adjustment: 0.91 0.96 0.82 0.90 0.92 0.92 0.90 0.95 0.81 0.93 0.93 0.93 1.00 1.00 1.00 1.00 0.81 0.19 1.00 1.00 1.00 1.00 0.68 0.32 Lanes: Final Sat.: 1736 1828 1554 1702 1405 334 1718 1809 1537 1769 1209 564 -----| Capacity Analysis Module: 0.12 0.19 0.19 0.04 0.24 0.06 0.13 0.43 Vol/Sat: 0.07 0.13 0.20 0.43 Crit Moves: **** *** *** Green/Cycle: 0.08 0.17 0.36 0.14 0.23 0.23 0.06 0.37 0.45 0.20 0.51 0.65 Volume/Cap: 0.84 0.81 0.55 0.81 0.84 0.13 0.64 0.84 0.66 0.84 0.65 0.64 Uniform Del: 45.4 40.1 25.4 41.5 36.7 36.7 46.0 25.8 15.9 37.0 21.1 IncremntDel: 35.0 14.6 1.1 17.6 14.9 14.9 14.2 2.1 0.1 4.0 7.3 1.4 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.00 1.00 1.00 1.00 1.00 1.00 Delay Adj: 1.00 1.00 1.00 1.00 1.00 Delay/Veh: 80.4 54.7 26.5 59.1 51.6 51.6 60.1 27.9 16.0 41.0 28.4 11.9 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 80.4 54.7 26.5 59.1 51.6 51.6 60.1 27.9 16.0 41.0 28.4 11.9 LOS by Move: F D С E D D E С B D C 6 10 8 8 13 13 HCM2kAvqQ: 3 12 2 ****************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ********************************** Intersection #2 Natividad / Boronda ****************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 12 Optimal Cycle: 95 Average Delay (sec/veh): 43.2 Level Of Service: ****************************** Natividad Street Name: Boronda Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RProtected Protected Control: Protected Protected Rights: Ovl Include Ovl Ov16 6 6 6 6 6 6 6 6 6 Min. Green: 4 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 Lanes: 1 0 1 0 1 1 0 0 1 0 1 0 1 0 1 1 0 0 1 0 Volume Module: 140 137 399 201 205 Base Vol: 44 68 553 101 254 606 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 68 553 Initial Bse: 140 137 201 205 399 44 101 254 606 User Adj: PHF Adj: PHF Volume: 159 156 453 228 233 50 77 628 115 289 689 0 Reduct Vol: 0 0 0 0 0 0 0 0 -0 0 0 Reduced Vol: 159 156 77 628 50 453 228 233 115 289 689 148 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adi: 1.00 1.00 1.00 1.00 1.00 50 77 628 115 FinalVolume: 159 156 453 228 233 289 689 _____ Saturation Flow Module: Adjustment: 0.93 0.98 0.83 0.91 0.94 0.94 0.93 0.98 0.83 0.93 0.95 0.95 1.00 1.00 1.00 1.00 0.82 0.18 1.00 1.00 1.00 1.00 0.82 0.18 Lanes: Final Sat.: 1769 1862 1583 1736 1464 314 1769 1862 1583 1769 1492 Capacity Analysis Module: Vol/Sat: 0.09 0.08 0.29 0.13 0.16 0.16 0.04 0.34 0.07 0.16 0.46 0.46 **** **** Crit Moves: **** Green/Cycle: 0.11 0.14 0.33 0.15 0.19 0.19 0.06 0.40 0.50 0.19 0.53 0.68 Volume/Cap: 0.85 0.59 0.86 0.87 0.85 0.85 0.73 0.85 0.14 0.85 0.87 Uniform Del: 44.0 40.2 31.2 41.5 39.4 39.4 46.2 27.5 13.4 39.0 20.7 IncremntDel: 29.8 3.6 13.5 26.1 18.9 18.9 22.3 9.3 0.1 18.2 9.0 1.6 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 Delay/Veh: 73.7 43.8 44.7 67.6 58.3 58.3 68.5 36.8 13.5 57.2 29.6 11.1 AdjDel/Veh: 73.7 43.8 44.7 67.6 58.3 58.3 68.5 36.8 13.5 57.2 29.6 11.1 В LOS by Move: E D D E E E E D \mathbf{E} С В 7 5 16 10 11 11 4 2 HCM2kAvgQ: 20 11 26 Note: Queue reported is the number of cars per lane. **************************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) Intersection #2 Natividad / Boronda ******************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 12 Optimal Cycle: 84 Average Delay (sec/veh): 40.9 Level Of Service: ******************************** Natividad Street Name: Boronda Approach: North Bound South Bound East Bound West Bound L-T-R L-T-R L-T-R Movement: L - T - R Protected Protected Control: Protected Protected Rights: Ovl Include Ovl Ovl 6 6 6 6 6 6 6 6 Min. Green: 4 6 6 6 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 Lanes: 1 0 1 0 1 1 0 0 1 0 1 0 1 0 1 1 0 0 1 0 -----||-----||-----||------| Volume Module: 109 131 474 Base Vol: 247 148 37 19 569 108 273 608 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 109 131 474 247 148 37 19 569 108 273 608 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 PHF Volume: 112 135 489 255 153 38 20 587 111 281 627 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 n Reduced Vol: 112 135 489 255 153 38 20 587 111 281 627 149 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 FinalVolume: 112 135 489 255 153 38 20 587 111 281 627 Saturation Flow Module: Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Adjustment: 0.94 0.99 0.84 0.92 0.94 0.94 0.93 0.98 0.83 0.94 0.96 0.96 1.00 1.00 1.00 1.00 0.80 0.20 1.00 1.00 1.00 0.81 0.19 Lanes: Final Sat.: 1787 1881 1599 1753 1432 358 1769 1862 1583 1787 1475 _____| | | Capacity Analysis Module: Vol/Sat: 0.06 0.07 0.31 0.15 0.11 0.11 0.01 0.32 0.07 0.16 0.43 0.43 **** **** Crit Moves: **** Green/Cycle: 0.12 0.17 0.35 0.17 0.21 0.21 0.06 0.36 0.49 0.18 0.49 0.65 Volume/Cap: 0.51 0.42 0.87 0.88 0.51 0.51 0.18 0.87 0.14 0.87 0.88 0.65 Uniform Del: 40.9 37.2 30.4 40.7 34.9 34.9 44.7 29.6 14.1 39.7 23.1 10.6 IncremntDel: 1.9 0.9 13.9 24.4 1.1 1.1 0.8 11.4 0.1 21.0 9.8 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 65.1 36.0 36.0 45.5 41.0 14.2 60.8 32.9 11.9 Delay/Veh: 42.8 38.1 44.3 1.00 1.00 1.00 AdjDel/Veh: 42.8 38.1 44.3 65.1 36.0 36.0 45.5 41.0 14.2 60.8 32.9 11.9 D LOS by Move: D D D E D D D В E C В 4 4 17 11 6 6 1 2 HCM2kAvgQ: 20 11 *********************************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************************ Intersection #2 Natividad / Boronda *********************************** Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 12 Optimal Cycle: 180 Average Delay (sec/veh): 483.2 Level Of Service: F ******************************** Street Name: Natividad Boronda South Bound Approach: North Bound East Bound West Bound L - T - R L - T - R L - T - RMovement: L - T - R Protected Protected Protected Control: Protected Rights: Ovl Include Ovl Ovl 6 Min. Green: 6 6 6 6 6 6 6 4 6 6 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 0 1 0 1 0 1 0 1 1 0 1 0 1 1 0 0 1 0 Volume Module: Base Vol: 450 700 350 200 660 200 200 1100 250 200 1800 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 450 700 350 200 660 200 200 1100 250 200 1800 User Adj: PHF Adj: 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 PHF Volume: 495 769 385 220 725 220 220 1209 275 220 1978 220 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 495 769 385 220 725 275 220 220 1209 220 1978 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 385 275 FinalVolume: 495 769 220 725 220 220 1209 220 1978 Saturation Flow Module: 1900 1900 1900 Adjustment: 0.91 0.96 0.82 0.90 0.91 0.91 0.90 0.95 0.81 0.93 0.97 0.97 1.00 1.00 1.00 1.00 0.77 0.23 1.00 1.00 1.00 1.00 0.90 0.10 Lanes: Final Sat.: 1736 1828 1554 1702 1327 402 1718 1809 1537 1769 1651 Capacity Analysis Module: Vol/Sat: 0.28 0.42 0.25 0.13 0.55 0.55 0.13 0.67 0.18 0.12 1.20 1.20 **** **** Crit Moves: Green/Cycle: 0.12 0.26 0.34 0.08 0.22 0.22 0.06 0.46 0.57 0.09 0.48 0.56 Volume/Cap: 2.48 1.64 0.72 1.64 2.48 2.48 2.13 1.46 0.31 1.46 2.48 Uniform Del: 44.2 37.1 28.7 46.1 39.0 39.0 47.0 27.1 11.1 45.7 25.8 21.9 IncremntDel:678.6 296 4.9 317.9 672 671.7 540.3 212 0.2 238.3 667 511.1 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay/Veh: 722.9 334 33.6 364.0 711 710.6 587.3 239 11.3 284.0 693 533.0 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 722.9 334 33.6 364.0 711 710.6 587.3 239 11.3 284.0 693 533.0 LOS by Move: F F С F F F F F В F HCM2kAvqQ: 52 61 12 19 98 98 22 84 17 226 *************************** Note: Queue reported is the number of cars per lane. *********************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************* Intersection #2 Natividad / Boronda ********************* Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 12 Optimal Cycle: 180 Average Delay (sec/veh): 624.3 Level Of Service: ******************************* Street Name: Natividad Boronda North Bound South Bound East Bound Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R Movement: Protected Protected Protected Control: Protected Rights: Ovl Include Ovl Ovl 6 6 6 Min. Green: - 6 6 6 6 6 4 6 6 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 1 0 1 1 0 1 0 1 1 0 0 1 0 1 0 0 1 0 Volume Module: 250 Base Vol: 480 670 300 1010 170 330 1710 345 160 1870 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 300 1010 170 330 1710 Initial Bse: 480 670 250 345 160 1870 User Adj: 1.00 1.00 1.00 PHF Adj: PHF Volume: 522 728 272 326 1098 185 359 1859 375 174 2033 207 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 522 728 326 1098 359 1859 375 272 185 174 2033 207 PCE Adj: MLF Adj: 1.00 1.00 1.00 272 FinalVolume: 522 728 326 1098 185 359 1859 375 174 2033 Saturation Flow Module: 1900 1900 1900 1900 1900 1900 1900 1900 1900 Sat/Lane: 1900 1900 1900 Adjustment: 0.93 0.98 0.83 0.91 0.94 0.94 0.93 0.98 0.83 0.93 0.97 0.97 1.00 1.00 1.00 1.00 0.86 0.14 1.00 1.00 1.00 1.00 0.91 0.09 Lanes: Final Sat.: 1769 1862 1583 1736 1530 -258 1769 1862 1583 1769 1667 -----||-----||-----||------| Capacity Analysis Module: Vol/Sat: 0.29 0.39 0.17 0.19 0.72 0.72 0.20 1.00 0.24 0.10 1.22 1.22 Crit Moves: **** Green/Cycle: 0.11 0.25 0.29 0.12 0.26 0.26 0.07 0.47 0.57 0.05 0.44 0.56 Volume/Cap: 2.77 1.58 0.59 1.58 2.77 2.77 2.77 2.13 0.41 2.13 2.77 Uniform Del: 44.7 37.6 30.1 44.1 37.0 37.0 46.3 26.6 11.9 47.7 28.0 22.0 IncremntDel:809.8 272 1.9 283.8 801 801.1 816.3 514 0.3 548.5 799 534.2 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Delay Adj: 1.00 1.00 1.00 1.00 Delay/Veh: 854.4 310 32.1 327.9 838 838.2 862.6 540 12.2 596.2 827 556.2 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 854.4 310 32.1 327.9 838 838.2 862.6 540 12.2 596.2 827 556.2 LOS by Move: F F С F F $\mathbf{F} = \mathbf{F}$ В F F F 56 8 26 140 140 40 177 7 HCM2kAvqQ: 58 18 242 ********************************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) Intersection #2 Natividad / Boronda ************************************* Cycle (sec): 100
Loss Time (sec): 12
Optimal Cycle: 180 Critical Vol./Cap.(X): Average Delay (sec/veh): Level Of Service: F **************************** Natividad Boronda North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: Protected Protected Protected Protected Rights: Ovl Include Ovl Ovl 6 6 6 6 6 6 6 6 Min. Green: 4 - 6 6 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 1 0 1 1 0 0 1 0 1 0 1 0 1 1 0 0 1 0 Volume Module: Base Vol: 400 740 200 270 900 170 280 2100 400 300 1500 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 400 740 200 270 900 280 2100 300 1500 170 400 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 0.96 0.96 0.96 PHF Adj: 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 281 938 177 PHF Volume: 417 771 208 417 292 2188 313 1563 0 - 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 208 Reduced Vol: 417 771 281 938 177 292 2188 417 313 1563 292 PCE Adj: MLF Adj: FinalVolume: 417 771 281 938 208 177 292 2188 417 313 1563 Saturation Flow Module: 1900 1900 1900 1900 1900 1900 1900 Sat/Lane: 1900 1900 1900 1900 Adjustment: 0.93 0.98 0.83 0.91 0.94 0.94 0.93 0.98 0.83 0.93 0.96 0.96 Lanes: 1.00 1.00 1.00 1.00 0.84 0.16 1.00 1.00 1.00 1.00 0.84 0.16 Final Sat.: 1769 1862 1583 1736 1501 283 1769 1862 1583 1769 1531 Capacity Analysis Module: 0.24 0.41 0.13 0.16 0.62 0.62 0.16 1.17 Vol/Sat: 0.26 0.18 1.02 1.02 Crit Moves: **** **** Green/Cycle: 0.09 0.25 0.32 0.10 0.25 0.25 0.07 0.47 0.56 0.07 0.46 0.56 Volume/Cap: 2.51 1.68 0.42 1.68 2.51 2.51 2.20 2.51 0.47 2.51 2.20 Uniform Del: 45.3 37.7 26.9 45.2 37.6 37.6 46.3 26.6 13.1 46.5 26.9 22.0 IncremntDel:698.6 317 0.6 331.8 688 687.7 565.7 684 0.4 704.3 546 375.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 1.00 Delay/Veh: 743.9 354 27.5 376.9 725 725.3 612.0 711 13.5 750.8 573 397.0 AdjDel/Veh: 743.9 354 27.5 376.9 725 725.3 612.0 711 13.5 750.8 573 397.0 LOS by Move: F В F HCM2kAvqQ: 34 180 Note: Queue reported is the number of cars per lane.

APPENDIX E INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

San Juan Grade Road/Boronda Road

```
Level Of Service Computation Report
        2000 HCM Operations Method (Base Volume Alternative)
*************************************
Intersection #3 San Juan Grade / Boronda
**********************************
             100
Cycle (sec):
                         Critical Vol./Cap.(X):
          12
45
Loss Time (sec):
                         Average Delay (sec/veh):
Optimal Cycle:
                         Level Of Service:
San Juan Grade
Street Name:
                                     Boronda
Approach: North Bound South Bound East South Movement: L - T - R L - T - R L - T - R
----|
                              Protected
Control:
         Protected
                   Protected
                                          Protected
          Include
                    Include
                               Include
                                          Include
                  6 6 6 6 6 6 6 6 6
4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0
       6 6 6
Min. Green:
         4.0 4.0 4.0
Y+R:
         2 0 2 0 1 2 0 2 0 1 2 0 1 1 0
Lanes:
                                         2 0 3 0 1
-----|
Volume Module:
        61 157
              23
Base Vol:
                  361 167
                         305
                               70 409
                                     39
                                         23 581
Growth Adj: 1.00 1.00 1.00 1.00 1.00
                             1.00 1.00 1.00 1.00 1.00 1.00
Initial Bse: 61 157
                   361 167
               23
                          305
                             70 409
                                    39
                                         23 581
       1.00 1.00 1.00 1.00 1.00 1.00
User Adj:
                             1.00 1.00 1.00 1.00 1.00 1.00
PHF Adj: 0.87 0.87 0.87 0.87 0.87
                             0.87 0.87 0.87 0.87 0.87
PHF Volume: 70 180 26
                  415 192
                         351
                             80 470
                                    45
                                         26 668
Reduct Vol:
         0 0
                0
                   0 0
                          0
                               0 0 0
                                           0
                                         0
                                                 0
Reduced Vol: 70 180
                   415 192
                26
                         351
                               80 470
                                    45
                                         26 668
                                                349
        PCE Adj:
                                               1.00
MLF Adi:
        1.00 1.00 1.00
FinalVolume: 70 180 26 415 192 351
                             80 470 45
                                         26 668
-----|
Saturation Flow Module:
Adjustment: 0.88 0.90 0.81 0.88 0.90 0.81 0.86 0.88 0.88 0.89 0.88 0.83
Lanes: 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.83 0.17 2.00 3.00 1.00
Final Sat.: 3334 3437 1537 3334 3437 1537 3274 3041 290 3400 5037 1568
-----|
Capacity Analysis Module:
Vol/Sat: 0.02 0.05 0.02 0.12 0.06 0.23 0.02 0.15 0.15 0.01 0.13 0.22
Crit Moves: ****
                         ****
                             ***
Green/Cycle: 0.06 0.14 0.14 0.30 0.38 0.38 0.06 0.31 0.31 0.12 0.38 0.38
Volume/Cap: 0.35 0.36 0.12 0.42 0.15 0.59 0.41 0.49 0.49 0.06 0.35 0.59
Uniform Del: 45.1 38.6 37.2 28.0 20.1 24.5
                             45.3 27.8 27.8 38.9 22.5 25.1
IncremntDel: 1.1 0.5
              0.2
                  0.3 0.1
                         1.6
                             1.4 0.4
                                    0.4
                                        0.1 0.1
InitQueuDel: 0.0 0.0
              0.0
                   0.0 0.0
                         0.0
                             0.0 0.0
                                        0.0 0.0
                                    0.0
                                               0.0
AdjDel/Veh: 46.2 39.1 37.5 28.3 20.1 26.2 46.7 28.2 28.2 38.9 22.6 26.7
              D C C
LOS by Move: D D
                         С
                             D C
                                    С
                                        D C
                                                С
         1
             3
                1
                   5
HCM2kAvqQ:
                       2 9
                               2
                                 7
                                     7
                                         0
******************************
Note: Queue reported is the number of cars per lane.
************************
```

Exist MD Fri May 27, 2011 15:45:08 Page 5-1 ______ Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************** Intersection #3 San Juan Grade / Boronda ************************ 100 Critical Vol./Cap.(X): Cycle (sec): 12 40 Average Delay (sec/veh): 28.1 Loss Time (sec): Optimal Cycle: Level Of Service: ************************** San Juan Grade Boronda Street Name: North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: _____| Protected Protected Protected Control: Protected Include Rights: Include Include Include 6 6 6 6 6 6 6 6 6 6 6 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 2 0 2 0 1 2 0 2 0 1 2 0 1 1 0 2 0 3 0 1 Lanes: _____| | | Volume Module: Base Vol: 157 274 77 273 204 161 82 697 63 88 554 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 77 273 204 88 554 Initial Bse: 157 274 82 697 161 63 238 User Adj: 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 PHF Adj: PHF Volume: 167 291 82 290 217 87 741 94 589 171 67 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 167 291 82 290 217 171 87 741 67 94 589 253 PCE Adj: MLF Adj: 171 67 87 741 FinalVolume: 167 291 82 290 217 94 589 -----|----|-----|------| Saturation Flow Module: Adjustment: 0.90 0.93 0.83 0.89 0.92 0.83 0.89 0.91 0.91 0.90 0.89 0.83 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.83 0.17 2.00 3.00 1.00 Lanes: Final Sat.: 3432 3538 1583 3400 3505 1568 3400 3176 287 3432 5083 1583 _____| Capacity Analysis Module: Vol/Sat: 0.05 0.08 0.05 0.09 0.06 0.11 0.03 0.23 0.23 0.03 0.12 0.16 *** **** Crit Moves: Green/Cycle: 0.12 0.17 0.17 0.17 0.22 0.22 0.15 0.48 0.48 0.06 0.39 0.39 0.49 0.28 0.49 0.18 0.49 0.49 Volume/Cap: 0.40 0.49 0.31 0.45 0.30 0.41 Uniform Del: 40.6 37.7 37.3 32.3 34.0 37.4 17.8 17.8 45.4 21.0 22.1 36.5 IncremntDel: 0.6 0.6 0.7 0.6 0.2 1.1 0.2 0.2 0.2 1.6 0.1 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Delay/Veh: 41.2 38.3 37.1 37.9 32.5 35.1 37.6 18.1 18.1 47.0 21.1 22.5 AdjDel/Veh: 41.2 38.3 37.1 37.9 32.5 35.1 37.6 18.1 18.1 47.0 21.1 22.5 LOS by Move: D D D D C D D B В D C С 3 5 2 3 5 9 9 2 5 1 HCM2kAvqQ:

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************** Intersection #3 San Juan Grade / Boronda ********************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 12 Average Delay (sec/veh): 40 Level Of Service: Optimal Cycle: ************************* San Juan Grade Boronda Street Name: North Bound South Bound East Bound L - T - R L - T - R Approach: West Bound L - T - R Movement: Protected Protected Protected Protected Control: Rights: Include Include Include Include 6 6 6 6 6 6 6 6 6 6 6 Min. Green: 6 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 2 0 2 0 1 2 0 3 0 1 Lanes: 2 0 2 0 1 2 0 1 1 0 _____ Volume Module: 168 254 81 229 180 134 120 812 75 Base Vol: 107 491 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 75 Initial Bse: 168 254 81 229 180 120 812 134 107 491 241 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 0.97 PHF Volume: 173 262 77 84 236 186 138 124 837 110 506 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 124 837 77 Reduced Vol: 173 262 84 236 186 138 110 506 248 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 138 77 FinalVolume: 173 262 84 236 186 124 837 110 506 _____| | | Saturation Flow Module: Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Adjustment: 0.91 0.94 0.84 0.89 0.92 0.83 0.90 0.92 0.92 0.90 0.89 0.83 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.83 0.17 2.00 3.00 1.00 Lanes: 295 3432 5083 1583 Final Sat.: 3467 3574 1599 3400 3505 1568 3432 3197 -----||-----| Capacity Analysis Module: Vol/Sat: 0.05 0.07 0.05 0.07 0.05 0.09 0.04 0.26 0.26 0.03 0.10 0.16 Crit Moves: **** **** **** 0.18 0.17 0.53 0.53 0.07 0.43 0.43 Green/Cycle: 0.10 0.14 0.14 0.14 0.18 0.51 0.30 0.49 0.22 0.49 Volume/Cap: 0.49 0.51 0.36 0.49 0.49 0.23 0.36 Uniform Del: 42.5 39.5 38.6 40.0 35.5 36.9 36.1 14.8 14.8 45.1 17.8 19.1 IncremntDel: 1.1 0.8 1.0 0.9 0.3 1.3 0.2 0.2 0.2 1.7 0.1 0.3 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 43.5 40.3 39.6 41.0 35.8 38.3 36.3 15.0 15.0 46.8 17.9 19.4 Delay/Veh: AdjDel/Veh: 43.5 40.3 39.6 41.0 35.8 38.3 36.3 15.0 15.0 46.8 17.9 19.4 В D D D LOS by Move: D D D D В D В В 3 5 3 3 9 9 4 4 2 2 HCM2kAvgQ: ********************

Note: Queue reported is the number of cars per lane.

______ Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ********************* Intersection #3 San Juan Grade / Boronda Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 12
Optimal Cycle: 55 Average Delay (sec/veh): 28.5 Level Of Service: **************************** Street Name: San Juan Grade Boronda Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Protected Protected Protected Protected Include Include Include Include Control: Rights: 6 6 6 6 6 6 6 4.0 4.0 4.0 4.0 4.0 Min. Green: 6 6 6 6 6 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 2 0 2 0 1 2 0 2 0 1 2 0 1 1 0 Lanes: 2 0 3 0 1 Volume Module: 61 172 23 Base Vol: 408 177 76 409 39 309 23 581 375 Initial Bse: 61 172 408 177 23 309 76 409 = 39 23 581 PHF Volume: 74 210 28 498 216 377 93 499 48 28 709 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 Ω Reduced Vol: 74 210 28 93 499 48 498 216 377 28 709 457 MLF Adi: 1.00 FinalVolume: 74 210 28 498 216 377 93 499 48 28 709 Saturation Flow Module: Adjustment: 0.88 0.90 0.81 0.88 0.90 0.81 0.86 0.88 0.89 0.88 0.83 Lanes: 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.83 0.17 2.00 3.00 1.00 Final Sat.: 3334 3437 1537 3334 3437 1537 3274 3041 290 3400 5037 1568 Capacity Analysis Module: Vol/Sat: 0.02 0.06 0.02 0.15 0.06 0.25 0.03 0.16 0.16 0.01 0.14 0.29 Crit Moves: **** **** *** Green/Cycle: 0.06 0.12 0.12 0.29 0.35 0.35 0.06 0.35 0.35 0.13 0.41 0.41 Volume/Cap: 0.37 0.52 0.15 0.52 0.18 0.71 0.47 0.47 0.47 0.07 0.34 0.71 Uniform Del: 45.2 41.4 39.6 29.7 22.7 28.2 45.5 25.6 25.6 38.5 20.1 24.3 IncremntDel: 1.2 1.2 0.4 0.5 0.1 4.3 1.8 0.3 0.3 - 0.1 0.1 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 LOS by Move: D D C C C D C C D C С 7 2 11 2 4 1 2 7 HCM2kAvgQ: 7 ************************** Note: Queue reported is the number of cars per lane. *****************************

```
Level Of Service Computation Report
        2000 HCM Operations Method (Base Volume Alternative)
******************
Intersection #3 San Juan Grade / Boronda
**********************
            100
                        Critical Vol./Cap.(X):
Cycle (sec):
Loss Time (sec): 12
Optimal Cycle: 42
                        Average Delay (sec/veh):
                        Level Of Service:
**************************
          San Juan Grade
                                    Boronda
Street Name:
Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-R
Protected Protected Protected Protected Include Include Include
Control:
Rights:
        6 6 6 6 6 6 6 6 6 6 6 6 6 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4
        6 6 6
Min. Green:
                            4.0 4.0 4.0 4.0 4.0
        \begin{smallmatrix}2&0&2&0&1&&2&0&2&0&1&&2&0&1&1&0&&2&0&3&0&1\end{smallmatrix}
Lanes:
Volume Module:
       157 280 77
                  317 213
Base Vol:
                         165
                             85 697
                                    63
                                        88 554
77
                  317 213
                             85 697
                                       88 554
Initial Bse: 157 280
                         165
                                   63
345 232
PHF Volume: 171 304 84
                         179 92 758 68
                                       96 602
Reduct Vol: 0 0
               0
                   0 0
                         0
                              0 0
                                    0
                                        0 0
Reduced Vol: 171 304
               84
                  345 232
                         179
                            92 758
                                   68
                                        96 602
                                              291
       PCE Adj:
MLF Adj:
                                   68
                                        96 602
FinalVolume: 171 304 84 345 232 179
                            92 758
_____|__|__|
Saturation Flow Module:
Adjustment: 0.90 0.93 0.83 0.89 0.92 0.83 0.89 0.91 0.91 0.90 0.89 0.83
Lanes: 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.83 0.17 2.00 3.00 1.00 Final Sat.: 3432 3538 1583 3400 3505 1568 3400 3176 287 3432 5083 1583
_____|
Capacity Analysis Module:
Vol/Sat: 0.05 0.09 0.05 0.10 0.07 0.11 0.03 0.24 0.24 0.03 0.12 0.18
         ****
                  ***
Crit Moves:
Green/Cycle: 0.12 0.17 0.17 0.20 0.24 0.24 0.13 0.46 0.46 0.06 0.39 0.39
                  0.52 0.28 0.48 0.21 0.52 0.52 0.46 0.30 0.47
Volume/Cap: 0.40 0.52 0.32
Uniform Del: 40.4 38.1 36.8 36.0 31.2 32.9 39.1 19.2 19.2 45.4 21.0 22.7
IncremntDel: 0.6 0.8
              0.7
                  0.7 0.2
                         1.0
                            0.2 0.3
                                   0.3
                                       1.7 0.1
InitQueuDel: 0.0 0.0
               0.0
                  0.0 0.0
                         0.0
                            0.0 0.0
                                   0.0
                                       0.0 0.0
Delay Adj: 1.00 1.00 1.00
                  41.0 38.9 37.5 36.8 31.4 33.9 39.4 19.5 19.5 47.1 21.1 23.2
Delay/Veh:
AdjDel/Veh: 41.0 38.9 37.5 36.8 31.4 33.9 39.4 19.5 19.5 47.1 21.1 23.2
              D
                  D C
                         С
                             D B
LOS by Move: D D
                                   В
                                       D C
                                                C
                3
                    5
                      3
                         5
                                         2
         3 5
                              1
                                 10
                                    10
HCM2kAvqQ:
************************
Note: Queue reported is the number of cars per lane.
**********************
```

Level Of Service Computation Report													
2000 HCM Operations Method (Base Volume Alternative)													

Intersection #3 San Juan Grade / Boronda													

Cycle (sec): 100						Critical Vol./Cap.(X): 0.516							
Loss Time (sec): 12						Average Delay (sec/veh): 27.3							
Optimal Cycle: 42 Level Of Service: C													

Street Name: San Juan Grade						Boronda							
Approach:					ound East Bound				West Bound				
Movement:								- R		Т			
Control:	•			•			•	cotect				•	
	Protected			Protected Include			FI			Protected Include			
Rights:		nclude						Incl		_			
Min. Green:			6		6	6		6	6		6	6	
		4.0		4.0			4.0			4.0		.4.0	
Lanes:			1		2	0 1	2 (1	1 0	2 () 3	0 1	
Volume Module	e:												
Base Vol:	168	258	81	251	184	136	121	812	75	107	491	258	
Growth Adj:	1.00 1	.00 1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:		258	81	251	184	136	121	812		107	491	258	
User Adj:	1.00 1		1.00	1.00		1.00		1.00		1.00		1.00	
PHF Adj:	0.95 0).95	0.95		0.95		0.95	0.95	0.95		0.95	
PHF Volume:		.93 272	85	264	194	143	127	855	79	113	517	272	
Reduct Vol:	0		0	0	0	0	0	0	0	0	0	0	
Reduced Vol:		272	85	264	194	143	127	855	79	113	517	272	
PCE Adj:	1.00 1		1.00	1.00		1.00		1.00	1.00	1.00		1.00	
MLF Adj:	1.00 1	.00 1	L.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	
FinalVolume:	177	272	85	264	194	143	127	855	79	113	517	272	
Saturation F	low Mod	ule:											
Sat/Lane:	1900 1	900 1	L900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Adjustment:	0.91 0		0.84	0.89		0.83		0.92		0.90		0.83	
Lanes:	2.00 2		1.00		2.00	1.00		1.83		2.00		1.00	
Final Sat.:	3467 3		L599		3505	1568		3197	295	3432		1583	
rinar bat	1	J/4]		1						1	3003	1	
Consoit: Ano	leraia M										_		
Capacity Anal				0 00	0 00	0 00	0 04	0 07	0 07	0 00	0 10	0 17	
Vol/Sat:	0.05 0		0.05		0.06	0.09	0.04	0.27	0.27	0.03	0.10	0.17	
Crit Moves:		***		****				****		****			
Green/Cycle:).15	0.15		0.18		0.52		0.06		0.43	
Volume/Cap:	0.43 0	.52 (36	0.52	0.31	0.51	0.25	0.52	0.52	0.52		0.40	
Uniform Del:	41.0 3	9.3	38.4	39.1	35.6	37.0	37.4	15.8	15.8	45.3	18.0	19.5	
IncremntDel:	0.7	0.9	1.0	0.9	0.3	1.5	0.2	0.3	0.3	2.1	0.1	0.4	
InitQueuDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Delay Adj:	1.00 1		1.00		1.00	1.00		1.00		1.00		1.00	
Delay/Veh:	41.7 4		39.4		35.9	38.5		16.1		47.5		19.9	
User DelAdj:			1.00		1.00	1.00		1.00			1.00	1.00	
AdjDel/Veh:	41.7 4		39.4		35.9	38.5		16.1	16.1		18.1	19.9	
							37.7 D						
LOS by Move:	. D	D	D D	D	D	D		B		D	B	В	
HCM2kAvgQ:	3	5	3	4	3	5	2	10	10	2	4	- 6	

Note: Queue reported is the number of cars per lane. ************************************													

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************* Intersection #3 San Juan Grade / Boronda ********************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 12 Optimal Cycle: 180 Average Delay (sec/veh): 75.7 Level Of Service: ************************** San Juan Grade Boronda South Bound Approach: North Bound East Bound West Bound L-T-R L-T-R L-T-R Movement: Protected Protected Protected Control: Protected Rights: Include Include Include Include 6 6 6 6 6 6 6 6 6 Min. Green: 6 6 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 2 0 2 0 1 2 0 2 0 1 2 0 1 1 0 2 0 3 0 1 Lanes: Volume Module: 250 170 290 420 180 430 100 1460 160 290 2070 Base Vol: Growth Adj: 1.00 1.00 1.00 1.00 Initial Bse: 250 170 290 420 180 430 100 1460 160 290 2070 1.00 1.00 1.00 User Adj: PHF Adj: 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91 PHF Volume: 275 187 319 462 198 473 110 1604 176 319 2275 396 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 319 473 Reduced Vol: 275 187 462 198 110 1604 176 319 2275 396 PCE Adj: MLF Adj: 1.00 1.00 1.00 319 473 FinalVolume: 275 187 462 198 110 1604 176 319 2275

_____| Saturation Flow Module: Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Adjustment: 0.88 0.90 0.81 0.88 0.90 0.81 0.86 0.87 0.87 0.89 0.88 0.83 Lanes: 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.80 0.20 2.00 3.00 Final Sat.: 3334 3437 1537 3334 3437 1537 3274 2996 328 3400 5037 1568 Capacity Analysis Module:

0.31 0.03 0.54 0.54 Vol/Sat: 0.08 0.05, 0.21 0.14 0.06 0.09 0.45 Crit Moves: **** Green/Cycle: 0.07 0.20 0.20 0.13 0.27 0.27 0.06 0.46 0.46 0.08 0.48 0.48 Volume/Cap: 1.16 0.27 1.03 1.03 0.22 1.16 0.53 1.16 1.16 1.16 0.94 0.53 Uniform Del: 46.4 33.7 39.9 43.3 28.6 36.7 45.4 26.9 26.9 46.0 24.7 IncremntDel:107.7 0.2 58.2 2.5 78.8 49.5 0.1 95.2 78.8 103.9 8.3 0.7 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay/Veh: 154.2 33.9 98.1 92.8 28.7 131.9 47.8 106 105.7 149.8 33.0 1.00 AdjDel/Veh: 154.2 33.9 98.1 92.8 28.7 131.9 47.8 106 105.7 149.8 33.0 18.8 LOS by Move: F C F F C F F D F F С 10 3 16 12 3 26 2 47 47 HCM2kAvgO: 11

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative)

Cycle (sec): 100 Critical Vol./Cap.(X): 1.461
Loss Time (sec): 12 Average Delay (sec/veh): 155.2
Optimal Cycle: 180 Level Of Service: F

6 6 6 Min. Green: 6 6 6 6 6 6 6 6 6 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 2 0 2 0 1 2 0 2 0 1 2 0 1 1 0 2 0 3 0 1

290

140 2390

170

440 1900

350 210

Volume Module:

Base Vol:

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 430 280 350 210 380 290 140 2390 170 440 1900 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 📧 1.00 1.00 PHF Adj: $0.94 \ 0.94 \ 0.94 \ 0.94 \ 0.94 \ 0.94 \ 0.94 \ 0.94 \ 0.94 \ 0.94$ PHF Volume: 457 298 404 372 223 309 149 2543 181 468 2021 319 0 0 0 0 0 0 0 - 0 0 Reduct Vol: 0 Reduced Vol: 457 298 404 372 223 309 149 2543 468 2021 181 319 PCE Adj: 1.00 1.00 MLF Adj: 404 FinalVolume: 457 298 372 223 309 149 2543 181 468 2021 _____|

Saturation Flow Module:

430 280

380

Vol/Sat: 0.13 0.08 0.26 0.11 0.06 0.20 0.04 0.78 0.78 0.14 0.40 0.20

Capacity Analysis Module:

*** *** Crit Moves: Green/Cycle: 0.10 0.17 0.17 0.07 0.15 0.15 0.08 0.54 0.54 0.09 0.55 0.55 Volume/Cap: 1.32 0.48 1.46 1.46 0.43 1.32 0.53 1.46 1.46 1.46 0.73 Uniform Del: 45.0 37.2 41.3 46.3 38.7 42.6 44.0 23.2 23.2 45.3 17.0 IncremntDel:163.7 0.6 226.6 228.0 0.6 171.5 1.9 211 210.7 224.2 1.0 0.3 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: Delay/Veh: 208.6 37.8 267.8 274.3 39.3 214.1 45.9 234 233.8 269.5 18.0 1.00 AdjDel/Veh: 208.6 37.8 267.8 274.3 39.3 214.1 45.9 234 233.8 269.5 18.0 13.1

LOS by Move: F D F F D F D F F B B HCM2kAvgQ: 17 5 30 15 4 21 3 99 99 19 18 6

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ***************************** Intersection #3 San Juan Grade / Boronda ************************** Cycle (sec): 100 Critical Vol./Cap.(X): 1.642
Loss Time (sec): 12 Average Delay (sec/veh): 196.2
Optimal Cycle: 180 Level Of Service: F ******************************* Street Name: San Juan Grade Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RControl: Protected Protected Protected Protected Rights: Include Include Include Include Min. Green: Y+R: Volume Module: Base Vol: 250 260 500 370 280 180 260 2600 370 340 1740 Initial Bse: 250 260 500 370 280 260 2600 180 370 340 1740 PHF Volume: 260 271 521 385 292 188 271 2708 385 354 1813 0 0 0 Reduct Vol: 0 0 0 0 0 0 Reduced Vol: 260 271 521 385 292 188 271 2708 385 354 1813 250 FinalVolume: 260 271 521 385 292 188 271 2708 385 354 1813 Saturation Flow Module: 1900 Adjustment: 0.90 0.93 0.83 0.89 0.92 0.83 0.89 0.90 0.90 0.90 0.89 0.83 Lanes: 2.00 2.00 1.00 2.00 2.00 1.00 2.00 1.75 0.25 2.00 3.00 1.00 Final Sat.: 3432 3538 1583 3400 3505 1568 3400 3010 428 3432 5083 1583 -----||-----||-----||-----| Capacity Analysis Module: Vol/Sat: 0.08 0.08 0.33 0.11 0.08 0.12 0.08 0.90 0.90 0.10 0.36 0.16 **** Crit Moves: Green/Cycle: 0.10 0.20 0.20 0.07 0.16 0.16 0.11 0.55 0.55 0.06 0.50 0.50 Volume/Cap: 0.73 0.38 1.64 1.64 0.50 0.73 0.71 1.64 1.64 1.64 0.71 Uniform Del: 43.4 34.6 40.0 46.5 38.0 39.6 42.9 22.6 22.6 46.9 19.5 14.9 IncremntDel: 7.2 0.3 302.9 307.5 0.7 9.8 6.3 292 291.5 309.0 1.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 50.6 35.0 342.9 354.1 38.8 49.4 49.2 314 314.1 355.9 20.5 15.1 AdjDel/Veh: 50.6 35.0 342.9 354.1 38.8 49.4 49.2 314 314.1 355.9 20.5 15.1 LOS by Move: D C F F D D D F F F F HCM2kAvgQ: 6 4 42 17 5 7 6 127 127 16 С Note: Queue reported is the number of cars per lane.

APPENDIX F INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

San Juan Grade Road/Russell Road

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ********************************** Intersection #4 San Juan Grade / Russell 100 Cycle (sec): Critical Vol./Cap.(X): 9 58 Loss Time (sec): Average Delay (sec/veh): 28.4 Optimal Cycle: Level Of Service: San Juan Grade Street Name: Russell Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|------| Protected Protected Split Phase Control: Split Phase Include Include Include Include 6 6 6 6 0 6 6 6 0 ... 0 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 Lanes: 1 0 1 0 0 1 0 1 0 1 1 0 0 0 1 0 0 0 0 0 Volume Module: Base Vol: 154 210 0 0 477 330 331 0 128 0 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 154 210 0 477 0 330 331 0 128 0 0 - 0 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: PHF Volume: 197 269 0 0 612 423 424 0 164 Reduct Vol: 0 0 0 0 0 0 0 Ω Ω 0 O Reduced Vol: 197 269 0 0 612 423 424 0 164 0 0 0 PCE Adj: MLF Adj: 1.00 1.00 FinalVolume: 197 269 0 0 612 423 424 0 164 0 0 -----|----|-----|------| Saturation Flow Module: Adjustment: 0.90 0.95 1.00 1.00 1.00 0.85 0.91 1.00 0.82 1.00 1.00 Lanes: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 0.00 0.00 0.00 Final Sat.: 1718 1809 0 1900 1900 1615 1736 0 1554 0 0 Capacity Analysis Module: Vol/Sat: 0.11 0.15 0.00 0.00 0.32 0.26 0.24 0.00 0.11 0.00 0.00 0.00 Crit Moves: **** *** **** Volume/Cap: 0.75 0.26 0.00 0.00 0.75 0.61 0.75 0.00 0.32 0.00 0.00 0.00 Uniform Del: 40.5 10.2 0.0 0.0 24.0 22.0 30.0 0.0 25.4 0.0 0.0 IncremntDel: 11.2 0.1 0.0 0.0 3.9 1.6 5.5 0.0 0.4 0.0 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 0.00 0.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 0.00 0.0 27.8 23.6 51.7 10.3 Delay/Veh: 35.5 0.0 25.7 0.0 0.0 0.0 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 51.7 10.3 0.0 0.0 27.8 23.6 35.5 0.0 25.7 0.0 0.0 0.0 Α LOS by Move: D В A C С D Α С Α Α Α 8 4 0 HCM2kAvgQ: 0 17 11 13 0 ******************************** Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) Intersection #4 San Juan Grade / Russell ****************************** Cycle (sec): 100 Critical Vol./Cap.(X): Average Delay (sec/veh): Level Of Service: Loss Time (sec): 9
Optimal Cycle: 39 23.7 ********************** San Juan Grade Street Name: Russell Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R _____| Control: Protected Protected Split Phase Split Phase Rights: Include Include Include Include Rights: Include Include Include Include Min. Green: 6 6 0 6 6 6 6 0 0 0 Y+R: Volume Module: 116 224 0 257 0 120 Base Vol: 0 336 227 0 336 Initial Bse: 116 224 0 227 257 0 120 0 0 0 436 PHF Volume: 151 291 0
Reduct Vol: 0 0 295 334 0 156 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 151 291 0 0 436 295 334 0 156 PCE Adj: MLF Adj: FinalVolume: 151 291 0 0 436 295 334 0 156 0 0 Saturation Flow Module: Adjustment: 0.92 0.97 1.00 1.00 0.96 0.82 0.90 1.00 0.81 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 Lanes: Final Sat.: 1753 1845 0 1900 1828 1554 1718 0 1537 0 0 -----||-----||-----||------| Capacity Analysis Module: Vol/Sat: 0.09 0.16 0.00 0.00 0.24 0.19 0.19 0.00 0.10 0.00 0.00 Crit Moves: **** **** **** Green/Cycle: 0.15 0.57 0.00 0.00 0.42 0.42 0.34 0.00 0.34 0.00 0.00 0.00 Volume/Cap: 0.57 0.28 0.00 0.00 0.57 0.45 0.57 0.00 0.30 0.00 0.00 0.00 Uniform Del: 39.5 11.0 0.0 22.2 20.9 27.0 0.0 24.2 0.0 0.0 0.0 0.5 0.3 IncremntDel: 3.0 0.1 0.0 0.0 1.0 1.3 0.0 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 0.00 0.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 Delay/Veh: 42.4 11.2 0.0 0.0 23.2 21.4 28.3 0.0 24.5 0.0 0.0 0.0 0.0 23.2 21.4 28.3 0.0 24.5 AdjDel/Veh: 42.4 11.2 0.0 0.0 0.0 Ā A C 0 11 LOS by Move: D B С C A C Α Α Α 5 5 0 7 9 0 4 0 -0 0 HCM2kAvgO: ******************************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ****************************** Intersection #4 San Juan Grade / Russell ************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 9
Optimal Cycle: 30 Average Delay (sec/veh): 25.1 Level Of Service: ******************************** San Juan Grade Street Name: Russell Approach: North Bound South Bound East Bound West Bound L-T-R L-T-R L-T-R Movement: Protected Protected Split Phase Split Phase Control: Include Include Include
6 6 6 6 0 6 0 0
4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Rights: Include Include 6 6 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 Lanes: Volume Module: 116 217 0 Base Vol: 4 260 127 258 0 78 0 4 260 Initial Bse: 116 217 0 127 258 0 . 78 0 0 PHF Volume: 126 236 0 4 283 138 280 0 85 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 126 236 0 4 283 138 280 85 0 0 PCE Adj: MLF Adj: 1.00 1.00 1.00 85 FinalVolume: 126 236 0 4 283 138 280 0 0 0 Saturation Flow Module: Adjustment: 0.93 0.98 1.00 0.93 0.98 0.83 0.92 1.00 0.83 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 Lanes: Final Sat.: 1769 1862 0 1769 1862 1583 1753 0 1568 Capacity Analysis Module: Vol/Sat: 0.07 0.13 0.00 0.00 0.15 0.09 0.16 0.00 0.05 0.00 0.00 0.00 Crit Moves: **** **** *** Green/Cycle: 0.17 0.36 0.00 0.17 0.36 0.36 0.38 0.00 0.38 0.00 0.00 0.00 0.42 0.00 0.14 0.00 0.01 0.42 0.24 Volume/Cap: 0.42 0.35 0.00 0.00 Uniform Del: 37.1 23.5 0.0 34.5 24.1 22.4 22.9 0.0 20.3 0.0 0.0 0.0 IncremntDel: 1.0 0.3 0.0 0.0 0.4 0.2 0.4 0.0 0.1 0.0 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 0.00 1.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 0.00 Delay/Veh: 38.1 23.8 0.0 34.5 24.5 22.6 23.3 0.0 20.4 0.0 0.0 AdjDel/Veh: 38.1 23.8 0.0 34.5 24.5 22.6 23.3 0.0 20.4 0.0 0.0 0.0 LOS by Move: D C A C C С С A C Α A Α 7 4 5 0 0 3 6 0 0 HCM2kAvqQ: ******************************* Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************************** Intersection #4 San Juan Grade / Russell 100 🖘 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 9
Optimal Cycle: 141 Average Delay (sec/veh): 41.6 Level Of Service: San Juan Grade Street Name: Russell Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R L - T - R Movement: -----|----|-----|-----| Protected Protected Split Phase Include Include Include Control: Split Phase Include Include 6 6 6 6 0 6 4.0 4.0 4.0 4.0 4.0 Rights: Include 0 0 6 6 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 4.0 1 0 1 0 0 1 0 1 0 1 Lanes: 1 0 0 0 1 0 0 0 0 0 Volume Module: 154 332 0 Base Vol: 0 558 360 375 0 128 0 Initial Bse: 154 332 0 0 558 360 375 0 128 0 0 0 PHF Volume: 223 481 0 0 809 522 543 0 186 0 0 0 - 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 543 0 Reduced Vol: 223 481 0 809 522 186 0 0 MLF Adj: 1.00 1.00 1.00 FinalVolume: 223 481 0 0 809 522 543 0 186 0 0 Saturation Flow Module: Adjustment: 0.90 0.95 1.00 1.00 1.00 0.85 0.91 1.00 0.82 1.00 1.00 1.00 Lanes: 1.00 1.00 0.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 Final Sat.: 1718 1809 0 1900 1900 1615 1736 0 1554 0 0 -----| -----| | ------| | ------| Capacity Analysis Module: Vol/Sat: 0.13 0.27 0.00 0.00 0.43 0.32 0.31 0.00 0.12 0.00 0.00 Crit Moves: **** **** *** Green/Cycle: 0.14 0.58 0.00 0.00 0.45 0.45 0.33 0.00 0.33 0.00 0.00 Volume/Cap: 0.95 0.46 0.00 0.00 0.95 0.72 0.95 0.00 0.36 0.00 0.00 Uniform Del: 42.9 11.9 0.0 0.0 26.7 22.7 32.9 0.0 25.6 0.0 0.0 IncremntDel: 46.0 0.3 0.0 0.0 20.6 3.7 26.7 0.0 0.4 0.0 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 0.00 0.00 1.00 1.00 0.00 1.00 0.00 0.00 AdjDel/Veh: 88.9 12.2 0.0 0.0 47.3 26.3 59.5 0.0 26.1 0.0 0.0 LOS by Move: F B A A D C E A С A A Α 0 0 30 14 22 HCM2kAvqQ: 11 8 0 4 0 ************************************** Note: Queue reported is the number of cars per lane. ***********************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************** Intersection #4 San Juan Grade / Russell ************************* 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 9
Optimal Cycle: 49 24.6 Average Delay (sec/veh): Level Of Service: ************************************ San Juan Grade Street Name: Russell Approach: North Bound South Bound East Bound West Bound L-T-R L-T-R L-T-R Movement: Protected Protected Control: Split Phase Split Phase Include Include Rights: Include 6 0 6 Include 0 0 6 6 0 6 6 6 4.0 4.0 4.0 4.0 4.0 4.0 6 6 0 Min. Green: 4.0 4.0 4.0 4.0 4.0 Lanes: 1 0 1 0 0 1 0 1 0 1 1 0 0 0 1 0 0 0 0 Volume Module: 0 Base Vol: 116 275 0 411 255 275 0 120 0 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 116 275 , 0 0 411 255 275 0 120 0 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 PHF Volume: 159 377 0 0 563 349 377 0 164 0 Reduct Vol: 0 0 0 0 0 0 0 0 - 0 0 0 0 Reduced Vol: 159 377 0 0 563 349 377 0 164 0 0 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 159 377 0 0 563 349 377 0 164 0 0 0 Saturation Flow Module: Adjustment: 0.92 0.97 1.00 1.00 0.96 0.82 0.90 1.00 0.81 1.00 1.00 1.00 Lanes: 1.00 1.00 0.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 Final Sat.: 1753 1845 0 1900 1828 1554 1718 0 1537 0 0 -----||-----||-----|-----| Capacity Analysis Module: Vol/Sat: 0.09 0.20 0.00 0.00 0.31 0.22 0.22 0.00 0.11 0.00 0.00 0.00 Crit Moves: **** **** **** Green/Cycle: 0.13 0.59 0.00 0.00 0.45 0.45 0.32 0.00 0.32 0.00 0.00 Volume/Cap: 0.68 0.35 0.00 0.00 0.68 0.50 0.68 0.00 0.33 0.00 0.00 Uniform Del: 41.3 10.7 0.0 0.0 21.6 19.3 29.4 0.0 25.7 0.0 0.0 IncremntDel: 7.8 0.2 0.0 0.0 2.3 0.6 3.4 0.0 0.4 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.00 1.00 0.00 Delay Adj: 0.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 0.00 32.8 0.0 26.1 49.1 10.9 0.0 23.9 19.8 Delay/Veh: 0.0 0.0 0.0 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 49.1 10.9 19.8 32.8 0.0 0.0 0.0 23.9 26.1 0.0 0.0 0.0 A LOS by Move: D B A C В С C , A Α Α A 6 6 0 0 14 8 0 HCM2kAvqQ: 11 *********************************** Note: Queue reported is the number of cars per lane. *************************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************************** Intersection #4 San Juan Grade / Russell ****************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 9
Optimal Cycle: 34 Average Delay (sec/veh): Level Of Service: ******************************** San Juan Grade Street Name: Russell Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R_____| Protected Protected Split Phase Control: Split Phase

 Rights:
 Include
 Include
 Include
 Include

 Min. Green:
 6
 6
 0
 6
 6
 0
 6
 0
 0
 0

 Y+R:
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 4.0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0< 4.0 4.0 4.0 -----| Volume Module: 116 246 0 4 297 269 0 78 Base Vol: 141 0. Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 116 246 4 297 269 0 78 0 141 0 0 PHF Adj: PHF Volume: 138 293 0 168 320 0 93 5 354 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 138 293 5 354 0 168 320 0 93 0 MLF Adj: 320 0 93 FinalVolume: 138 293 0 5 354 168 0 0 _____| Saturation Flow Module: Adjustment: 0.93 0.98 1.00 0.93 0.98 0.83 0.92 1.00 0.83 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 Lanes: Final Sat.: 1769 1862 0 1769 1862 1583 1753 0 1568 ______|__|___|___| Capacity Analysis Module: Vol/Sat: 0.08 0.16 0.00 0.00 0.19 0.11 0.18 0.00 0.06 0.00 0.00 0.00 Crit Moves: **** **** *** Green/Cycle: 0.16 0.39 0.00 0.15 0.38 0.38 0.37 0.00 0.37 0.00 0.00 Volume/Cap: 0.50 0.40 0.00 0.02 0.50 0.28 0.50 0.00 0.16 0.00 0.00 0.0 36.3 23.5 21.3 24.4 0.0 21.2 Uniform Del: 38.5 22.0 0.0 0.0 0.0 0.2 0.6 0.0 IncremntDel: 1.4 0.4 0.0 0.0 0.5 0.1 0.0 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 0.00 1.00 1.00 1.00 0.00 1.00 0.00 0.00 39.9 22.3 0.0 36.3 24.0 21.5 25.0 0.0 21.3 Delay/Veh: 0.0 0.0 0.0 36.3 24.0 21.5 25.0 0.0 21.3 AdjDel/Veh: 39.9 22.3 0.0 0.0 0.0 A C **A** D C A LOS by Move: D C C С Α Α 0 4 7 0 8 4 8 0 2 0 HCM2kAvgQ: ******************************* Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ********************* Intersection #4 San Juan Grade / Russell ********************* 100 Critical Vol./Cap.(X): Cycle (sec): 1.131 Loss Time (sec): 12 Optimal Cycle: 180 Average Delay (sec/veh): Level Of Service: ****************************** Street Name: San Juan Grade Russell North Bound South Bound Approach: East Bound West Bound L-T-R L-T-R L-T-R Movement: -----| Protected Protected Control: Protected Protected Include Rights: Include Include Include Min. Green: 6 6 6 6 6 6 :: 6 6 6 6 $4.0 \ 4.0 \ 4.0 \ 4.0 \ 4.0 \ 4.0 \ 4.0 \ 4.0 \ 4.0 \ 4.0$ 2 0 1 1 0 2 0 2 0 1 2 0 2 0 1 2 0 2 0 1 Lanes: -----||-----||-----||-----| Volume Module: 300 230 510 Base Vol: 530 490 270 280 690 250 400 740 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 300 230 530 490 510 270 280 690 250 400 740 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86 0.86 PHF Volume: 349 267 616 570 326 802 593 314 291 465 860 0 , Reduct Vol: E 0 0 0 0 0 0 0 - 0 0 0 0 291 Reduced Vol: 349 267 593 616 570 314 326 802 465 860 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 349 267 593 616 570 314 326 802 291 465 860 Saturation Flow Module: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Sat/Lane: Adjustment: 0.89 0.83 0.83 0.88 0.90 0.81 0.89 0.91 0.82 0.89 0.91 2.00 2.00 1.00 Final Sat.: 3400 1572 1572 3334 3437 1537 3369 3473 1554 3369 3473 1554 Capacity Analysis Module: Vol/Sat: 0.10 0.17 0.38 0.18 0.17 0.20 0.10 0.23 0.19 0.14 0.25 0.34 **** Crit Moves: Green/Cycle: 0.17 0.33 0.33 0.16 0.33 0.33 0.09 0.24 0.24 0.14 0.30 0.30 Volume/Cap: 0.62 0.51 1.13 1.13 0.50 0.62 1.13 0.96 0.78 0.96 0.83 Uniform Del: 38.7 26.8 33.3 41.8 26.8 28.1 45.7 37.6 35.5 42.6 32.8 IncremntDel: 2.1 0.3 75.1 93.2 22.6 80.1 0.4 2.3 10.1 31.6 5.9 83.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Delay/Veh: 40.8 27.0 108.4 121.9 27.2 30.4 139.0 60.1 45.7 74.2 38.6 118.1 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 40.8 27.0 108.4 121.9 27.2 30.4 139.0 60.1 45.7 74.2 38.6 118.1 F C LOS by Move: D C F C F E D 8 9 11 18 HCM2kAvqQ: 6 7 32 18 10 12 ******************************* Note: Queue reported is the number of cars per lane. ************************

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ************************* Intersection #4 San Juan Grade / Russell ********************************* 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 12 Optimal Cycle: 180 Average Delay (sec/veh): Level Of Service: San Juan Grade Russell Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RProtected Protected Protected Control: Protected Rights: Include Include | Include Include 6 6 6 Min. Green: 6 6 6 6 6 6 6 6 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 2 0 1 1 0 2 0 2 0 1 2 0 2 0 1 2 0 2 0 1 Volume Module: Base Vol: 280 230 430 490 360 160 180 800 290 540 740 1.00 1.00 1.00 Initial Bse: 280 230 490 360 430 160 180 800 290 540 740 User Adj: PHF Adj: PHF Volume: 326 267 500 570 419 186 209 930 337 628 860 674 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 326 267 500 570 419 186 209 930 337 628 860 674 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 500 209 930 337 FinalVolume: 326 267 570 419 186 628 860 Saturation Flow Module: Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 Adjustment: 0.89 0.83 0.83 0.89 0.91 0.82 0.88 0.90 0.81 0.88 0.90 Lanes: Final Sat.: 3400 1581 1581 3369 3473 1554 3334 3437 1537 3334 3437 1537 Capacity Analysis Module: Vol/Sat: 0.10 0.17 0.32 0.17 0.12 0.12 0.06 0.27 0.22 0.19 0.25 **** Crit Moves: 0.15 0.24 0.24 0.06 0.26 0.26 Green/Cycle: 0.19 0.28 0.28 0.18 0.39 0.39 Volume/Cap: 0.50 0.60 1.13 1.13 0.50 0.50 1.05 1.02 0.83 1.02 0.64 Uniform Del: 36.2 31.1 36.0 42.5 32.8 32.8 47.0 36.8 34.6 40.8 24.9 IncremntDel: 0.6 0.8 75.0 79.7 0.5 13.2 1.176.4 35.4 41.9 1.1 76.9 InitQueuDel: 0.0 0.0 . 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 36.8 32.0 111.0 122.2 33.3 33.9 123.4 72.2 47.8 Delay/Veh: 82.7 26.0 107.5 1.00 1.00 AdjDel/Veh: 36.8 32.0 111.0 122.2 33.3 33.9 123.4 72.2 47.8 82.7 26.0 107.5 LOS by Move: D C F HCM2kAvgQ: 5 8 27 F C C F E D 17 6 5 7 22 12 F С 16 12 ********************************* Note: Queue reported is the number of cars per lane. Intersection #4 San Juan Grade / Russell

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) **************************

***************************** 100 Critical Vol./Cap.(X): Cycle (sec): Loss Time (sec): 12 Optimal Cycle: 48 Average Delay (sec/veh): Level Of Service: С

San Juan Grade Russell South Bound East Bound Approach: North Bound West Bound

Movement: L-T-R L-T-R L-T-R Protected Protected Protected Control: Protected Rights: Include Include Include Include Min. Green: 6 6 6 6 6 6 6 6 6 6 6 6 4.0 4.0 4.0 4.0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 2 0 1 1 0 2 0 2 0 1 2 0 2 0 1 Lanes: 2 0 2 0 1

Volume Module: 220 150 215 245 340 120 120 400 300 Base Vol: 270 370 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 220 150 215 245 340 120 120 400 300 270 370 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 1.00 PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 PHF Volume: 239 163 234 266 370 130 130 435 326 293 402 315

0 Reduct Vol: 0 0 . 0 0 . 0 0 0 0 0 0 0 Reduced Vol: 239 163 234 266 370 130 130 435 326 293 402 315 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PCE Adj: 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 234 FinalVolume: 239 163 266 370 130 130 435 326 293 402

Saturation Flow Module: 1900 1900 1900 1900 1900 1900 1900 1900 1900 Sat/Lane: 1900 1900 Adjustment: 0.89 0.84 0.84 0.89 0.91 0.82 0.88 0.90 0.81 0.88 0.90 0.81 Lanes: 2.00 1.00 1.00 2.00 2.00 1.00 2.00 2.00 1.00 2.00 2.00 1.00

Final Sat.: 3400 1598 1598 3369 3473 1554 3334 3437 1537 3334 3437 1537 _____|

Capacity Analysis Module: 0.08 0.11 0.08 0.04 0.13 Vol/Sat: 0.07 0.10 0.15 0.21 0.09 0.12 0.21 Crit Moves: Green/Cycle: 0.15 0.24 0.24 0.13 0.23 0.23 0.11 0.36 0.36 0.15 0.39 0.39

Volume/Cap: 0.47 0.42 0.60 0.60 0.47 0.37 0.34 0.36 0.60 0.60 0.30 Uniform Del: 38.8 31.7 33.4 40.9 33.4 32.6 40.9 23.8 26.4 39.8 21.1 IncremntDel: 0.7 0.3 0.7 1.5 2.2 0.4 0.5 0.2 1.8 2.0 0.1 0.9 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 39.5 32.0 34.9 43.1 33.9 33.3 41.4 24.0 Delay/Veh: 28.2 41.9 21.3

1.00 1.00 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 39.5 32.0 34.9 43.1 33.9 33.3 41.4 24.0 28.2 41.9 21.3 24.4 LOS by Move: D C C D C HCM2kAvgQ: 4 5 7 5 6 С D C C D С 4 2 5 9 ********************************

Note: Queue reported is the number of cars per lane.

APPENDIX G INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

San Juan Grade Road/Penzance Streeet

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********************************* Intersection #5 San Juan Grade / Penzance ************************************* Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx] *********************************** San Juan Grade Street Name: Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R
 Control:
 Stop Sign
 Stop Sign
 Uncontrolled
 Uncontrolled

 Rights:
 Include
 Include
 Include
 Include

 Lanes:
 0 0 1! 0 0 1 0 0 1 0 0 1! 0 0 0 1! 0 0
 0 0 1! 0 0
 0 0 1! 0 0
 -----| Volume Module: Base Vol: 1 411 118 8 497 0 1 1 5 168 3 Initial Bse: 1 411 118 8 497 0 1 1 5 168 3 10 PHF Adj: PHF Volume: 1 478 137 9 578 Reduct Vol: 0 0 0 0 0 FinalVolume: 1 478 137 9 578 0 1 1 6 195 3 12 0 0 0 6 195 -----|-----|------|------| Critical Gap Module: Critical Gp: 7.2 6.6 6.3 7.2 6.6 6.3 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx -----||-----||-----| Capacity Module: Cnflict Vol: 695 412 4 714 409 9 15 xxxx xxxxx 7 xxxx xxxxx Potent Cap.: 352 525 1071 342 527 1064 1616 xxxx xxxxx 1627 xxxx xxxxx Move Cap.: 0 454 1071 0 456 1064 1616 xxxx xxxxx 1627 xxxx xxxxx Volume/Cap: xxxx 1.05 0.13 xxxx 1.27 0.00 0.00 xxxx xxxx 0.12 xxxx xxxx -----| Level Of Service Module: LOS by Move: * * * * * * A * * A * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT . LT - LTR - RT Shrd ConDel:xxxxx xxxx xxxxx xxxxx xxxx 163.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx Shared LOS: * * * * * F * * * * * ApproachLOS: xxxxxx ApproachLOS: F +Inf XXXXXX XXXXXX \mathbf{F}_{i} Note: Queue reported is the number of cars per lane. *************************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************************** Intersection #5 San Juan Grade / Penzance ******************************** Average Delay (sec/veh): 25.9 Worst Case Level Of Service: D[29.9] ********************************* San Juan Grade Street Name: Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RControl: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 1 0 0 1 0 0 0 1! 0 0 0 0 1! 0 0 -----||-----||------| Volume Module: Base Vol: 9 394 97 13 453 3 1 2 3 92 3 Initial Bse: 9 394 97 13 453 3 1 2 3 92 3 15 PHF Volume: 10 419 103 14 482 3 1 2 3 Reduct Vol: 0 0 0 0 0 0 0 0 98 3 16 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 10 419 103 14 482 3 1 2 3 0 0 0 98 Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.2 6.6 6.3 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.6 4.1 3.4 2.2 xxxx xxxxx 2.2 xxxx xxxxx -----|----|-----|------||**---**----||-----||------| Capacity Module: Cnflict Vol: 455 221 4 474 214 11 19 xxxx xxxxx 5 xxxx xxxxx Potent Cap.: 512 674 1074 494 676 1058 1610 xxxx xxxxx 1629 xxxx xxxxx Move Cap.: 183 631 1074 199 633 1058 1610 xxxx xxxxx 1629 xxxx xxxxx Volume/Cap: 0.05 0.66 0.10 0.07 0.76 0.00 0.00 xxxx xxxx 0.06 xxxx xxxx -----| Level Of Service Module: Control Del:xxxxx xxxx xxxxx 24.4 xxxx xxxxx 7.2 xxxx xxxxx 7.4 xxxx xxxxx LOS by Move: * * * C * * A * * A * * Movement: LT - LTR - RT Shared LOS: * D * * * D * * * * * ApproachDel: 29.9
ApproachLOS: D 26.6 XXXXXX D *********************************** Note: Queue reported is the number of cars per lane. ********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************** Intersection #5 San Juan Grade / Penzance *********************************** 18.8 Average Delay (sec/veh): Worst Case Level Of Service: C[22.4] ********************************** Street Name: San Juan Grade Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R-----| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Rights: Include Includ Volume Module: Base Vol: 10 382 86 11 353 4 2 4 61 2 Initial Bse: 10 382 86 11 353 4 2 4 4 PHF Volume: 11 429 97 12 397 Reduct Vol: 0 0 0 0 0 4 2 4 4 69 2 0 0 0 0 4 2 4 0 0 4 0 0 FinalVolume: 11 429 97 12 397 69 -----| Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 354 157 7 417 156 6 9 xxxx xxxxx 9 xxxx xxxxx Potent Cap.: 601 735 1076 545 734 1074 1624 xxxx xxxxx 1624 xxxx xxxxx Move Cap.: 320 702 1076 246 701 1074 1624 xxxx xxxxx 1624 xxxx xxxxx Volume/Cap: 0.04 0.61 0.09 0.05 0.57 0.00 0.00 xxxx xxxx 0.04 xxxx xxxx Level Of Service Module: Control Del:xxxxx xxxx xxxxx 20.4 xxxx xxxxx 7.2 xxxx xxxxx 7.3 xxxx xxxxx LOS by Move: * * * C * * A * * A * * Movement: LT - LTR - RT Shrd ConDel:xxxxx 22.4 xxxxx xxxxx xxxx 16.7 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx Shared LOS: * C * * * C * * * * * 16.8 ApproachDel: 22.4
ApproachLOS: C XXXXXX XXXXXX С Note: Queue reported is the number of cars per lane. ***************************** -----

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************** Intersection #5 San Juan Grade / Penzance ******************************* Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx] San Juan Grade Street Name: Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R _____
 Control:
 Stop Sign
 Stop Sign
 Uncontrolled
 Uncontrolled

 Rights:
 Include
 Include
 Include
 Include

 Lanes:
 0 0 1! 0 0 1 0 0 1 0 0 1! 0 0 0 1! 0 0
 0 0 1! 0 0
 0 0 1! 0 0
 _____| Volume Module: Base Vol: 1 577 118 8 608 0 1 7 5 168 7 Initial Bse: 1 577 118 8 608 0 1 7 5 168 7 10 PHF Volume: 1 801 164 11 844 0 1 10 7 233 10 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 1 801 164 11 844 0 1 10 7 233 10 -----| Critical Gap Module: Critical Gp: 7.2 6.6 6.3 7.2 6.6 6.3 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 922 506 13 982 503 17 24 xxxx xxxxx 17 xxxx xxxxx Potent Cap.: 248 465 1058 225 467 1054 1604 xxxx xxxxx 1614 xxxx xxxxx Move Cap.: 0 387 1058 0 389 1054 1604 xxxx xxxxx 1614 xxxx xxxxx Volume/Cap: xxxx 2.07 0.15 xxxx 2.17 0.00 0.00 xxxx xxxx 0.14 xxxx xxxx Level Of Service Module: LOS by Move: * * * * * A * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shrd ConDel:xxxxx xxxx xxxxx xxxxx xxxx 558.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx ApproachDel: xxxxx ApproachLOS: F ************************************* Note: Queue reported is the number of cars per lane. **********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************* Intersection #5 San Juan Grade / Penzance ************************** Average Delay (sec/veh): OVERFLOW Worst Case Level Of Service: F[xxxxx] ****************************** Street Name: San Juan Grade Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R-----|----|-----|------| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 1 0 0 1 0 0 0 1! 0 0 0 0 1! 0 0 Volume Module: Base Vol: 9 463 97 13 556 3 1 5 3 92 7 Initial Bse: 9 463 97 13 556 3 1 5 3 92 7 15 PHF Volume: 11 551 115 15 662 4 1 6 4 110 8 18 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 11 551 115 15 662 4 1 6 4 110 8 18 Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.2 6.6 6.3 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.6 4.1 3.4 2.2 xxxx xxxxx 2.2 xxxx xxxxx _____ Capacity Module: Cnflict Vol: 579 255 8 580 248 17 26 xxxx xxxxx 10 xxxx xxxxx Potent Cap.: 423 645 1069 420 648 1050 1601 xxxx xxxxx 1623 xxxx xxxxx Move Cap.: 0 598 1069 70 601 1050 1601 xxxx xxxxx 1623 xxxx xxxxx Volume/Cap: xxxx 0.92 0.11 0.22 1.10 0.00 0.00 xxxx xxxx 0.07 xxxx xxxx Level Of Service Module: Control Del:xxxxx xxxx xxxxx 71.0 xxxx xxxxx 7.3 xxxx xxxxx 7.4 xxxx xxxxx LOS by Move: * * * F * * A * * Movement: LT - LTR - RT Shared LOS: * * * * * F * * * * * ApproachDel: xxxxxx 93.7 XXXXXX F ApproachLOS: F *************************** Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************** Intersection #5 San Juan Grade / Penzance ************************************ 33.7 Average Delay (sec/veh): Worst Case Level Of Service: E[45.5] ***************************** Street Name: San Juan Grade Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 1 0 0 1 0 0 0 1! 0 0 0 0 1! 0 0 Volume Module: Base Vol: 10 422 86 11 404 4 2 5 4 61 4 Initial Bse: 10 422 86 11 404 4 2 5 4 61 4 6 PHF Volume: 13 528 108 14 505 5 3 6 5 76 5 8 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 13 528 108 14 505 5 3 6 5 76 5 8 Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 430 179 9 493 178 9 13 xxxx xxxxx 11 xxxx xxxxx Potent Cap.: 535 715 1073 485 714 1070 1619 xxxx xxxxx 1621 xxxx xxxxx Move Cap.: 202 679 1073 152 678 1070 1619 xxxx xxxxx 1621 xxxx xxxxx Volume/Cap: 0.06 0.78 0.10 0.09 0.74 0.00 0.00 xxxx xxxx 0.05 xxxx xxxx -----| Level Of Service Module: Control Del:xxxxx xxxx xxxxx 31.1 xxxx xxxxx 7.2 xxxx xxxxx 7.3 xxxx xxxxx LOS by Move: * * * D * * A * * A * * Movement: LT - LTR - RT Shrd ConDel:xxxxx 45.5 xxxxx xxxxx xxxx 24.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx Shared LOS: * E * * * C * * * * * ApproachDel: 45.5
ApproachLOS: E 24.5 XXXXXX XXXXXX С *********************************** Note: Queue reported is the number of cars per lane.

MILIGO - EXI												
w/mitigation												
Level Of Service Computation Report												
2000 HCM Operations Method (Base Volume Alternative)												

Intersection #5 San Juan Grade / Penzance												
Cycle (sec): 100					Critical Vol./Ca				p.(X): 0.816			
Loss Time (sec): 9					Averag	e Dela	ay (se	c/veh)	: 25.1			
Optimal Cycle: 72					Level	Of Service:					С	
Cycle (sec): 100 Critical Vol./Cap.(X): 0.816 Loss Time (sec): 9 Average Delay (sec/veh): 25.1 Optimal Cycle: 72 Level Of Service: C ***********************************												
Street Name:		n Grad	de		Penzance					40		
Approach:	NOTTH B	ouna	501	ith Bo	ouna	E E	ast BC	una	, We	est BC	ouna	
Movement:		- K	1 — — — -	_ 4	- K	1	- I	- K		1 T	- K	
Control:	Protec	Protected			1	Permit	ted	Permitted				
Rights:	Protected Include 6 6 6		Include				Inclu	ide	Include			
Min. Green:	6 6	6	6	6	0	6	6	6	- 6	6	0	
Y+R:	4.0 4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Y+R: Lanes:	1 0 0	1 0	1 (0 0	1 0	0 (1!	0 0	0 (1!	0 0	
Volume Module	e:								12.			
Base Vol:	1 577 1.00 1.00	118	8	608	0	1	7	5	168	7	10	
Growth Adj: Initial Bse:	1.00 1.00	118	1.00	1.00			1.00		1.00	1.00	1.00	
			, 8 1 00	1.00	0 1.00	_	1.00			1.00		
User Adj: PHF Adj:	0 72 0 72	0.72			0.72		0.72					
PHF Volume:	1 801	164			0.72						14	
Reduct Vol:			0	0	O ₁	0	0	0	0	0	0	
Reduced Vol:			11	844	.0	ĺ	10	7	233	10	14	
PCE Adj:					1.00					1.00	1.00	
MLF Adj:					1.00			1.00		1.00		
FinalVolume:	1 801	164	11	844	0	1	10	7	233	10	14	
Saturation F			1000	1000	1000	1000	1000	1000	1000	1000	1000	
Sat/Lane: Adjustment:					1.00					1900		
Lanes:	1 00 0.93	0.93			0.00					0.72		
Final Sat.:					. 0						74	
Capacity Anal						-		'				
Vol/Sat:	0.00 0.55	0.55		0.47	0.00	0.01	0.01	0.01	0.19	0.19	0.19	
Crit Moves:	****		****							****		
Green/Cycle:					0.00						0.22	
Volume/Cap:	0.01 0.87			0.76	0.00		0.05	0.05		0.87	0.87	
Uniform Del:				14.0	0.0		30.9	30.9		37.7	37.7	
IncremntDel:	0.0 7.3		0.5	3.2	0.0	0.1	0.1	0.1		22.5	22.5	
<pre>InitQueuDel: Delay Adj:</pre>	0.0 0.0		1 0.0	0.0	0.0	0.0	0.0	0.0	1 0.0	0.0	0.0	
Delay/Veh:	42.5 22.3			17.2	0.00		30.9	30.9		1.00	1.00	
User DelAdj:				1.00	1.00		1.00	1.00		1.00	1.00	
AdjDel/Veh:	42.5 22.3			17.2	0.0		30.9	30.9		60.2	60.2	
LOS by Move:	D C		D	В	A	C	C	C	E	E	E	
HCM2kAvgQ:	0 27		0	20	0	0	0	0	11	11	11	

Note: Queue reported is the number of cars per lane.												

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to HIGGINS ASSOC., GILROY

~ MITIGATION Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ****************** Intersection #5 San Juan Grade / Penzance ******************** Cycle (sec): 100 Critical Vol./Cap.(X): 0.522
Loss Time (sec): 9 Average Delay (sec/veh): 13.8
Optimal Cycle: 35 Level Of Service: B ********************** Street Name: San Juan Grade Penzance
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R
 Control:
 Protected
 Protected
 Permitted
 Permitted

 Rights:
 Include
 Include
 Include
 Include

 Min. Green:
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6
 6 Volume Module: PHF Volume: 11 551 115 15 662 4 1 6 4 110 8 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 11 551 115 15 662 4 1 6 4 110 8 0 8 18 FinalVolume: 11 551 115 15 662 4 1 6 4 110 8 18 _____| ____| ____| ____| ____| ____| ____| ____| ____| ____| ____| ____| Saturation Flow Module: Capacity Analysis Module: Vol/Sat: 0.01 0.37 0.37 0.01 0.37 0.01 0.01 0.01 0.10 0.10 Crit Moves: **** Green/Cycle: 0.06 0.63 0.63 0.10 0.67 0.67 0.18 0.18 0.18 0.18 0.18 0.18 Uniform Del: 44.5 10.8 10.8 40.8 8.5 8.5 34.1 34.1 34.1 37.6 37.6 37.6 0.8 0.2 0.6 0.6 0.0 0.0 0.0 IncremntDel: 0.4 0.8 2.7 2.7 2.7 Delay/Veh: 44.9 11.6 11.6 41.0 9.0 9.0 34.2 34.2 34.2 40.3 40.3 40.3 AdjDel/Veh: 44.9 11.6 11.6 41.0 9.0 9.0 34.2 34.2 34.2 40.3 40.3 40.3 LOS by Move: D B B D A A C C C D D D HCM2kAvgQ: 0 12 12 0 11 11 0 0 0 5 5 5 ******************** Note: Queue reported is the number of cars per lane. **********************

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to HIGGINS ASSOC., GILROY

nortapiting fur Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************* Intersection #5 San Juan Grade / Penzance ************************* Cycle (sec): 100 Critical Vol./Cap.(X): 0.464 Loss Time (sec): 9 Average Delay (sec/veh): 11.1 Optimal Cycle: 32 Level Of Service: B Cycle (sec): ************************ Street Name: San Juan Grade Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----||----||-----||-----| Control: Protected Protected Permitted Permitted Rights: Include Include Include Include Min. Green: 6 6 6 6 6 6 6 6 6 6 6 6 Volume Module: Base Vol: 10 422 86 11 404 2 5 4 4 61 PHF Adj: 0.00 111 PHF Volume: 13 528 Poduct Vol: 0 0 14 505 5 3 6 5 76 5 0 0 0 0 0 0 0 14 505 5 3 6 5 76 5 108 0 0 Reduced Vol: 13 528 108 FinalVolume: 13 528 108 14 505 5 3 6 5 76 5 8 Saturation Flow Module: Adjustment: 0.93 0.96 0.96 0.92 0.97 0.97 0.91 0.91 0.91 0.73 0.73 0.73 Lanes: 1.00 0.83 0.17 1.00 0.99 0.01 0.18 0.46 0.36 0.86 0.06 0.08 Final Sat.: 1769 1508 307 1753 1825 18 316 789 631 1188 78 117 Capacity Analysis Module: Vol/Sat: 0.01 0.35 0.35 0.01 0.28 0.28 0.01 0.01 0.01 0.06 0.06 0.06 Crit Moves: **** **** Green/Cycle: 0.14 0.72 0.72 0.06 0.64 0.64 0.13 0.13 0.13 0.13 0.13 0.13 Volume/Cap: 0.05 0.49 0.49 0.13 0.43 0.43 0.06 0.06 0.06 0.49 0.49 0.49 Uniform Del: 37.4 6.1 6.1 44.5 9.0 9.0 38.0 38.0 38.0 40.3 40.3 40.3 IncremntDel: 0.1 0.3 0.3 0.6 0.3 0.3 0.1 0.1 0.1 2.0 2.0 Delay/Veh: 37.4 6.4 6.4 45.1 9.2 9.2 38.1 38.1 38.1 42.3 42.3 42.3 AdjDel/Veh: 37.4 6.4 6.4 45.1 9.2 9.2 38.1 38.1 38.1 42.3 42.3 42.3 LOS by Move: D A A D A A D D D D D D HCM2kAvqQ: 0 9 9 1 8 8 0 0 0 3 3 3 HCM2kAvgQ: 3 ********************** Note: Queue reported is the number of cars per lane. *************************

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to HIGGINS ASSOC., GILROY

Tue Jun 21, 2011 15:14:49 GPBO AM Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************** Intersection #5 San Juan Grade / Penzance ************************* Average Delay (sec/veh): 15.4 Worst Case Level Of Service: F[132.6] ************************* Street Name: San Juan Grade Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----| Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include Lanes: 0 0 1! 0 0 1 0 0 0 0 1! 0 0 0 0 1! 0 0 Volume Module: Base Vol: 1 590 118 8 700 0 1 7 168 Initial Bse: 1 590 118 8 700 0 1 7 5 168 7 PHF Volume: 1 702 140 10 833 0 1 8 6 200 8 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 1 702 140 10 833 0 1 8 6 200 8 12 0 12 Critical Gap Module: Critical Gp: 4.1 xxxx xxxxx 4.1 xxxx xxxxx 7.1 6.5 6.2 7.1 6.5 FollowUpTim: 2.2 xxxx xxxxx 2.2 xxxx xxxxx 3.5 4.0 3.3 3.5 4.0 Capacity Module: Cnflict Vol: 833 xxxx xxxxx 843 xxxx xxxxx 1638 1698 833 1635 1627 773 Potent Cap.: 787 xxxx xxxxx 785 xxxx xxxxx 81 93 371 82 103 402 Move Cap.: 787 xxxx xxxxx 785 xxxx xxxxx 73 92 371 74 102 402 Total Cap: xxxx xxxx xxxx xxxx xxxx xxxx 196 212 xxxxx 199 225 xxxxx Volume/Cap: 0.00 xxxx xxxx 0.01 xxxx xxxx 0.01 0.04 0.02 1.01 0.04 0.03 Level Of Service Module: LT - LTR - RT Movement: Shared Cap.: xxxx xxxx xxxxx xxxx xxxx xxxx 252 xxxxx xxxx 205 xxxxx SharedQueue:xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.2 xxxxx xxxxx 10.1 xxxxx Shrd ConDel:xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 20.2 xxxxx xxxxx 133 xxxxx Shared LOS: * * * * * * C * * F xxxxxx ApproachDel: 20.2 132.6 XXXXXX

Note: Queue reported is the number of cars per lane. *************************

С

*

ApproachLOS:

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************** Intersection #5 San Juan Grade / Penzance ******************************* Average Delay (sec/veh): 2.9 Worst Case Level Of Service: D[34.8] Street Name: San Juan Grade Penzance Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include Rights: Include Include Include Include Lanes: 0 0 1! 0 0 1 0 0 1 0 0 0 1! 0 0 0 0 1! 0 0 Volume Module: Base Vol: 9 590 97 13 680 3 1 - 5 92 Initial Bse: 9 590 97 13 680 3 1 5 3 92 7 PHF Volume: 10 641 105 14 739 3 1 5 3 100 8 16 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 10 641 105 14 739 3 1 5 3 100 8 16 Critical Gap Module: Critical Gp: 4.2 xxxx xxxxx 4.1 xxxx xxxxx 7.1 6.5 6.2 7.1 6.5 6.2 FollowUpTim: 2.3 xxxx xxxxx 2.2 xxxx xxxxx 3.5 4.0 3.3 3.5 4.0 Capacity Module: Cnflict Vol: 742 xxxx xxxxx 747 xxxx xxxxx 1495 1535 741 1487 1484 694 Potent Cap.: 847 xxxx xxxxx 853 xxxx xxxxx 102 117 420 104 126 Move Cap.: 847 xxxx xxxxx 853 xxxx xxxxx 92 114 420 97 122 446 Total Cap: xxxx xxxx xxxx xxxx xxxx xxxx 217 235 xxxxx 224 245 xxxxx Volume/Cap: 0.01 xxxx xxxx 0.02 xxxx xxxx 0.01 0.02 0.01 0.45 0.03 0.04 Level Of Service Module: LOS by Move: A * * A * * * * * * * LT - LTR - RT Movement: SharedQueue:xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 0.1 xxxxx xxxxx 2.7 xxxxx Shrd ConDel:xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 18.7 xxxxx xxxxx 34.8 xxxxx Shared LOS: * * * * * * * C * * D ApproachDel: xxxxx ApproachLOS: * XXXXXX. 18.7 34.8 С ******************************* Note: Queue reported is the number of cars per lane. *******************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************ Intersection #5 San Juan Grade / Penzance ************************** Average Delay (sec/veh): 1.7 Worst Case Level Of Service: D[25.6] ************************* Street Name:

Approach:

North Bound

South Bound

Movement:

L - T - R

L - T - R

Penzance

Penzance

Penzance

L - T - R

L - T - R

L - T - R Control: Uncontrolled Uncontrolled Stop Sign Stop Sign Rights: Include Include Include Include 0 0 1! 0 0 1 0 0 1 0 0 0 1! 0 0 0 0 1! 0 0 Lanes: Volume Module: Base Vol: 10 470 86 11 670 2 Initial Bse: 10 470 86 4 2 5 4 11 670 61 4 Critical Gap Module: Critical Gp: 4.2 xxxx xxxxx 4.1 xxxx xxxxx 7.1 6.5 6.2 6.2 7.1 6.5 FollowUpTim: 2.3 xxxx xxxxx 2.2 xxxx xxxxx 3.5 4.0 3.3 3.5 4.0 Capacity Module: Cnflict Vol: 757 xxxx xxxxx 625 xxxx xxxxx 1384 1427 Potent Cap.: 836 xxxx xxxxx 947 xxxx xxxxx 122 136 755 1384 1381 576 412 122 145 Move Cap.: 836 xxxx xxxxx 947 xxxx xxxxx 115 133 412 115 142 520 Total Cap: xxxx xxxx xxxx xxxx xxxx xxxx 243 255 xxxxx 240 262 xxxxx Volume/Cap: 0.01 xxxx xxxx 0.01 xxxx xxxx 0.01 0.02 0.01 0.29 0.02 0.01 Level Of Service Module: LOS by Move: A * * A * * * * * * * LT - LTR - RT Movement: SharedQueue:xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.1 xxxxx xxxxx 1.3 xxxxx Shrd ConDel:xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 17.8 xxxxx xxxxx 25.6 xxxxx Shared LOS: * * * * * * * C * * D xxxxxx 17.8 ApproachDel: 25.6 XXXXXX ApproachLOS: С ****************************** Note: Queue reported is the number of cars per lane.

APPENDIX H INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

San Juan Grade Road/Rogge Road

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ******************************* Intersection #6 San Juan Grade / Rogge *********************** Cycle (sec): 100 Critical Vol./Cap.(X): 0.729 Loss Time (sec): 0
Optimal Cycle: 0 Average Delay (sec/veh): 18.3 Level Of Service: San Juan Grade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----|-----|------|------|
 Control:
 Stop Sign
 Stop Sign
 Stop Sign
 Stop Sign

 Rights:
 Include
 Include
 Include

 Min. Green:
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 -----||-----||-----||-----| Volume Module: Base Vol: 1 123 317 67 221 3 8 22 8 278 Initial Bse: 1 123 67 221 317 8 22 8 3 278 6 39 PHF Volume: 1 152 391 83 273 4 10 27 10 343 7 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 4 10 27 10 Reduced Vol: 1 152 391 83 273 343 7 FinalVolume: 1 152 391 83 273 4 10 27 10 343 7 48 -----|----|-----|-----| Saturation Flow Module: Lanes: 0.01 0.99 1.00 0.46 1.52 0.02 0.21 0.58 0.21 1.00 0.13 0.87 Final Sat.: 4 514 582 221 744 10 88 241 88 471 72 470 -----|-----||------| Capacity Analysis Module: Vol/Sat: 0.30 0.30 0.67 0.37 0.37 0.36 0.11 0.11 0.11 0.73 0.10 0.10 Crit Moves: **** **** Delay/Veh: 12.2 12.2 19.6 14.0 13.7 13.4 11.7 11.7 11.7 26.7 9.8 9.8 AdjDel/Veh: 12.2 12.2 19.6 14.0 13.7 13.4 11.7 11.7 11.7 26.7 9.8 9.8 LOS by Move: B B C B B B B B D A 13.7 ApproachDel: 17.5 11.7 24.4 Delay Adj: Delay Adj: 1.00 ApprAdjDel: 17.5 LOS by Appr: C 1.00 1.00 1.00 13.7 11.7 24.4 В В С AllWayAvgQ: 0.4 0.4 1.8 0.5 0.5 0.5 0.1 0.1 0.1 2.2 0.1 ****************************** Note: Queue reported is the number of cars per lane. *******************

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) Intersection #6 San Juan Grade / Rogge Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 0
Optimal Cycle: 0 Average Delay (sec/veh): Level Of Service: 12.0 ****************************** San Juan Grade Street Name: Roaae Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----| Control: Stop Sign Stop Sign Stop Sign Stop Sign Rights: Include Include Include Include Min. Green: 0 0 0 0 0 0 0 0 0 0 Lanes: 0 1 0 0 1 0 1 0 1 0 0 0 1! 0 0 1 0 0 1 0 _____|__|__| Volume Module: Base Vol: 4 138 195 36 205 1 13 7 207 Initial Bse: 4 138 195 36 205 1 13 7 4 207 2 PHF Volume: 5 164 232 43 244 1 15 8 5 246 2 57 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 5 164 232 43 244 1 15 8 5 246 2 FinalVolume: 5 164 232 43 244 1 15 8 5 246 2 57 -----| Saturation Flow Module: Lanes: 0.03 0.97 1.00 0.30 1.69 0.01 0.54 0.29 0.17 1.00 0.04 0.96 Final Sat.: 16 557 647 163 943 5 263 142 81 505 24 576 Capacity Analysis Module: Vol/Sat: 0.30 0.30 0.36 0.26 0.26 0.26 0.06 0.06 0.06 0.49 0.10 0.10 Crit Moves: **** *** **** Delay/Veh: 11.2 11.2 10.9 11.2 11.0 10.9 10.2 10.2 10.2 15.5 9.0 AdjDel/Veh: 11.2 11.2 10.9 11.2 11.0 10.9 10.2 10.2 10.2 15.5 9.0 LOS by Move: B B в в в B 11.0 1.00 В в в в C A ApproachDel: 11.0 Delay Adj: 1.00 10.2 14.3 Delay Adj: ApprAdjDel: 1.00 1.00 ApprAdjDel: 11.0 LOS by Appr: B 11.0 10.2 14.3 В В В AllWayAvgQ: 0.4 0.4 0.5 0.3 0.3 0.3 0.1 0.1 0.1 0.8 0.1 ********************************* Note: Queue reported is the number of cars per lane. **************************************

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ************************* Intersection #6 San Juan Grade / Rogge ************************ Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 0
Optimal Cycle: 0 Average Delay (sec/veh): 10.7 Level Of Service: *********************************** San Juan Grade Street Name: Roaae Approach: North Bound South Bound East Bound West Bound L-T-R L-T-R L-T-RMovement: _____|
 Control:
 Stop Sign
 Rights:
 Include
 Include< Volume Module: Base Vol: 7 183 109 43 202 2 10 4 10 146 27 Initial Bse: 7 183 43 202 2 10 4 10 109 146 0 PHF Volume: 8 220 131 52 243 2 12 5 12 176 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 8 220 131 52 243 2 12 5 12 Reduced Vol: 176 FinalVolume: 8 220 131 52 243 2 12 5 12 176 0 Saturation Flow Module: Lanes: 0.04 0.96 1.00 0.35 1.63 0.02 0.41 0.17 0.42 1.00 0.00 1.00 Final Sat.: 23 595 700 206 987 10 217 87 217 516 0 620 -----| Capacity Analysis Module: Vol/Sat: 0.37 0.37 0.19 0.25 0.25 0.24 0.06 0.06 0.06 0.34 xxxx 0.05 **** *** **** Crit Moves: Delay/Veh: 11.5 11.5 8.8 10.5 10.3 10.2 9.7 9.7 9.7 12.5 0.0 AdjDel/Veh: 11.5 11.5 8.8 10.5 10.3 10.2 9.7 9.7 9.7 12.5 0.0 8.4 Α A A A В LOS by Move: B B в в B * 10.5 9.7 ApproachDel: 10.3 11.9 Delay Adj: 1.00 1.00 1.00 1.00 ApprAdjDel: 10.5 LOS by Appr: B 10.3 9.7 11.9 В A В AllWayAvgQ: 0.5 0.5 0.2 0.3 0.3 0.0 0.0 0.0 0.5 0.0 ********************************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ******************* Intersection #6 San Juan Grade / Rogge Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 0
Optimal Cycle: 0 Average Delay (sec/veh): 133.6 Level Of Service: San Juan Grade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----|
 Control:
 Stop Sign
 Rights:
 Include
 Include< -----|----|-----|------| Volume Module: Base Vol: 1 123 483 76 221 3 8 40 8 389 47 18 Initial Bse: 1 123 76 221 483 3 8 40 8 389 18 PHF Volume: 1 184 721 113 330 4 12 60 12 581 27 Reduct Vol: 0 0 0 0 0 0 Reduced Vol: 1 184 721 113 330 0 0 0 0 0 0 4 12 60 12 581 27 70 -----| Saturation Flow Module: Lanes: 0.01 0.99 1.00 0.51 1.47 0.02 0.14 0.72 0.14 1.00 0.28 0.72 Final Sat.: 4 460 511 216 643 9 55 274 55 432 135 -----| Capacity Analysis Module: Vol/Sat: 0.40 0.40 1.41 0.52 0.51 0.51 0.22 0.22 0.22 1.35 0.20 0.20 **** Crit Moves: **** Delay/Veh: 15.5 15.5 216.5 19.9 19.2 18.9 14.7 14.7 14.7 194.0 11.8 11.8 AdjDel/Veh: 15.5 15.5 216.5 19.9 19.2 18.9 14.7 14.7 14.7 194.0 11.8 11.8 LOS by Move: C C F C CB B B F B ApproachDel: 175.4 19.4 14.7 167.9 Delay Adj: 1.00 1.00 1.00 1.00 ApprAdjDel: 175.4 LOS by Appr: F 19.4 14.7 167.9 C В AllWayAvgQ: 0.6 0.6 29.3 1.0 1.0 1.0 0.3 0.3 0.3 21.9 0.2 ************************** Note: Queue reported is the number of cars per lane. **********************

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ************************* Intersection #6 San Juan Grade / Rogge ****************************** 100 Cycle (sec): Critical Vol./Cap.(X): Loss Time (sec): 0 Average Delay (sec/veh): Optimal Cycle: 0 Level Of Service: 23.0 ******************************** San Juan Grade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R
 Control:
 Stop Sign
 Include
 Incl -----| Volume Module: Base Vol: 4 138 264 41 205 1 13 14 4 310 13 Initial Bse: 4 138 264 41 205 1 13 14 4 310 13 PHF Volume: 5 184 352 55 273 1 17 19 5 413 17 73 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 5 184 352 55 273 1 17 19 5 413 17 73 FinalVolume: 5 184 352 55 273 1 `17 19 5 413 17 73 Saturation Flow Module: Lanes: 0.03 0.97 1.00 0.33 1.66 0.01 0.42 0.45 0.13 1.00 0.19 0.81 Final Sat.: 14 481 553 154 781 4 173 186 53 474 104 439 _____| Capacity Analysis Module: Vol/Sat: 0.38 0.38 0.64 0.35 0.35 0.35 0.10 0.10 0.10 0.87 0.17 0.17 **** **** **** **** Crit Moves: Delay/Veh: 13.9 13.9 19.0 14.0 13.8 13.7 11.8 11.8 11.8 41.7 10.4 10.4 AdjDel/Veh: 13.9 13.9 19.0 14.0 13.8 13.7 11.8 11.8 11.8 41.7 10.4 10.4 LOS by Move: B B C B B в в в в E B 13.8 17.2 11.8 ApproachDel: 36.1 Delay Adj: 1.00 1.00 1.00 ApprAdjDel: 17.2 LOS by Appr: C 13.8 11.8 36.1 В В E AllWayAvgQ: 0.6 0.6 1.5 0.5 0.5 0.5 0.1 0.1 0.1 4.2 0.2 ************************* Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) *********************** Intersection #6 San Juan Grade / Rogge ************************ Cycle (sec): 100 Critical Vol./Cap.(X): .Loss Time (sec): 0
Optimal Cycle: 0 Average Delay (sec/veh): Level Of Service: ******************************** San Juan Grade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----| Control: Stop Sign Stop Sign Stop Sign Stop Sign Rights: Include Include Include Include Min. Green: 0 0 0 0 0 0 0 0 0 0

 Rights:
 Include
 Include
 Include
 Include

 Min. Green:
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0 ------||-----||------| Volume Module: Base Vol: 7 183 149 46 202 2 10 8 10 197 Initial Bse: 7 183 149 46 202 2 10 8 10 197 5 31 PHF Volume: 9 247 201 62 273 3 14 11 14 266 7 42 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 9 247 201 62 273 3 14 11 14 266 7 42 FinalVolume: 9 247 201 62 273 3 14 11 14 266 7 Saturation Flow Module: Lanes: 0.04 0.96 1.00 0.37 1.61 0.02 0.36 0.28 0.36 1.00 0.14 0.86 Final Sat.: 21 541 631 196 875 9 167 133 167 493 80 494 -----| Capacity Analysis Module: Vol/Sat: 0.46 0.46 0.32 0.32 0.31 0.31 0.08 0.08 0.08 0.54 0.08 0.08 Crit Moves: **** *** **** Delay/Veh: 13.8 13.8 10.7 12.1 11.9 11.7 10.6 10.6 10.6 17.2 9.2 AdjDel/Veh: 13.8 13.8 10.7 12.1 11.9 11.7 10.6 10.6 10.6 17.2 9.2 9.2 LOS by Move: B B B B B B B 11.9 в в В В C A Α ApproachDel: 12.4 10.6 16.0 Delay Adj: 1.00 1.00 1.00 11.9 ApprAdjDel: 12.4 LOS by Appr: B 10.6 16.0 В В C AllWayAvgQ: 0.8 0.8 0.4 0.4 0.4 0.4 0.1 0.1 1.0 0.1 ****************************** Note: Queue reported is the number of cars per lane. *****************************

multigation Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ****************** Intersection #6 San Juan Grade / Rogge ****************************** Cycle (sec): 100 Critical Vol./Cap.(X): Loss Time (sec): 12 Average Delay (sec/veh): Optimal Cycle: 54 Level Of Service: Critical Vol./Cap.(X): 0.659 *********************************** Street Name: San Juan Grade Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Protected Protected Split Phase Split Phase Ovl Include Include Include 6 6 6 6 6 6 6 6 6 6 6 Control: Rights: Volume Module: FinalVolume: 1 184 721 113 330 4 12 60 12 581 27 70 Saturation Flow Module: Adjustment: 0.91 0.96 0.82 0.87 0.92 0.92 0.94 0.94 0.94 0.91 0.91 0.91 Lanes: 1.00 1.00 1.00 1.00 0.99 0.01 0.14 0.72 0.14 1.75 0.07 0.18 Final Sat.: 1736 1828 1554 1655 1716 23 254 1272 254 3018 120 312 Capacity Analysis Module: Vol/Sat: 0.00 0.10 0.46 0.07 0.19 0.19 0.05 0.05 0.05 0.19 0.22 0.22 Crit Moves: **** **** Crit Moves: Green/Cycle: 0.11 0.36 0.70 0.10 0.36 0.36 0.07 0.07 0.07 0.34 0.34 0.34 Volume/Cap: 0.01 0.28 0.66 0.66 0.54 0.54 0.66 0.66 0.66 0.66 0.66 Uniform Del: 39.5 22.5 8.1 43.1 25.6 25.6 45.2 45.2 45.2 26.9 28.0 28.0 Delay/Veh: 39.5 22.7 9.6 52.1 26.6 26.6 57.3 57.3 57.3 27.5 29.6 29.6 AdjDel/Veh: 39.5 22.7 9.6 52.1 26.6 26.6 57.3 57.3 57.3 27.5 29.6 29.6 LOS by Move: D C A D C C E E E C C C HCM2kAvgQ: 0 4 13 5 9 9 4 4 4 9 11 11 ************************** Note: Queue reported is the number of cars per lane.

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to HIGGINS ASSOC., GILROY

W/MI+1924ion									
Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative)									

Intersection #6 San Juan Grade / Rogge									
Loss Time (sec): 12				erage Dela	ay (sec/	veh):	: 21.2		
Cycle (sec): 100									
Street Name:	San Juan Grade Rogge North Bound South Bound East Bound West Bound								
		d Son	uth_Bound	i Ea	ast_Bour	ıd	West B	ound	
Movement:	L - T -	R L	- T -	R L	- T -	R	L - T	- R	
Control	Protostod		rotostod	حد ا ا	 li+ Dbac	-	Cn1i+ D		
Rights:	Ovi	Ρ.	Include		IIC PHAS	e	Split Phase Include 6 6 4.0 4.0 4.0 1 0 1! 0 0		
Min. Green:	6 6	6 6	6	6 6	6	6	6 6	6	
Y+R:	4.0 4.0	4.0 4.0	4.0	1.0 4.0	4.0	4.0	4.0 4.0	4.0	
Lanes:	1 0 1 0	1 1	0 0 1	0 0 0	0 1! 0	0	1 0 1!	0 0	
						-			
Volume Module	e:								
	4 138			1 13				55	
Growth Adj:							00 1.00		
Initial Bse:		264 41		1 13		4			
User Adj: PHF Adj:							1.00 1.00).75 0.75		
PHF Volume:		352 55		1 17		5	413 17		
Reduct Vol:		0 0	0	0 0			0 0		
Reduced Vol:	5 184	352 55	273	. 1 17	19	5	413 17	73	
PCE Adj:	1.00 1.00 1				1.00 1		.00 1.00		
MLF Adj:				.00 1.00			1.00 1.00		
FinalVolume:				1 17					
		000 1000	1000 10	1000	1000 1	000 1	000 1000	1000	
Adjustment:	1900 1900 1			900 1900 .94 0.96			1900 1900 0.88 0.88		
Lanes:			0.99 0.		0.45		L.69 0.06		
Final Sat.:	1702 1792 1	523 1702	1781	9 767	826	236 2	2830 97	412	
	- ⁻					-			
Final Sat.: 1702 1792 1523 1702 1781 9 767 826 236 2830 97 412									
Vol/Sat:	0.00 0.10 0	.23 0.03		15 0.02		0.02			
Crit Moves:			****		****		***		
	0.06 0.26 0								
Volume/Cap:							0.36 0.44		
Uniform Del: IncremntDel:		7.2 37.2 0.2 0.4		1.8 45.2 0.5 2.2		15.2 2 2.2	20.5 21.3 0.2 0.3		
InitQueuDel:		0.0 0.0		0.0 0.0	0.0	0.0	0.0 0.0		
Delay Adj:	104						1.00 1.00		
Delay/Veh:							20.7 21.6		
User DelAdj:							1.00 1.00		
AdjDel/Veh:							20.7 21.6		
LOS by Move:	D C	A D		C D	D .	D	C C		
HCM2kAvgQ:	0 5	5 2		7 2	2	2	5 7		

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to HIGGINS ASSOC., GILROY

Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************* Intersection #6 San Juan Grade / Rogge ******************** Cycle (sec): 100 Critical Vol./Cap.(X): 0.319 Loss Time (sec): 12 Average Delay (sec/veh): Optimal Cycle: 36 Level Of Service: ******* Street Name: San Juan Grade Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Protected Protected Split Phase Split Phase Rights: Ovl Include Include Include Min. Green: 6 6 6 6 6 6 6 6 6 6 6 6 Volume Module: Base Vol: 7 183 149 46 202 10 8 10 2 197 5 Initial Bse: 7 183 User Adj: PHF Adj: PHF Volume: 9 247 Reduct Vol: 0 0 Reduced Vol: 9 247 62 273 3 14 11 14 266 0 0 0 0 0 0 0 62 273 3 14 11 14 266 201 266 7 0 0 0 7 201 42 FinalVolume: 9 247 201 62 273 3 14 11 14 266 7 42 Saturation Flow Module: Adjustment: 0.92 0.97 0.83 0.91 0.96 0.96 0.92 0.92 0.92 0.93 0.93 0.93 Lanes: 1.00 1.00 1.00 1.00 0.99 0.01 0.36 0.28 0.36 1.73 0.04 0.23 Final Sat.: 1753 1845 1568 1736 1808 18 622 497 622 3062 66 407 Capacity Analysis Module: Vol/Sat: 0.01 0.13 0.13 0.04 0.15 0.15 0.02 0.02 0.02 0.09 0.10 0.10 Crit Moves: **** **** **** Green/Cycle: 0.06 0.35 0.66 0.16 0.45 0.45 0.06 0.06 0.06 0.31 0.31 Volume/Cap: 0.09 0.38 0.20 0.23 0.34 0.34 0.34 0.34 0.34 0.34 0.34 6.7 36.8 17.9 17.9 44.7 44.7 44.7 26.4 26.8 26.8 Uniform Del: 44.4 24.3 0.1 IncremntDel: 0.4 0.4 0.4 0.2 0.2 1.8 1.8 1.8 0.1 0.2 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay/Veh: 44.8 24.6 6.8 37.2 18.1 18.1 46.5 46.5 46.5 26.5 27.1 27.1 AdjDel/Veh: 44.8 24.6 6.8 37.2 18.1 18.1 46.5 46.5 46.5 26.5 27.1 27.1 LOS by Move: D C A D B B D D D C C C HCM2kAvgQ: 0 6 2 2 5 5 1 1 1 4 4 4 ************************* Note: Queue reported is the number of cars per lane. ******************************

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to HIGGINS ASSOC., GILROY

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ************************* Intersection #6 San Juan Grade / Rogge ******************* Cycle (sec): 100 Critical Vol./Cap.(X):
Loss Time (sec): 0 Average Delay (sec/veh):
Optimal Cycle: 0 Level Of Service: Critical Vol./Cap.(X): 1.003 Average Delay (sec/veh): **************************** San Juan Grade Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: Control: Stop Sign Stop Sign Stop Sign Stop Sign Rights: Include Include Include Include Min. Green: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Lanes: 0 1 0 0 1 0 1 0 1 0 0 0 1! 0 0 1 0 0 1 0 Volume Module: Base Vol: 1 265 370 120 475 3 8 40 8 320 18 Initial Bse: 1 265 370 120 475 3 8 40 8 320 18 User Adj: PHF Adi: 1 331 463 150 594 4 10 50 10 400 23 PHF Volume: 88 0 0 4 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 1 331 150 594 10 50 10 463 400 23 88 4 FinalVolume: 1 331 463 150 594 10 50 10 400 23 _____| | | Saturation Flow Module: Lanes: 0.01 0.99 1.00 0.40 1.59 0.01 0.14 0.72 0.14 1.00 0.20 0.80 Final Sat.: 2 439 483 171 687 4 53 264 53 399 90 350 Capacity Analysis Module: Vol/Sat: 0.75 0.75 0.96 0.88 0.86 0.86 0.19 0.19 0.19 1.00 0.25 0.25 **** **** **** Crit Moves: Delay/Veh: 31.8 31.8 58.1 47.3 44.8 43.3 14.9 14.9 14.9 76.3 13.2 13.2 LOS by Move: D D F E E E B B B F B ApproachDel: 47.1 45.3 14.9 62.7 1.00 1.00 Delay Adj: 1.00
ApprAdjDel: 47.1
LOS by Appr: E 1.00 45.3 E 14.9 62.7 В AllWayAvgQ: 2.6 2.6 6.4 4.3 3.9 3.9 0.2 0.2 0.2 7.2 0.3 ************************* Note: Queue reported is the number of cars per lane. *******************

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ******************** Intersection #6 San Juan Grade / Rogge *********************** Cycle (sec): 100 Critical Vol./Cap.(X):
Loss Time (sec): 0 Average Delay (sec/veh):
Optimal Cycle: 0 Level Of Service: 0.797 23.8 C *********************** San Juan Grade North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: _____| Stop Sign Stop Sign Stop Sign Stop Sign Include Include Include Include Control: Rights: Include Include 0 0 0 0 0 Include 0 0 0 0 0 0 Min. Green: Lanes: 0 1 0 0 1 0 1 0 1 0 0 0 1! 0 0 1 0 0 1 0 Volume Module: Base Vol: 4 330 230 75 490 1 250 13 14 4 Initial Bse: 4 330 230 75 490 1 13 14 4 250 13 User Adi: PHF Adj: 15 16 5 291 1 PHF Volume: 5 384 267 87 570 15 116 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 Reduced Vol: 5 384 267 87 570 1 15 16 5 291 15 116 MLF Adj: FinalVolume: 5 384 267 87 570 1 15 16 5 291 15 Saturation Flow Module: Lanes: 0.01 0.99 1.00 0.26 1.73 0.01 0.42 0.45 0.13 1.00 0.12 0.88 Final Sat.: 6 482 533 127 839 2 161 174 50 424 55 Capacity Analysis Module: Vol/Sat: 0.80 0.80 0.50 0.69 0.68 0.67 0.09 0.09 0.09 0.69 0.27 0.27 Crit Moves: **** *** **** Delay/Veh: 32.3 32.3 15.5 24.3 23.7 23.3 12.6 12.6 12.6 26.5 12.5 12.5 AdjDel/Veh: 32.3 32.3 15.5 24.3 23.7 23.3 12.6 12.6 12.6 26.5 12.5 12.5 LOS by Move: D D C C с в в в D B 25.5 23.8 ApproachDel: 12.6 22.1 1.00 1.00 1.00 Delay Adj: 1.00 ApprAdjDel: 25.5
LOS by Appr: D 23.8 12.6 22.1 С В AllWayAvgQ: 3.0 3.0 0.9 1.9 1.8 1.8 0.1 0.1 0.1 1.8 0.3 *********************

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM 4-Way Stop Method (Base Volume Alternative) ************************* Intersection #6 San Juan Grade / Rogge ******************************* Cycle (sec): 100 Critical Vol./Cap.(X):
Loss Time (sec): 0 Average Delay (sec/veh):
Optimal Cycle: 0 Level Of Service: ************************* San Juan Grade Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RControl:Stop SignStop SignStop SignStop SignRights:IncludeIncludeIncludeMin. Green:0000000 Lanes: 0 1 0 0 1 0 1 0 1 0 0 0 1! 0 0 1 0 0 1 0 Volume Module: Base Vol: 7 410 140 90 450 2 10 8 10 180 5 Initial Bse: 7 410 140 90 450 2 10 8 10 180 5 60 PHF Adj: PHF Volume: 8 482 165 106 529 2 12 9 12 212 6 71 0 = Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 Reduced Vol: 8 482 9 12 212 165 106 529 2 12 71 FinalVolume: 8 482 165 106 529 2 12 9 12 212 6 Saturation Flow Module: Lanes: 0.02 0.98 1.00 0.33 1.66 0.01 0.36 0.28 0.36 1.00 0.08 0.92 Final Sat.: 9 522 580 170 864 4 147 118 147 428 38 455 Capacity Analysis Module: Vol/Sat: 0.92 0.92 0.28 0.62 0.61 0.61 0.08 0.08 0.08 0.50 0.16 0.16 Crit Moves: **** *** **** Delay/Veh: 47.4 47.4 11.0 20.0 19.4 19.0 11.9 11.9 11.9 18.2 11.0 11.0 AdjDel/Veh: 47.4 47.4 11.0 20.0 19.4 19.0 11.9 11.9 11.9 18.2 11.0 11.0 LOS by Move: E E B C C C B B B C B 19.5 ApproachDel: 38.3 11.9 16.3 1.00 1.00 1.00 Delay Adj: 1.00 ApprAdjDel: 38.3
LOS by Appr: E 19.5 C 11.9 16.3 В AllWayAvgQ: 5.6 5.6 0.4 1.5 1.4 1.4 0.1 0.1 0.1 0.9 0.2 Note: Queue reported is the number of cars per lane. ******************************

APPENDIX I INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

Kelton Drive/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #7 Kelton / Rogge ******************** Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B[12.9] ************************************* Kelton Street Name: Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: -----|----|-----| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 1 0 0 1 0 0 Volume Module: 2 0 19 0 0 0 Base Vol: 0 392 0 11 321 Initial Bse: 2 0 19 0 0 0 0 392 0 11 321 0 PHF Volume: 3 0 Reduct Vol: 0 0 27 0 0 0 0 552 0 15 452 0 FinalVolume: 3 0 0 0 0 0 0 0 0 0 0 27 0 0 0 0 552 0 15 452 -----| Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxx xxxx xxxx xxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 1035 1035 552 xxxx xxxx xxxx xxxx xxxx 552 xxxx xxxxx Potent Cap.: 259 234 -----| Level Of Service Module: LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT Shared LOS: * B * * * * * * * * 12.9 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ******************************* Note: Queue reported is the number of cars per lane. ***********************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************************* Intersection #7 Kelton / Rogge ****************************** Average Delay (sec/veh): 0.7 Worst Case Level Of Service: B[12.1] ************************************ Kelton Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RStop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Volume Module: 5 0 11 0 0 0 237 4 Base Vol: 0 22 274 Initial Bse: 5 0 11 0 0 0 0 237 4 22 274 0 PHF Volume: 7 0 16
Reduct Vol: 0 0 0
FinalVolume: 7 0 16 0 0 0 343 6 32 397 0 0 0 0 0 0 0 0 0 0 0 343 6 0 0 32 397 0 -----| Critical Gap Module: _____ Capacity Module: Cnflict Vol: 807 807 Potent Cap.: 353 317 Level Of Service Module: LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT Shared LOS: * B * * * * * * * * * ApproachDel: 12.1
ApproachLOS: B XXXXXX XXXXXX XXXXXX ************************************* Note: Queue reported is the number of cars per lane. ******************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************** Intersection #7 Kelton / Rogge ************************* Average Delay (sec/veh): 0.5 Worst Case Level Of Service: A[9.7] ***** Kelton Street Name: Rogge Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R L - T - R Movement: _____| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 1 0 1 0 0 _____| Volume Module: Base Vol: 1 0 11 0 0 7 14 179 0 202 Initial Bse: 1 0 11 0 0 0 0 202 7 14 179 0 0 0 230 8 16 203 PHF Volume: 1 0 13 0 0 Reduct Vol: 0 0 0 0 0 0 FinalVolume: 1 0 13 0 0 0 0 0 0 0 0 0 0 230 8 16 203 -----| Critical Gap Module: -----| Capacity Module: Cnflict Vol: 469 469 Level Of Service Module: LOS by Move: * * * * * * * * LT - LTR - RT Movement: Shared LOS: * A * * * * * * * * * * 9.7 XXXXXX ApproachDel: XXXXXX ApproachLOS: ************************** Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************************** Intersection #7 Kelton / Rogge Average Delay (sec/veh): 0.6 Worst Case Level Of Service: C[22.4] ********************************* Kelton Street Name: Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: -----|-----|------| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 -----| Volume Module: 2 0 22 Base Vol: 0 0 0 0 588 0 13 452 0 Initial Bse: 2 0 22 0 0 0 0 588 0 13 452 0 PHF Volume: 3 0 37
Reduct Vol: 0 0 0
FinalVolume: 3 0 37 0 0 0 0 997 0 22 766 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 997 22 766 -----| Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxx xxxx xxxx 2.2 xxxx xxxxx -----| Capacity Module: Cnflict Vol: 1807 1807 299 xxxx xxxx xxxxx xxxx xxxx 686 xxxx xxxxx Potent Cap.: 88 80 86 77 299 xxxx xxxx xxxxx xxxx xxxx xxxxx 686 xxxx xxxxx Move Cap.: -----| Level Of Service Module: LOS by Move: * * * * * * * * B * * Movement: LT - LTR - RT Shared LOS: * C * * * * * * * * * 22.4 ApproachDel: XXXXXX XXXXXX XXXXXX С ApproachLOS: Note: Queue reported is the number of cars per lane. **********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) **************** Intersection #7 Kelton / Rogge ************************** 0.6 Average Delay (sec/veh): Worst Case Level Of Service: C[15.6] *********** Street Name: Kelton Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RVolume Module: Base Vol: 5 0 12 0 0 0 0 318 4 24 395 Initial Bse: 5 0 12 0 0 0 0 318 4 24 395 0
 8
 0
 19
 0
 0
 0
 505
 6
 38
 627
 0

 0
 0
 0
 0
 0
 0
 0
 0
 0
 0

 8
 0
 19
 0
 0
 0
 0
 0
 0
 0
 0

 0
 0
 19
 0
 0
 0
 0
 505
 6
 38
 627
 0
 PHF Volume: 8 0 19 Reduct Vol: 0 0 FinalVolume: 8 0 Critical Gap Module: Capacity Module: Cnflict Vol: 1211 1211 508 xxxx xxxx xxxxx xxxx xxxx 511 xxxx xxxxx Potent Cap.: 203 184 569 xxxx xxxx xxxxx xxxx xxxx xxxx 1029 xxxx xxxxx Move Cap.: 198 177 569 xxxx xxxx xxxxx xxxx xxxx xxxxx 1029 xxxx xxxxx _____| Level Of Service Module: LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT Shared LOS: * C * * * * * * * * * * * xxxxxx ApproachDel: 15.6
ApproachLOS: C XXXXXX XXXXXX ************************* Note: Queue reported is the number of cars per lane. ***************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************** Intersection #7 Kelton / Rogge ******************* Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B[10.7] ************************* Kelton Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RVolume Module: Base Vol: 1 0 12 0 0 0 0 249 7 15 239 Initial Bse: 1 0 12 0 0 0 0 249 7 15 239 0 PHF Volume: 1 0 17 0 0 Reduct Vol: 0 0 0 0 0 0 FinalVolume: 1 0 17 0 0 0 351 10 21 337 0 0 0 0 0 0 0 0 0 0 0 351 10 21 337 0 Critical Gap Module: FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx -----| Capacity Module: Cnflict Vol: 735 735 356 xxxx xxxx xxxxx xxxx xxxx 361 xxxx xxxxx Potent Cap.: 390 350 693 xxxx xxxx xxxxx xxxx xxxx xxxxx 1204 xxxx xxxxx Move Cap.: 385 343 693 xxxx xxxx xxxx xxxx xxxx xxxx 1204 xxxx xxxx Level Of Service Module: LOS by Move: * * * * * * * * A * * LT - LTR - RT Movement: Shared LOS: * B * * * * * * * * * * ApproachDel: 10.7 xxxxxx XXXXXX ApproachLOS: *********************************** Note: Queue reported is the number of cars per lane. *********************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #7 Kelton / Rogge ************************** Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[16.4] ************************* Street Name: Kelton Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RStop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Rights: 0 0 1! 0 0 0 0 0 0 0 0 1 0 0 Lanes: 1 0 1 0 0 _____| Volume Module: 2 0 22 Base Vol: 0 - 0 0 0 520 0 13 405 Initial Bse: 2 0 22 0 0 0 0 520 0 13 405 PHF Adj: PHF Volume: 3 0 33 0 0 776 0 19 604 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 FinalVolume: 3 0 0 0 0 0 33 0 776 19 604 Critical Gap Module: FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 1419 1419 776 xxxx xxxx xxxxx xxxx xxxx xxxxx 776 xxxx xxxxx Move Cap.: Level Of Service Module: 9.4 xxxx xxxxx A * * LOS by Move: * * * * * * * * LT - LTR - RT Movement: Shared LOS: * C * * * * * * * * 16.4 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: С ************************** Note: Queue reported is the number of cars per lane. *****************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ***************************** Intersection #7 Kelton / Rogge ********************* Average Delay (sec/veh): 0.6 Worst Case Level Of Service: B[14.2] ******************************* Street Name: Kelton Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RL - T - R -----||-----| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Include 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 Lanes: 1 0 1 0 0 Volume Module: Base Vol: 5 0 - 12 0 0 0 0 323 24 380 Initial Bse: 5 0 0 323 12 0 0 0 4 24 380 0 PHF Volume: 7 0 17 0 0 0 461 6 34 543 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 FinalVolume: 7 0 17 0 6 0 0 0 461 34 543 Critical Gap Module: Capacity Module: 467 xxxx xxxxx Potent Cap.: 245 221 602 xxxx xxxx xxxxx xxxx xxxx xxxxx 1069 xxxx xxxxx Move Cap.: 239 214 602 xxxx xxxx xxxxx xxxx xxxx 1069 xxxx xxxxx Level Of Service Module: 8.5 xxxx xxxxx LOS by Move: * * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shared LOS: * B * * * * * * * * * * 14.2 ApproachDel: XXXXXX XXXXXX XXXXXX В ApproachLOS: ********************* Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************* Intersection #7 Kelton / Rogge ******************** Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[10.7] ************************* Street Name: Kelton Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Movement: Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Lanes: Volume Module: Base Vol: 1 0 12 0 0 0 0 290 15 330 Initial Bse: 1 0 12 0 0 0 0 290 7 15 330 0 1.00 1.00 1.00 PHF Adj: 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 PHF Volume: 1 0 0 0 0 345 14 0 8 18 393 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 14 0 0 0 FinalVolume: - 1 0 345 8 18 393 -----||-----| Critical Gap Module: Critical Gp: 6.4 6.5 FollowUpTim: 3.5 4.0 -----||----||-----||-----||-----|| Capacity Module: Cnflict Vol: 778 778 Move Cap.: Level Of Service Module: 8.1 xxxx xxxxx * * * * * * * * LOS by Move: LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shared LOS: * B * * * * * * * * * 10.7 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ****************************** Note: Queue reported is the number of cars per lane. ************************************

APPENDIX J INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

Bollenbacher Drive/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************** Intersection #8 Bollenbacher / Rogge ************************* Average Delay (sec/veh): 1.7 Worst Case Level Of Service: B[11.8] ********************************* Bollenbacher Street Name: Rogge Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Stop Sign Include -----|----|-----|------| Volume Module: 0 0 53 Base Vol: 0 0 0 0 261 95 66 232 Initial Bse: 0 0 53 0 0 0 0 261 95 66 232 0 PHF Adj: PHF Volume: 0 0 78 0 0 0 0 384 140 97 341 0 0 === 0 Reduct Vol: 0 0 FinalVolume: 0 0 0__ 0 0 0 0 0 0 0 0 78 0 0 0 0 0 384 140 97 341 -----| Critical Gap Module: Critical Gp:xxxxx xxxx 6.2 xxxxx xxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim:xxxxx xxxx 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: xxxx xxxx 454 XXXX XXXX XXXXX XXXX XXXX 524 xxxx xxxxx -----||-----||-----| Level Of Service Module: 8.8 xxxx xxxxx LOS by Move: * * B * * * * * * A * * Movement: LT - LTR - RT 8.8 xxxx xxxxx Shared LOS: * * * * * * * * A * 11.8 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************** Intersection #8 Bollenbacher / Rogge ********************************* Average Delay (sec/veh): Worst Case Level Of Service: B[11.7] *************************** Bollenbacher Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RVolume Module: 1 0 30 0 0 0 Base Vol: 0 261 95 28 199 Initial Bse: 1 0 30 0 0 0 0 261 95 28 199 0 PHF Volume: 2 0 45 0 0 0 0 395 144 42 302 0 0 Reduct Vol: 0 0 0 0 0 0 0 Ω 0 0 FinalVolume: 2 0 45 0 0 0 0 395 144 42 302 0 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.2 xxxx xxxxx _____| Capacity Module: Cnflict Vol: 854 854 467 xxxx xxxx xxxxx xxxx xxxx 539 xxxx xxxxx Potent Cap.: 332 298 Move Cap.: Level Of Service Module: 0.1 xxxx xxxxx 8.7 XXXX XXXXX A * * LOS by Move: * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT 8.7 xxxx xxxxx A * * Shared LOS: * B * * * * * * * ApproachDel: 11.7
ApproachLOS: B XXXXXX XXXXXX XXXXXX Note: Queue reported is the number of cars per lane. ************************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********************************** Intersection #8 Bollenbacher / Rogge ****************************** 0.7 Average Delay (sec/veh): Worst Case Level Of Service: A[9.3] ******************************* Street Name: Bollenbacher Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RVolume Module: Base Vol: 0 0 11 0 0 0 159 12 21 165 Initial Bse: 0 0 11 0 0 0 159 12 21 165 13 0 185 PHF Volume: 0 0 0 0 0 14 24 192 0 Reduct Vol: 0 0 FinalVolume: 0 0 0 0 0 - 0 0 0 0 - 0 0 0 0 13 0 0 185 14 0 0 24 192 0 -----| Critical Gap Module: _____| ___| ____| ____| ____| | _____| | _____| | ____| | ___| Capacity Module: -----| Level Of Service Module: Control Del:xxxxx xxxx 9.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 7.7 xxxx xxxxx A * * LOS by Move: * * A * * * * * Movement: LT - LTR - RT 7.7 xxxx xxxxx A * * Shared LOS: * * * * * * * * * 9.3 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: A ********************************** Note: Queue reported is the number of cars per lane. **********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) *********************************** Intersection #8 Bollenbacher / Rogge ************************ 1.8 Average Delay (sec/veh): Worst Case Level Of Service: C[19.6] ********************************** Bollenbacher Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Stop Sign Uncontrolled Uncontrolled Include Include Control: Stop Sign Include Lanes: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 0 0 -----| Volume Module: 0 0 56 0 0 0 0 460 95 68 365 Base Vol: Initial Bse: 0 0 56 0 0 0 460 95 68 365 0 PHF Volume: 0 0 98 0 0 0 0 807 167 119 640 0 Reduct Vol: 0 0 FinalVolume: 0 0 0 0 0 0 0 0 0 0 807 167 0 0 0 0 0 0 0 98 119 640 -----| Critical Gap Module: Critical Gp:xxxxx xxxx 6.2 xxxxx xxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim:xxxxx xxxx 3.3 xxxxx xxxx xxxxx xxxxx xxxx xxxxx 2.2 xxxx xxxxx -----| Capacity Module: Cnflict Vol: xxxx xxxx 890 xxxx xxxx xxxxx xxxx xxxx 974 xxxx xxxxx 700 xxxx xxxxx Volume/Cap: xxxx xxxx 0.29 xxxx xxxx xxxx xxxx xxxx 0.17 xxxx xxxx Level Of Service Module: LOS by Move: * * C * * * * * * B * * Movement: LT - LTR - RT Shared LOS: * * * * * * * * B * * ApproachDel: 19.6 XXXXXX XXXXXX XXXXXX ApproachLOS: С Note: Queue reported is the number of cars per lane. ************************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #8 Bollenbacher / Rogge ********************* 0.9 Average Delay (sec/veh): Worst Case Level Of Service: B[13.8] ************************* Street Name: Bollenbacher Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RControl: Stop Sign
Rights: Include Stop Sign Uncontrolled Uncontrolled Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 ------|-----|------||------||------||-----| Volume Module: Base Vol: 1 0 31 0 0 0 343 95 30 322 Initial Bse: 1 0 31 0 0 0 0 343 95 30 322 0 PHF Volume: 2 0 50
Reduct Vol: 0 0 0
FinalVolume: 2 0 50 0 553 0 - 0 0 153 48 519 0 0 0 0 0 0 0 0 0 0 0 553 153 2 0 50 0 0 48 519 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.2 xxxx xxxxx _____| Capacity Module: Cnflict Vol: 1246 1246 630 xxxx xxxx xxxxx xxxx xxxx xxxx 706 xxxx xxxxx 869 xxxx xxxxx Move Cap.: 185 165 485 xxxx xxxx xxxxx xxxx xxxx xxxx 869 xxxx xxxxx Level Of Service Module: 0.2 xxxx xxxxx 9.4 xxxx xxxxx LOS by Move: * * * * * * * * * A * * Movement: LT - LTR - RT LTR - RT LT - LTR - RT LT - LTR - RT Shrd ConDel:xxxxx 13.8 xxxxx xxxxx xxxx xxxxx xxxxx xxxxx 9.4 xxxx xxxxx A * * Shared LOS: * B * * * * * * * 13.8 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В 4 ****************************** Note: Queue reported is the number of cars per lane. ******************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************* Intersection #8 Bollenbacher / Rogge ************************* Average Delay (sec/veh): 0.6 Worst Case Level Of Service: B[10.0] ***************************** Bollenbacher Street Name: Rogge Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Include Lanes: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 0 0 -----| Volume Module: 0 0 12 0 0 0 207 12 Base Vol: 0 22 226 Initial Bse: 0 0 12 0 0 0 0 207 12 22 226 0 0 300 PHF Volume: 0 0 17 17 32 328 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 - 0 0 FinalVolume: 0 0 0 0 300 17 17 0 0 32 328 Critical Gap Module: FollowUpTim:xxxxx xxxx 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx -----|----|-----|------| Capacity Module: Cnflict Vol: xxxx xxxx 309 xxxx xxxx xxxxx xxxx xxxx 317 xxxx xxxxx 736 xxxx xxxx xxxxx xxxx xxxx 1248 xxxx xxxxx Potent Cap.: xxxx xxxx Level Of Service Module: Control Del:xxxxx xxxx 10.0 xxxxx xxxx xxxxx xxxxx xxxxx xxxx 8.0 xxxx xxxxx LOS by Move: * * B * * * * * * Movement: LT - LTR - RT 0.1 xxxx xxxxx 8.0 xxxx xxxx Shared LOS: * * * * * * * * * A * * 10.0 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ************************** Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #8 Bollenbacher / Rogge *********************** Average Delay (sec/veh): 1.6 Worst Case Level Of Service: B[14.7] ******************* Bollenbacher Street Name: Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R Control: Rights: Lanes: Volume Module: 0 0 56 Base Vol: 0 0 0 0 - 392 95 68 318 0 Initial Bse: 0 0 56 0 0 0 0 392 95 68 318 0 0 603 146 105 489 PHF Volume: 0 0 86 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 0 0 86 0 0 0 105 489 0 603 146 _____| Critical Gap Module: Critical Gp:xxxxx xxxx 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim:xxxxx xxxx 3.3 xxxxx xxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx -----||-----||-----||------| Capacity Module: 749 xxxx xxxxx _____ Level Of Service Module: Control Del:xxxxx xxxx 14.7 xxxxx xxxx xxxxx xxxxx xxxxx xxxx 9.8 xxxx xxxxx LOS by Move: * * B * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shared LOS: * * * * * * * * * A * 14.7 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ******************************* Note: Queue reported is the number of cars per lane. *******************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************** Intersection #8 Bollenbacher / Rogge ******************** Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[13.1] ************************** Bollenbacher Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 Lanes: Volume Module: 1 0 Base Vol: 31 0 0 0 348 95 0 30 307 Initial Bse: 1 0 31 0 0 0 0 348 95 30 307 PHF Volume: 1 0 46 0 0 Reduct Vol: 0 0 0 0 0 0 FinalVolume: 1 0 46 0 0 0 0 519 142 45 458 0 0 0 0 0 0 0 0 0 FinalVolume: 1 0 0 519 142 45 458 Critical Gap Module: Critical Gp: 6.4 6.5 FollowUpTim: 3.5 4.0 Capacity Module: Cnflict Vol: 1138 1138 590 xxxx xxxx xxxxx xxxx xxxx 661 xxxx xxxxx Potent Cap.: 225 203 511 xxxx xxxx xxxxx xxxx xxxx xxxxx 904 xxxx xxxxx Move Cap.: 216 193 511 xxxx xxxx xxxxx xxxx xxxx 904 xxxx xxxxx Level Of Service Module: 0.2 xxxx xxxxx 9.2 xxxx xxxxx LOS by Move: * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT 0.2 xxxx xxxxx 9.2 xxxx xxxxx Shared LOS: * B * * * * * * * A * xxxxxx 13.1 ApproachDel: XXXXXX XXXXXX ApproachLOS: В ******************************* Note: Queue reported is the number of cars per lane. **********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #8 Bollenbacher / Rogge ************************* Average Delay (sec/veh): 0.5 Worst Case Level Of Service: A[9.9] ****************************** Street Name: Bollenbacher Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 1 0 0 0 Lanes: Volume Module: Base Vol: 0 0 12 0 0 0 0 240 12 22 320 12 Initial Bse: 0 0 12 0 0 0 0 240 22 320 PHF Adj: PHF Volume: 0 0 14 0 0 0 0 289 14 27 386 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 0 0 14 0 0 289 14 27 386 -----|----|----|-----||------| Critical Gap Module: -----|----|-----| Capacity Module: 304 xxxx xxxxx Level Of Service Module: 0.1 xxxx xxxxx 9.9 XXXXX XXXX XXXXX XXXXX XXXX 8.0 xxxx xxxx LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT 8.0 xxxx xxxxx Shared LOS: * * * * * * * * * A *: 9.9 ApproachDel: XXXXXX XXXXXX XXXXXX Α ApproachLOS: *************************** Note: Queue reported is the number of cars per lane.

APPENDIX K INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

Jasper Way/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #9 Jasper / Rogge ************************ Average Delay (sec/veh): 0.5 Worst Case Level Of Service: B[14.3] ************************ Jasper Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RStop Sign Include Stop Sign Uncontrolled Uncontrolled Include Include Include -----|----|-----|------||------||-----| Volume Module: 14 0 4 0 0 0 Base Vol: 0 290 Initial Bse: 14 0 4 0 0 0 0 290 1 2 265 PHF Volume: 19 0 5 0 0 0 0 397 1 3 363 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 5 0 FinalVolume: 19 0 0 0 0 397 1 3 363 -----| Critical Gap Module: FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx -----| Capacity Module: Cnflict Vol: 766 766 398 xxxx xxxx xxxxx xxxx xxxx 399 xxxx xxxxx -----|-----|------| Level Of Service Module: 0.0 xxxx xxxxx 8.1 xxxx xxxxx LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT SharedQueue:xxxxx 0.2 xxxxx xxxxx xxxx xxxxx xxxxx xxxxx 0.0 xxxx xxxxx 8.1 xxxx xxxxx Shared LOS: * B * * * * * * * ApproachLOS:
******* A * * XXXXXX XXXXXX XXXXXX Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) **************************** Intersection #9 Jasper / Rogge ****************** 0.7 Average Delay (sec/veh): Worst Case Level Of Service: B[13.3] ************************** Jasper Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R_____| Volume Module: Base Vol: 16 0 7 0 0 0 268 21 7 225 Initial Bse: 16 0 7 0 0 0 268 21 7 225 .0 PHF Volume: 22 0
Reduct Vol: 0 0 0 362 28 9 304 0 9 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 22 0 9 28 0 0 0 0 362 9 304 Critical Gap Module: Capacity Module: Cnflict Vol: 699 699 376 xxxx xxxx xxxxx xxxx xxxx xxxx 391 xxxx xxxxx Potent Cap.: 409 366 675 xxxx xxxx xxxxx xxxx xxxx xxxxx 1141 xxxx xxxxx Move Cap.: 406 363 675 xxxx xxxx xxxx xxxx xxxx xxxx 1141 xxxx xxxx Level Of Service Module: 8.2 xxxx xxxxx LOS by Move: * * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shrd ConDel:xxxxx 13.3 xxxxx xxxxx xxxx xxxxx xxxxx xxxxx 8.2 xxxx xxxxx Shared LOS: * B * * * * * * A * * ApproachDel: 13.3
ApproachLOS: B XXXXXX XXXXXX XXXXXX ****************************** Note: Queue reported is the number of cars per lane. ****************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #9 Jasper / Rogge ****************** 0.4 Average Delay (sec/veh): Worst Case Level Of Service: A[9.8] ************************* Jasper Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R_____| Volume Module: Base Vol: 4 0 5 0 0 0 151 6 6 180 Initial Bse: 4 0 5 0 0 0 151 6 6 180 0 0 161 6 6 191 0 0 0 0 PHF Volume: 4 0 5 0 Reduct Vol: 0 0 FinalVolume: 4 0 0 0 0 0 0 0 0 0 5 0 0 6 0 0 161 6 191 O Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Potent Cap.: 636 564 -----| Level Of Service Module: 0.0 xxxx xxxxx 7.6 xxxx xxxxx LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT 7.6 xxxx xxxxx A * * Shared LOS: * A * * * * * * * 9.8 ApproachDel: XXXXXX XXXXXX XXXXXX Α ApproachLOS: *************************** Note: Queue reported is the number of cars per lane. ***************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************************** Intersection #9 Jasper / Rogge 0.9 Average Delay (sec/veh): Worst Case Level Of Service: E[35.4] *********************************** Jasper Street Name: Rogge North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: -----|----|-----|-----| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 Volume Module: 7 14 0 0 0 0 Base Vol: 0 492 1 4 400 Initial Bse: 14 0 7 0 0 0 0 492 1 4 400 0 0 0 PHF Volume: 25 0 12 0 0 863 7 702 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 FinalVolume: 25 0 0 0 12 0 0 863 2 7 702 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 1580 1580 864 xxxx xxxx xxxxx xxxx xxxx 865 xxxx xxxxx Potent Cap.: 121 110 357 XXXX XXXX XXXXX XXXX XXXX 770 xxxx xxxxx Move Cap.: 121 109 357 xxxx xxxx xxxxx xxxx xxxx xxxx 770 xxxx xxxxx Level Of Service Module: 0.0 xxxx xxxx 9.7 xxxx xxxxx LOS by Move: * * * * * * * * * A * * Movement: LT - LTR - RT SharedQueue:xxxxx 0.9 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.0 xxxx xxxxx Shrd ConDel:xxxxx 35.4 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 9.7 xxxx xxxxx Shared LOS: * E * * * * * * * * A * ApproachDel: 35.4 XXXXXX XXXXXX XXXXXX ApproachLOS: E ************************************* Note: Queue reported is the number of cars per lane. ***********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #9 Jasper / Rogge ***************************** 0.7 Average Delay (sec/veh): Worst Case Level Of Service: C[19.3] ************************* Street Name: Jasper Rogge Approach: North Bound South Bound Last Bound

Movement: L - T - R L - T - R L - T - R -----| Stop Sign Uncontrolled Uncontrolled Include Include Include Stop Sign Control: Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 -----| Volume Module: Base Vol: 16 0 8 9 350 0 0 0 0 351 21 Initial Bse: 16 0 8 0 0 0 351 21 9 350 PHF Volume: 24 0 Reduct Vol: 0 0 0 0 0 532 12 0 32 14 530 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 24 0 12 0 0 532 32 0 0 14 530 0 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.2 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.3 xxxx xxxxx Capacity Module: Cnflict Vol: 1105 1105 548 xxxx xxxx xxxxx xxxx xxxx 564 xxxx xxxxx Potent Cap.: 235 212 540 xxxx xxxx xxxxx xxxx xxxx xxxx 983 xxxx xxxxx 233 209 540 xxxx xxxx xxxxx xxxx xxxx Move Cap.: 983 xxxx xxxxx _____ Level Of Service Module: 0.0 xxxx xxxx 8.7 XXXX XXXXX A * * LOS by Move: * * * * * * * * * Movement: LT - LTR - RT 0.0 xxxx xxxxx 8.7 xxxx xxxxx A * * 19.3 ApproachDel: XXXXXX XXXXXX XXXXXX C ApproachLOS: ********************************* Note: Queue reported is the number of cars per lane. ****************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************** Intersection #9 Jasper / Rogge ****************** Average Delay (sec/veh): 0.4 Worst Case Level Of Service: B[11.2] ************************** Jasper Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Rights: Include Includ Volume Module: Base Vol: 4 0 6 0 0 0 200 6 7 242 Initial Bse: 4 0 6 0 0 0 200 6 7 242 0 PHF Volume: 6 0 8 0 0
Reduct Vol: 0 0 0 0
FinalVolume: 6 0 8 0 0 0 0 278 8 10 336 0 0 0 0 0 0 0 0 0 0 278 8 10 336 0 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx _____| Capacity Module: Cnflict Vol: 638 638 282 xxxx xxxx xxxxx xxxx xxxx xxxxx 286 xxxx xxxxx Level Of Service Module: 0.0 xxxx xxxxx LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT 0.0 xxxx xxxxx 7.8 xxxx xxxxx A * * Shared LOS: * B * * * * * * * ApproachDel: 11.2
ApproachLOS: B XXXXXX ************************* Note: Queue reported is the number of cars per lane. *****************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********************* Intersection #9 Jasper / Rogge ************************* Average Delay (sec/veh): 0.6 Worst Case Level Of Service: C[20.6] ************************ Street Name: Jasper Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 -----| Volume Module: Base Vol: 14 0 7 0 0 424 0 0 1 4 353 Initial Bse: 14 0 7 0 0 0 0 424 1 4 353 Critical Gap Module: FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Potent Cap.: 214 194 483 xxxx xxxx xxxxx xxxx xxxx xxxx 939 xxxx xxxxx Move Cap.: 213 193 483 xxxx xxxx xxxxx xxxx xxxx xxxxx 939 xxxx xxxxx Level Of Service Module: 0.0 xxxx xxxx 8.9 xxxx xxxxx LOS by Move: * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT SharedQueue:xxxxx 0.4 xxxxx xxxxx xxxxx xxxxx xxxxx xxxxx 0.0 xxxx xxxxx Shared LOS: * C * * * * * * * A * 20.6 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: С Note: Queue reported is the number of cars per lane. *******************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********************************* Intersection #9 Jasper / Rogge ************************* Average Delay (sec/veh): 0.6 Worst Case Level Of Service: C[16.8] *********************************** Street Name: Jasper Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RStop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 Lanes: Volume Module: Base Vol: 0 0 356 16 8 0 0 0 21 9 335 0 356 9 335 Initial Bse: 16 0 8 0 0 21 0 PHF Volume: 22 0 11 0 0 0 0 481 28 12 453 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 22 11 0 0 481 28 12 453 Critical Gap Module: Critical Gp: 6.4 6.5 FollowUpTim: 3.5 4.0 _____ Capacity Module: Cnflict Vol: 972 972 495 xxxx xxxx xxxxx xxxx xxxx xxxx 509 xxxx xxxxx Level Of Service Module: 0.0 xxxx xxxxx 8.5 xxxx xxxxx * * * * * * * * * LOS by Move: A * * Movement: LT - LTR - RT 0.0 xxxx xxxx 8.5 xxxx xxxxx Shared LOS: * C * * * * * * * A * ApproachDel: 16.8 XXXXXX XXXXXX XXXXXX С ********************************* Note: Queue reported is the number of cars per lane. ********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************** Intersection #9 Jasper / Rogge ****************** Average Delay (sec/veh): 0.3 Worst Case Level Of Service: B[11.4] ********************************** Street Name: Jasper Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 Lanes: Volume Module: Base Vol: 4 0 6 0 0 0 0 240 7 330 1.00 1.00 1.00 Initial Bse: 4 0 6 0 240 0 0 0 6 7 330 PHF Adj: 7 PHF Volume: 5 0 7 0 0 0 0 279 8 384 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 5 0 0 FinalVolume: 7 0 0 0 279 7 8 384 ------||-----||-------||------| Critical Gap Module: FollowUpTim: 3.5 4.0 Capacity Module: Cnflict Vol: 683 683 283 xxxx xxxx xxxxx xxxx xxxx xxxxx 286 xxxx xxxxx Potent Cap.: 418 374 761 xxxx xxxx xxxxx xxxx xxxx xxxxx 1248 xxxx xxxxx Move Cap.: 416 372 761 xxxx xxxx xxxxx xxxx xxxx xxxxx 1248 xxxx xxxxx -----|----|-----| Level Of Service Module: 0.0 xxxx xxxx 7.9 xxxx xxxxx LOS by Move: * * * * * * A * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT 0.0 xxxx xxxxx Shrd ConDel:xxxxx 11.4 xxxxx xxxxx xxxx xxxxx xxxxx xxxxx 7.9 xxxx xxxxx Shared LOS: * B * * * * * * * A * ApproachDel: 11.4 XXXXXX XXXXXX XXXXXX ApproachLOS: ****************************** Note: Queue reported is the number of cars per lane. ************************************

APPENDIX L INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

Jade Drive/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************ Intersection #10 Jade / Rogge ******************************* 0.8 Average Delay (sec/veh): Worst Case Level Of Service: B[11.8] *********************************** Jade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R-----|-----|------| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 -----|----|-----|-----| Volume Module: 7 0 28 Base Vol: 0 0 0 0 291 9 260 Initial Bse: 7 0 28 0 0 0 0 291 3 9 260 0 PHF Adj: PHF Volume: 9 0 0 383 4 12 342 0 37 0 0 0 Reduct Vol: 0 0 FinalVolume: 9 0 0 = 0 0 0 0 0 0 0 0 0 0 2 0 37 0 0 383 4 12 342 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 751 751 385 xxxx xxxx xxxxx xxxx xxxx 387 xxxx xxxxx Potent Cap.: 382 342 667 XXXX XXXX XXXXX XXXX XXXXX 1161 XXXX XXXXX Move Cap.: 379 339 667 xxxx xxxx xxxxx xxxx xxxx xxxxx 1161 xxxx xxxxx -----|----|-----|------| Level Of Service Module: 8.1 xxxx xxxxx LOS by Move: * * * * * * * * A * * Movement: LT - LTR - RT Shrd ConDel:xxxxx 11.8 xxxxx xxxxx xxxx xxxxx xxxxx xxxxx 8.1 xxxx xxxxx Shared LOS: * B * * * * * * * A * 11.8 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В *********************************** Note: Queue reported is the number of cars per lane. *********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************** Intersection #10 Jade / Rogge ************************** 0.5 Average Delay (sec/veh): Worst Case Level Of Service: A[9.5] *********************** Jade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RControl: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Include Volume Module: Base Vol: 3 0 16 0 0 0 266 9 11 229 Initial Bse: 3 0 16 0 0 0 0 266 9 11 229 6 PHF Volume: 2 0 11 0 0 0 189 6
Reduct Vol: 0 0 0 0 0 0 0
FinalVolume: 2 0 11 0 0 0 189 6 0 0 189 6 8 163 0 0 Ω 8 163 -----| Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx 4.2 xxxx xxxxx ______|__|___|___| Capacity Module: Cnflict Vol: 372 375 Potent Cap.: 632 559 855 xxxx xxxx xxxxx xxxx xxxx xxxxx 1348 xxxx xxxxx Move Cap.: 630 556 855 xxxx xxxx xxxxx xxxx xxxx xxxxx 1348 xxxx xxxxx Level Of Service Module: Movement: LT - LTR - RT Shared LOS: * A * * * * * * * * * * ApproachDel: 9.5
ApproachLOS: A XXXXXX XXXXXX XXXXXX ***************************** Note: Queue reported is the number of cars per lane. **************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) **************** Intersection #10 Jade / Rogge ************************* Average Delay (sec/veh): 0.8 Worst Case Level Of Service: A[9.8] ********************************** Jade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 _____| Volume Module: 6 0 13 0 0 0 Base Vol: 0 149 7 14 180 0 Initial Bse: 6 0 13 0 0 0 149 7 14 180 0 0 0 160 8 15 194 0 0 0 0 0 0 0 0 0 0 160 8 15 194 0 PHF Volume: 6 0 14 0 0 Reduct Vol: 0 0 FinalVolume: 6 0 0 0 0 14 Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx _____ Capacity Module: Cnflict Vol: 388 388 _____| | | Level Of Service Module: 0.0 xxxx xxxxx 7.6 xxxx xxxxx LOS by Move: * * * * * * * * * A * * Movement: LT - LTR - RT 0.0 xxxx xxxxx 7.6 xxxx xxxxx A * * ApproachLOS: 9.8
ApproachLOS: A XXXXXX XXXXXX *********************************** Note: Queue reported is the number of cars per lane. *****************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #10 Jade / Rogge ************************ 2.1 Average Delay (sec/veh): Worst Case Level Of Service: D[26.9] Jade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 0 -----|----|-----|------| Volume Module: 7 0 60 0 0 0 496 3 Base Vol: Initial Bse: 7 0 60 0 0 0 0 496 3 30 397 0 0 870 5 53 696 0 0 0 0 0 0 Reduct Vol: Reduct Vol: 0 0 0 0 0 0 FinalVolume: 12 0 105 0 0 0 0 870 5 53 696 -----|----|-----|-----| Critical Gap Module: Critical Gp: 6.4 6.5 6.2 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx -----| Capacity Module: Cnflict Vol: 1675 1675 873 xxxx xxxx xxxxx xxxx xxxx xxxxx 875 xxxx xxxxx Potent Cap.: 106 96 353 xxxx xxxx xxxxx xxxx xxxx xxxxx 763 xxxx xxxxx Move Cap.: 100 90 353 xxxx xxxx xxxxx xxxx xxxx xxxxx 763 xxxx xxxxx -----|----|-----|------| Level Of Service Module: 0.2 xxxx xxxxx LOS by Move: * * * * * * * * * B * * Movement: LT - LTR - RT Shared LOS: * D * * * * * * B * ApproachLOS: XXXXXX XXXXXX XXXXXX Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #10 Jade / Rogge ************************** 0.9 Average Delay (sec/veh): Worst Case Level Of Service: B[13.8] *********************** Jade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RControl: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 0 0 0 1! 0 0 -----| Volume Module: Base Vol: 3 0 30 0 0 0 350 9 32 356 Initial Bse: 3 0 30 0 0 0 0 350 9 32 356 6 PHF Volume: 5 0 47 0 0 0 0 547 14 50 556 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 5 0 47 0 0 0 0 547 14 50 556 0 -----| Critical Gap Module: Capacity Module: Cnflict Vol: 1215 1220 554 xxxx xxxx xxxxx xxxx xxxx xxxx 561 xxxx xxxxx Potent Cap: 202 182 536 xxxx xxxx xxxxx xxxx xxxx xxxxx 986 xxxx xxxxx Move Cap.: _____| Level Of Service Module: . LOS by Move: * * * * * * * * A * * LT - LTR - RT Movement: Shared LOS: * B * * * * * * * * * * * ApproachLOS: XXXXXX XXXXXX XXXXXX *********************************** Note: Queue reported is the number of cars per lane. ****************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #10 Jade / Rogge ************************** 1.0 Average Delay (sec/veh): Worst Case Level Of Service: B[11.1] *********************** Jade Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RControl: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Volume Module: 6 0 20 0 0 0 Base Vol: 0 199 7 24 243 Initial Bse: 6 0 20 0 0 0 199 7 24 243 0 34 347 PHF Volume: 9 0 29 0 284 10 0 0 0 Reduct Vol: 0 0 FinalVolume: 9 0 0 0 0 0 0 0 0 284 10 0 0 0 0 0 0 29 0 0 34 347 0 Critical Gap Module: FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxx xxxx xxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 705 705 289 xxxx xxxx xxxxx xxxx xxxx 294 xxxx xxxxx 755 xxxx xxxx xxxxx xxxx xxxx 1273 xxxx xxxxx Potent Cap.: 406 363 Move Cap.: Level Of Service Module: A * * LOS by Move: * * * * * * * * Movement: LT - LTR - RT Shrd ConDel:xxxxx 11.1 xxxxx xxxxx xxxx xxxxx xxxxx xxxxx 7.9 xxxx xxxxx A * * Shared LOS: * B * * * * * * * ApproachDel: 11.1 XXXXXX XXXXXX XXXXXX ApproachLOS: В ********************************* Note: Queue reported is the number of cars per lane. ***************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************** Intersection #10 Jade / Rogge ******************************* Average Delay (sec/veh): 1.6 Worst Case Level Of Service: C[16.6] ******************************** Street Name: Jade Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RL - T - R Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Rights: Lanes: -----||-----||-----| Volume Module: 7 0 Base Vol: 61 0 0 0 0 428 3 31 350 Initial Bse: 7 0 61 0 0 0 0 428 3 31 350 4 46 522 PHF Volume: 10 0 91 0 0 0 0 639 0 = Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 639 4 0 FinalVolume: 10 0 91 0 0 46 522 Critical Gap Module: FollowUpTim: 3.5 4.0 3.3 xxxxx xxxx xxxxx xxxxx xxxxx 2.2 xxxx xxxxx Capacity Module: Cnflict Vol: 1256 1256 641 xxxx xxxx xxxxx xxxx xxxx xxxx 643 xxxx xxxxx Potent Cap.: 191 173 478 xxxx xxxx xxxxx xxxx xxxx xxxx 932 xxxx xxxxx Move Cap.: Level Of Service Module: 0.2 xxxx xxxxx 9.1 xxxx xxxxx LOS by Move: * * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT 9.1 xxxx xxxxx Shared LOS: * C * * * * * * * A * 16.6 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: С ****************************** Note: Queue reported is the number of cars per lane. ********************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********************** Intersection #10 Jade / Rogge ************************** Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[13.0] ********************************* Street Name: Jade Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RMovement: -----| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: Lanes: 0 0 1! 0 0 0 0 0 0 0 0 0 1 0 0 0 1! 0 0 Volume Module: Base Vol: 3 0 30 0 ... 0 0 0 355 32 341 Initial Bse: 3 0 30 0 355 0 0 0 9 32 341 PHF Adj: $0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70$ PHF Volume: 4 0 43 0 0 0 0 507 13 46 487 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 4 0 43 0 0 13 FinalVolume: 0 0 507 46 487 Critical Gap Module: Capacity Module: Cnflict Vol: 1096 1101 514 xxxx xxxx xxxxx xxxx xxxx xxxx 520 xxxx xxxxx Level Of Service Module: 8.7 xxxx xxxxx LOS by Move: * * * * * * * * A * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT Shared LOS: * B * * * * * * * * * 13.0 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ******************** Note: Oueue reported is the number of cars per lane. ***********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #10 Jade / Rogge **************** Average Delay (sec/veh): 0.8 Worst Case Level Of Service: B[11.1] ***************************** Street Name: Jade Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Movement: -----|----|-----|------| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: Lanes: Volume Module: Base Vol: 6 0 20 . 0 0 0 0 236 24 333 Initial Bse: 6 0 20 0 236 0 0 0 7 24 333 PHF Volume: 7 0
Reduct Vol: 0 0 24 0 0 0 0 281 8 29 396 0 0 0 0 0 0 0 0 0 0 8 0 281 7 0 24 0 0 29 396 FinalVolume: Critical Gap Module: Critical Gp: 6.4 6.5 FollowUpTim: 3.5 4.0 Capacity Module: Cnflict Vol: 739 739 285 xxxx xxxx xxxxx xxxx xxxx xxxx 289 xxxx xxxxx Level Of Service Module: 8.0 xxxx xxxx LOS by Move: * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT 0.1 xxxx xxxxx Shrd ConDel:xxxxx 11.1 xxxxx xxxxx xxxx xxxxx xxxxx xxxx xxxx 8.0 xxxx xxxx Shared LOS: * B * * * * * * * A * 11.1 ApproachDel: xxxxxx . 11.1 B XXXXXX XXXXXX ApproachLOS: ***************************** Note: Queue reported is the number of cars per lane. *****************

APPENDIX M INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

High School Driveway #1/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #11 Driveway 1 / Rogge ******************* Average Delay (sec/veh): 18.5 Worst Case Level Of Service: F[99.8] ******************** Street Name: Driveway 1 Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Lanes: 1 0 0 0 1 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Volume Module: 158 0 34 0 386 170 43 269 Base Vol: 0 0 0 Initial Bse: 158 0 34 0 0 0 386 170 43 269 0 PHF Volume: 282 0 61 0 0 0 689 304 77 480 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 282 0 61 0 0 0 0 689 304 77 480 0 Critical Gap Module: _____| Capacity Module: Level Of Service Module: Control Del:117.7 xxxx 16.7 xxxxx xxxx xxxxx xxxxx xxxx xxxxx 10.9 xxxx xxxxx LOS by Move: F * C * * * * * * * B * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT ApproachLOS: 99.8
ApproachLOS: F Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************************** Intersection #11 Driveway 1 / Rogge ******************* Average Delay (sec/veh): 3.4 Worst Case Level Of Service: C[20.3] ****************** Street Name: Driveway 1 East Bound West Bound
L - T - R L - T - R Approach: North Bound South Bound East Bound Movement: L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Lanes: 1 0 0 0 1 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Volume Module: Base Vol: 105 0 20 0 0 0 0 282 98 24 289 Initial Bse: 105 0 20 0 0 0 282 98 24 289 0 PHF Volume: 167 0 32 0 0 0 0 448 156 38 459 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 167 0 32 0 0 0 0 448 156 38 459 _____| Critical Gap Module: Capacity Module: Level Of Service Module: LOS by Move: C * B * * * * * * A * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT ApproachLOS: 20.3
ApproachLOS: C Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #11 Driveway 1 / Rogge ******************** Average Delay (sec/veh): 1.5 Worst Case Level Of Service: B[12.2] ************************* Street Name: Driveway 1 Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Volume Module: Base Vol: 46 0 11 0 0 0 0 192 27 7 221 PHF Volume: 68 0 16 0 0 0 0 282 40 10 325 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 68 0 16 0 0 0 0 282 40 10 325 0 -----||-----||-----| Critical Gap Module: Capacity Module: Level Of Service Module: LOS by Move: B * A * * * * * * * A * * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT *************************** Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ****************************** Intersection #11 Driveway 1 / Rogge ******************* Average Delay (sec/veh): 200.4 Worst Case Level Of Service: F[850.2] ***** Street Name: Driveway 1 Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Lanes: 1 0 0 0 1 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Volume Module: Base Vol: 71 0 253 0 0 0 0 412 77 278 310 Initial Bse: 71 0 253 0 0 0 412 77 278 310 0 PHF Adj: PHF Volume: 129 0 460 0 0 0 749 140 505 564 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 129 0 460 0 0 0 0 749 140 505 564 Λ Critical Gap Module: ______| Capacity Module: -----| Level Of Service Module: Control Del: 3346 xxxx 149.9 xxxxx xxxx xxxxx xxxxx xxxxx 18.9 xxxx xxxxx LOS by Move: F * F * * * * * * * C * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT ApproachDel: 850.2 ApproachLOS: F Note: Queue reported is the number of cars per lane. ***********

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************* Intersection #11 Driveway 1 / Rogge ************************ Average Delay (sec/veh): 9.4 Worst Case Level Of Service: E[40.7] ************************* Street Name: Driveway 1 Approach: Driveway 1 Rogge
Approach: North Bound South Bound East Bound West Bound
Movement: L - T - R L - T - R L - T - R Volume Module: Base Vol: 48 0 165 0 0 0 0 341 44 160 331 Initial Bse: 48 0 165 0 0 0 0 341 44 160 331 0 PHF Volume: 75 0 258 0 0 0 0 533 69 250 517 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 75 0 258 0 0 0 0 533 69 250 517 Λ Critical Gap Module: -----| |-----| |-----| |------| |-----| |-----| Capacity Module: Volume/Cap: 0.78 xxxx 0.49 xxxx xxxx xxxx xxxx xxxx xxxx 0.26 xxxx xxxx Level Of Service Module: LOS by Move: F * C * * * * * * B * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT Shared LOS: * * * * * * * * * * * xxxxxx ApproachDel: 40.7 XXXXXX XXXXXX E ApproachLOS: Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #11 Driveway 1 / Rogge *************************** Average Delay (sec/veh): 2.2 Worst Case Level Of Service: B[12.9] ******************** Street Name: Driveway 1 Street Name: Driveway 1 Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Volume Module: Base Vol: 21 0 74 0 0 0 0 244 12 44 336 Initial Bse: 21 0 74 0 0 0 0 244 12 44 336 0 PHF Volume: 30 0 104 0 0 0 0 344 17 62 473 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 30 0 104 0 0 0 0 344 17 62 473 0 Critical Gap Module: Capacity Module: Level Of Service Module: LOS by Move: C * B * * * * * * A * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT xxxxxx ApproachDel: 12.9
ApproachLOS: B XXXXXX xxxxxx Note: Queue reported is the number of cars per lane. *****************************

APPENDIX N INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

High School Driveway #2/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************ Intersection #12 Driveway 2 / Rogge *********************** Average Delay (sec/veh): 0.3 Worst Case Level Of Service: A[9.0] ************************ Driveway 2 Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----|----|-----|-----| -----|----|-----|-----| Volume Module: 0 0 0 0 0 0 Base Vol: 0 353 67 23 306 0 0 0 0 353 67 Initial Bse: 0 0 0 23 306 0 0 0 PHF Volume: 0 0 0 0 0 543 103 35 471 Reduct Vol: 0 0 FinalVolume: 0 0 0 0 0 0 0 0 0 0 0 0 0 543 103 0 0 0 35 471 Critical Gap Module: 4.1 xxxx xxxxx 2.2 xxxx xxxxx -----| Capacity Module: 646 xxxx xxxxx 930 xxxx xxxxx 930 xxxx xxxxx -----| Level Of Service Module: LOS by Move: * * * * * * * * * A * * Movement: LT - LTR - RT Shared LOS: * * * * * * * * * * * ApproachDel: xxxxxx XXXXXX XXXXXX XXXXXX ApproachLOS: Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************* Intersection #12 Driveway 2 / Rogge ***************************** Average Delay (sec/veh): Worst Case Level Of Service: A[8.3] ******************** Street Name: Driveway 2 Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R _____| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 0 0 0 0 0 0 0 0 0 0 0 1 0 0 1 0 0 Volume Module: Base Vol: 0 0 0 0 0 0 302 0 6 307 Initial Bse: 0 0 0 0 0 0 0 302 0 6 307 0 9 451 PHF Volume: 0 0 Reduct Vol: 0 0 0 0 444 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 444 0 9 451 0 0 0 0 FinalVolume: 0 0 0 0 0 0 Critical Gap Module: Capacity Module: -----| Level Of Service Module: LOS by Move: * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT ApproachDel: XXXXXX XXXXXX XXXXXX XXXXXX * ApproachLOS: ********************************** Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #12 Driveway 2 / Rogge ************************ Average Delay (sec/veh): 0.1 Worst Case Level Of Service: A[7.8] ************************* Driveway 2 Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Control: Include Include Rights: Lanes: 0 0 0 0 0 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Volume Module: 0 0 0 Base Vol: 0 0 0 0 173 30 7 228 Initial Bse: 0 0 0 0 0 0 173 30 7 228 PHF Volume: 0 0 0 0 0 0 0 231 40 9 304 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 9 304 FinalVolume: 0 0 0 0 0 231 40 -----|----|----|-----| Critical Gap Module: 4.1 xxxx xxxxx _____| Capacity Module: _____| Level Of Service Module: LOS by Move: * * * * * * * * A * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT ApproachDel: xxxxxx XXXXXX XXXXXX XXXXXX ApproachLOS: ************************** Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #12 Driveway 2 / Rogge *********************************** Average Delay (sec/veh): 1.3 Worst Case Level Of Service: B[15.0] ************************************ Street Name: Driveway 2 Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R L - T - R Uncontrolled Uncontrolled Stop Sign Stop Sign Include Include Include Rights: Include Include 0 0 0 0 0 0 0 0 0 0 0 0 1 0 1 0 1 0 0 Lanes: ______| | ____| | _____| | _____| | _____| | _____| | ____| | _____| | _____| | _____| Volume Module: 0 Base Vol: 0 0 0 0 0 0 635 30 116 582 1.00 1.00 1.00 Initial Bse: 0 0 0 0 0 0 0 635 30 116 582 PHF Adj: 0 0 1155 55 211 1058 PHF Volume: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: FinalVolume: 0 0 0 0 0 0 0 1155 55 211 1058 _____| Critical Gap Module: Capacity Module: Level Of Service Module: 1.7 xxxx xxxxx LOS by Move: * * * * * * * * B * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT * * * * * * * * * * * Shared LOS: XXXXXX ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: ******************************** Note: Queue reported is the number of cars per lane. **********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) *************************** Intersection #12 Driveway 2 / Rogge **************************** Average Delay (sec/veh): 0.1 Worst Case Level Of Service: A[9.4] **************************** Street Name: Driveway 2 Rogge

North Bound South Bound East Bound West Bound
L - T - R L - T - R L - T - R Driveway 2 Approach: L - T - R Movement: Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Rights: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Lanes: 1 0 1 0 0 Volume Module: 0 0 506 6 485 Base Vol: 0 0 0 0 0 0 Initial Bse: 0 0 0 0 0 0 0 506 0 6 485 PHF Adj: 0 0 778 0 9 746 PHF Volume: 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 Ω 0 FinalVolume: 0 0 0 9 746 0 778 Critical Gap Module: Capacity Module: Level Of Service Module: 0.0 xxxx xxxxx 9.4 xxxx xxxxx LOS by Move: * * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT * * * * * * * * * * * Shared LOS: ApproachDel: XXXXXX XXXXXX XXXXXX XXXXXX ApproachLOS: *******************************

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) *********************** Intersection #12 Driveway 2 / Rogge ************************* Average Delay (sec/veh): 0.6 Worst Case Level Of Service: A[8.6] *********************** Street Name: Driveway 2 Rogge Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RStop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: Lanes: Volume Module: Base Vol: 0 0 0 0 0 0 318 14 49 380 0 Initial Bse: 0 0 0 0 0 0 0 318 14 49 380 PHF Adj: $0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70\ 0.70$ PHF Volume: 0 0 20 70 543 0 0 ... 0 0 0 454 0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 20 FinalVolume: 0 0 0 454 70 543 Critical Gap Module: Capacity Module: 474 xxxx xxxxx Level Of Service Module: 8.6 xxxx xxxxx A * * LOS by Move: * * * * * * * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT Shared LOS: * * * * * * * * * * ApproachDel: XXXXXX XXXXXX XXXXXX XXXXXX ApproachLOS: Note: Queue reported is the number of cars per lane.

APPENDIX O INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

High School Driveway #3/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #13 Driveway 3 / Rogge ********************** Average Delay (sec/veh): 0.0 Worst Case Level Of Service: A[0.0] ********************** Street Name: Driveway 3 Rogge Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R L - T - RMovement: -----| Volume Module: Base Vol: 0 0 0 0 0 0 0 353 0 329 Initial Bse: 0 0 0 0 0 0 353 0 0 329 0 0 0 0 519 0 0 484 0 PHF Volume: 0 0 0 0 0 0 Reduct Vol: 0 0 0 0 0 0 0 0 FinalVolume: 0 0 0 0 0 519 0 0 484 Critical Gap Module: -----|----|-----| Capacity Module: Cnflict Vol: 1003 xxxx Potent Cap.: 271 xxxx Level Of Service Module: LOS by Move: * * * * * * * * * * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT ApproachDel: xxxxxx ApproachLOS: * XXXXXX XXXXXX XXXXXX ************************ Note: Queue reported is the number of cars per lane. ************************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ***************************** Intersection #13 Driveway 3 / Rogge ****************************** Average Delay (sec/veh): 1.4 Worst Case Level Of Service: C[17.0] ****************************** Street Name: Driveway 3 Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R -----|----|-----|------| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Volume Module: Base Vol: 43 0 10 0 0 0 302 0 0 270 Initial Bse: 43 0 10 0 0 0 0 302 0 0 270 0 0 0 0 403 PHF Volume: 64 0 15 0 0 451 0 0 ő 0 Reduct Vol: 0 0 0 0 0 0 0 0 FinalVolume: 64 0 15 0 0 451 0 0 0 0 403 Critical Gap Module: -----| Capacity Module: Cnflict Vol: 854 xxxx Level Of Service Module: LOS by Move: C * B * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT 17.0 ApproachDel: XXXXXX XXXXXX XXXXXX С ApproachLOS: Note: Queue reported is the number of cars per lane. ***********************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) *********************************** Intersection #13 Driveway 3 / Rogge ******************* Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[11.5] ******************* Driveway 3 Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - RStop Sign Uncontrolled Uncontrolled Include Include Stop Sign Include Rights: Lanes: 1 0 0 0 1 0 0 0 0 0 0 1 0 0 0 0 1 0 0 Volume Module: 27 0 7 0 173 0 Base Vol: 0 0 0 0 208 Initial Bse: 27 0 7 0 0 0 173 0 0 208 9 PHF Volume: 34 0
Reduct Vol: 0 0 0 219 0 0 0 0 0 263 0 0 0 0 0 0 0 0 0 0 FinalVolume: 34 0 9 0 0 0 0 219 0 0 263 0 Critical Gap Module: Capacity Module: Move Cap.: _____| Level Of Service Module: LOS by Move: B * A * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shared LOS: * * * * * * * * * * * * 11.5 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ************************* Note: Queue reported is the number of cars per lane. *************************

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) **************************** Intersection #13 Driveway 3 / Rogge ************************** Average Delay (sec/veh): 0.0 Worst Case Level Of Service: A[0.0] ************************* Driveway 3 Street Name: Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R Rights: Lanes: _____| Volume Module: 0 0 Base Vol: 0 0 0 0 0 635 0 0 698 Initial Bse: 0 0 0 0 0 0 0 635 0 0 698 PHF Volume: 0 0 0 0 0 0 0 1155 0 0 1269 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 ő 0 0 0 0 0 0 0 0 0 1155 0 0 1269 0 0 0 0 0 FinalVolume: 0 0 0 0 Critical Gap Module: _____| Capacity Module: _____| Level Of Service Module: LOS by Move: * * * * * * * * * * * LT - LTR - RT Movement: * * * * * * * * * * Shared LOS: XXXXXX ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: **************************** Note: Queue reported is the number of cars per lane. ********************

_____ Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #13 Driveway 3 / Rogge ******************** Average Delay (sec/veh): 1.8 Worst Case Level Of Service: C[22.5] ****************************** Street Name: Driveway 3 North Bound South Bound East Bound West Bound
L - T - R L - T - R L - T - F Approach: L - T - R Movement: Rights: Lanes: -----| |-----| |-----| |------| Volume Module: 19 0 Base Vol: 68 0 0 0 0 506 0 -0 472 Initial Bse: 19 0 68 0 0 0 0 506 0 0 472 PHF Volume: 30 0 106 0 0 0 791 Reduct Vol: 0 0 0 0 0 0 0 0 0 738 0 0 0 0 0 0 0 791 0 0 0 FinalVolume: 30 0 106 0 0 0 738 Critical Gap Module: Capacity Module: Cnflict Vol: 1528 xxxx -----||-----||-----||------| Level Of Service Module: LOS by Move: E * C * * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT Shared LOS: * * * * * * * * * * * 22.5 ApproachDel: XXXXXX XXXXXX XXXXXX C ApproachLOS: ********************************

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) **************** Intersection #13 Driveway 3 / Rogge ********************* Average Delay (sec/veh): 1.0 Worst Case Level Of Service: B[13.7] ***************** Street Name: Driveway 3 Rogge South Bound East Bound West Bound L - T - R L - T - R Approach: North Bound South Bound L - T - R L - T - R Movement: _____| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: Include Include Include 0 0 0 0 0 0 0 1 0 0 1 0 0 0 1 Lanes: 0 0 1 0 0 Volume Module: Base Vol: 12 0 44 0 0 318 0 0 - 0 0 417 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 12 0 44 0 0 0 0 318 . 0 0 417 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.68 0.68 0.68 0.68 0.68 0.68 0.68 0.68 0.68 0.68 0.68 PHF Adj: PHF Volume: 18 0 65 0 0 0 0 468 0 0 613 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 0 FinalVolume: 18 65 0 . 0 0 0 613 0 468 -----||-----||-----||-----||------|| Critical Gap Module: Critical Gp: 6.4 xxxx FollowUpTim: 3.5 xxxx _____ Capacity Module: Cnflict Vol: 1081 xxxx Potent Cap.: 243 xxxx 243 xxxx Move Cap.: Level Of Service Module: C * B * * * * * * LOS by Move: LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT Shared LOS: * * * * * * *** ***. * * 13.7 ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: В ******************************* Note: Queue reported is the number of cars per lane. ***********************************

APPENDIX P INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

High School Driveway #4/Rogge Road

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************************ Intersection #14 Bus Exit / Rogge ********************** Average Delay (sec/veh): 0.2 Worst Case Level Of Service: B[11.6] ************************************** Bus Exit Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R -----| Volume Module: Base Vol: 0 0 12 0 0 0 353 0 0 329 Initial Bse: 0 0 12 0 0 0 0 353 0 0 329 0 PHF Volume: 0 0 18
Reduct Vol: 0 0 0
FinalVolume: 0 0 18 0 0 18 0 0 0 0 0 0 0 0 0 18 0 0 0 0 519 0 0 484 0 0 0 0 0 0 0 0 0 0 519 0 Critical Gap Module: _____| Capacity Module: Level Of Service Module: LOS by Move: * * B * * * * * * * * * Movement: LT - LTR - RT ApproachLOS: XXXXXX XXXXXX XXXXXX Note: Queue reported is the number of cars per lane. *******************************

Movement:

ApproachDel:

ApproachLOS: B

Exist + Proj MD Fri May 27, 2011 15:49:28 Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ******************** Intersection #14 Bus Exit / Rogge ************************** Average Delay (sec/veh): 0.2 Worst Case Level Of Service: B[11.1] ***** Bus Exit Street Name: Rogge Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R_____| _____ Volume Module: Base Vol: 0 0 12 0 0 0 0 312 0 0 270 Initial Bse: 0 0 12 0 0 0 0 312 0 0 270 0 0 397 0 0 0 397 PHF Volume: 0 0 18 0 0 0 0 459 0 0 0 0 0 0 0 0 0 18 0 0 Reduct Vol: 0 0 0 0 0 0 Ö FinalVolume: 0 459 0 Critical Gap Module: _____| Capacity Module: Level Of Service Module:

Note: Queue reported is the number of cars per lane. *************************

11.1

LOS by Move: * * B * * * * * * * * *

XXXXXX

LT - LTR - RT LT - LTR - RT LT - LTR - RT

XXXXXX

XXXXXX

```
Level Of Service Computation Report
   2000 HCM Unsignalized Method (Base Volume Alternative)
*********************
Intersection #14 Bus Exit / Rogge
*******************************
Average Delay (sec/veh): 0.0
            Worst Case Level Of Service: A[ 0.0]
*************************
       Bus Exit
Street Name:
                   Rogge
Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-R
_____|
Control: Stop Sign
Rights: Include
_____|
Volume Module:
Base Vol: 0 0 0 0 0
               0 180 0
                     0 208
Initial Bse: 0 0 0 0 0 0 180 0 0 208
PHF Volume: 0 0 0 0
             0 0 214 0 0 248
                    0 0
                   0
        0
           0
Reduct Vol: 0 0
          0
              0
               0 0
                         0
              0
        0 0 0
                   0
FinalVolume:
    0 0
               0 214
                     0 248
Critical Gap Module:
Capacity Module:
Level Of Service Module:
LOS by Move: * * * * * * * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT
                    LT - LTR - RT
ApproachDel:
    XXXXXX
          XXXXXX
               XXXXXX
                     XXXXXX
ApproachLOS:
    *
******************************
Note: Queue reported is the number of cars per lane.
***********************
```

Tue Jun 21, 2011 15:14:49 GPBO AM Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************ Intersection #14 Bus Exit / Rogge ***************** Average Delay (sec/veh): 0.1 Worst Case Level Of Service: C[20.6] ************************* Bus Exit North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: Movement: Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Rights: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 0 1 0 0 Lanes: _____| | | Volume Module: 0 0 6 0 Base Vol: - 0 0 0 635 0 0 698 1.00 1.00 1.00 Initial Bse: 0 0 6 0 0 0 0 635 0 0 698 PHF Adj: PHF Volume: 0 0 11 0 0 0 0 1155 0 0 1269 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: FinalVolume: 0 0 11 0 0 0 0 1155 0 0 1269 -----||-----||-----||------| Critical Gap Module: ______| Capacity Module: _____| | | Level Of Service Module: LOS by Move: * * C * * * * * * * * * LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT LT - LTR - RT

* * * *

XXXXXX

Shared LOS: * *

20.6

C

ApproachDel:

ApproachLOS:

* *

XXXXXX

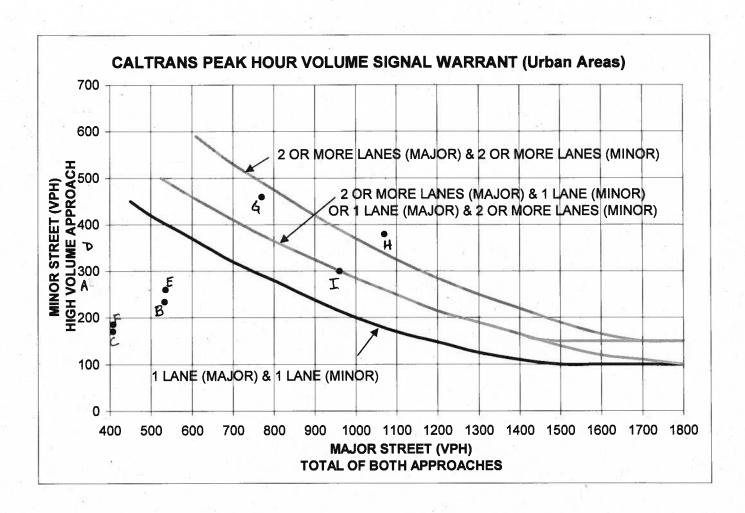
XXXXXX

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********************** Intersection #14 Bus Exit / Rogge ************************* Average Delay (sec/veh): 0.2 Worst Case Level Of Service: C[16.2] ************************* Bus Exit North Bound South Bound East Bound West Bound Approach: $L - T - R \quad L - T - R \quad L - T - R$ L - T - R Movement: Rights: Lanes: _____| Volume Module: 0 0 12 0 0 574 0 Base Vol: 0 0 - 0 0 472 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 0 0 12 0 0 0 574 0 0 472 0 0 0 0 897 0 0 0 0 0 PHF Volume: 0 0 19 0 0 738 0 0, 0 0 0 0 0 Reduct Vol: FinalVolume: 0 0 19 0 0 . 0 0 897 0 0 738 _____| | -----| Critical Gap Module: _____| Capacity Module: Level Of Service Module: LOS by Move: * * C * * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT * * * * * * * * Shared LOS: ApproachDel: 16.2 XXXXXX XXXXXX XXXXXX C ApproachLOS: ********************* Note: Queue reported is the number of cars per lane. ************************

_____ Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************************* Intersection #14 Bus Exit / Rogge ***************************** Average Delay (sec/veh): 0.0 Worst Case Level Of Service: A[0.0] ****************************** Street Name: Bus Exit Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RStop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Rights: 0 0 0 0 1 0 0 0 0 0 0 0 1 0 0 0 0 1 0 0 Lanes: -----||-----| Volume Module: 0 417 Base Vol: 0 0 0 0 0 0 362 0 Ω 1.00 1.00 1.00 Initial Bse: 0 0 0 0 0 0 0 362 0 0 417 U 0 0 0 0 525 0 0 0 0 0 0 604 PHF Volume: 0 0 0 0 0 0 0 0 0 0 0 Reduct Vol: 0 0 FinalVolume: 0 0 0 0 0 525 0 0 604 Critical Gap Module: Capacity Module: Level Of Service Module: LOS by Move: * * * * * * * * * * * LT - LTR - RT LT - LTR - RT LT - LTR - RT Movement: LT - LTR - RT * * * * * * * * * * Shared LOS: XXXXXX ApproachDel: XXXXXX XXXXXX XXXXXX ApproachLOS: *************************

Note: Queue reported is the number of cars per lane.

APPENDIX Q WARRANT WORKSHEETS

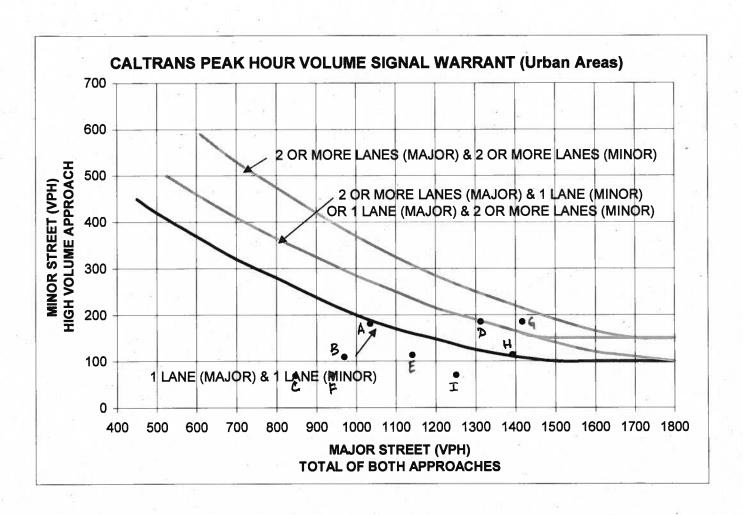


		Major St	Minor St	
	Scenario	San Juan Gr.	Rogge	Warrant
		North/South	EBL+NL	Met?
A.	Existing AM	345	284	No
В.	Existing Mid	533	235	No
C.	Existing PM	407	171	No
D.	Ex+ProPh1 AM	351	342	No
E.	Ex+ProPh1 Mid	535	261	No
F.	Ex+ProPh1 PM	408	186	No
G.	GPBO AM	770	460	Yes
Н.	GPBO MD	1070	380	Yes
١.	GPBO PM	960	300	Yes

Notes:

- 1. 150 VPH applies as the lower threshold volume for a minor street approach with two or more lanes and 100 VPH applies as the lower threshold volume for a minor street approaching with one lane.
- 2. Bold line applies to intersection geometry.

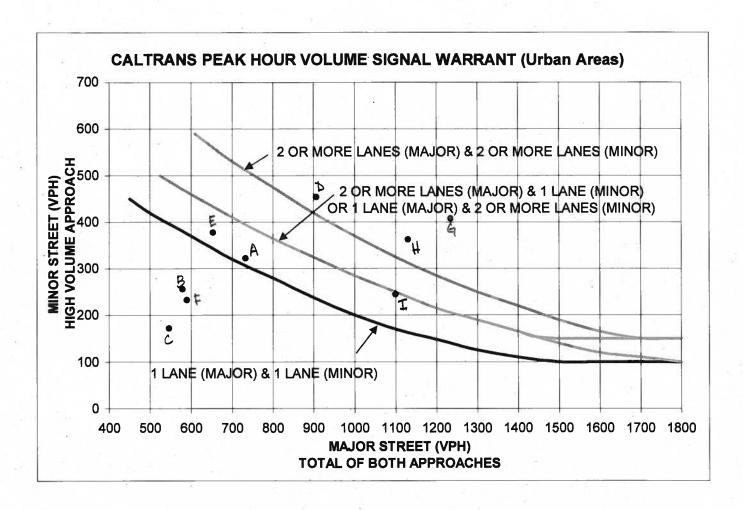
Intersection #5 San Juan Grade - Penzance



		Major St	Minor St	-
	Scenario	San Juan Gr.	Penzance	Warrant
		North/South	East/West	Met?
Α.	Existing AM	1035	181	No
В.	Existing Mid	969	110	No
C.	Existing PM	846	69	No
D.	Ex+ProPh1 AM	1312	185	Yes
E.	Ex+ProPh1 Mid	1141	114	No
F.	Ex+ProPh1 PM	937	71	No
G.	GPBO AM	1417	185	Yes
H.	GPBO MD	1392	114	Yes
l.	GPBO PM	1251	71	No

Notes:

- 1. 150 VPH applies as the lower threshold volume for a minor street approach with two or more lanes and 100 VPH applies as the lower threshold volume for a minor street approaching with one lane.
- 2. Bold line applies to intersection geometry.



		Major St	Minor St	
	Scenario	San Juan Gr.	Rogge	Warrant
	2.0	North/South	East/West	Met?
A.	Existing AM	732	323	Yes
B.	Existing Mid	579	257	No
C.	Existing PM	546	173	No
D.	Ex+ProPh1 AM	907	454	Yes
E.	Ex+ProPh1 MId	653	378	Yes
F.	Ex+ProPh1 PM	589	233	No
G.	GPBO AM	1234	408	Yes
H.	GPBO MD	1130	363	Yes
l.	GPBO PM	1099	245	Yes

Notes:

- 1. 150 VPH applies as the lower threshold volume for a minor street approach with two or more lanes and 100 VPH applies as the lower threshold volume for a minor street approaching with one lane.
- 2. Bold line applies to intersection geometry.

APPENDIX R PEAK HOUR FACTOR CALCULATION

SALINAS #6 HIGH SCHOOL PEAK HOUR FACTOR CALCULATION

	a s	Natividad/Rogge	306	Nat	Natividad/Boronda	nda ,	San Jue	San Juan Grade/Boronda	wonda	San Ju.	San Juan Grade/Russell	nssell	San Jua	San Juan Grade/Penzance	nzance	San Ju	San Juan Grade/Rogge	Rogge		Kelton/Rogge Intersection 7	9 -
	WW	QW OW	¥	W	MD	M	AM	QW	¥	AM	MD	M	W	MD	PM	W	MD	PM	AM	MD	P.
PROJECT PHASE 1 CONDITION Existing Volume	888	1013	713	2219	2784	2839	2500	2868	2892	1630	1280	1080	1223	1085	925	1083	88	743	745	553	414
Existing PHF	0.77	0.8	6.0	98.0	0.89	96.0	0.87	0.94	0.97	0.78	0.77	0.92	98.0	96.0	0.89	0.81	0.84	0.83	0.71	0.69	0.88
% in peak 15 min	32%	31%	28%	29%	28%	26%	29%	27%	28%	32%	32%	27%	28%	27%	28%	31%	30%	30%	35%	36%	28%
Volume in peak 15	282	317	198	645	782	724	718	763	745	223	416	288	356	289	260	337	256	224	262	200	118
Project Volume	100	8	32	8	22	28	153	88	20	277	172	16	287	179	2	324	202	107	332	205	109
Project PHF	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42
% in peak 15 min	%09	48%	81%	%09	48%	61%	80%	48%	61%	%09	48%	61%	%09 80%	48%	61%	%09	48%	61%	%09	48%	61%
Volume in peak 15	8	29	20	22	92	18	16	46	34	165	8	88	171	88	25	183	46	85	198	88	88
Sum of peak 15 min	351	345	218	669	808	742	808	808	9/1	687	498	34	526	374	317	230	353	289	460	588	181
Peak 15 min as one-hour vol.	1406	1381	870	2785	3232	2968	3238	3235	3103	2749	1993	1374	2105	1498	1269	2121	1412	1156	1840	1195	736
Intersection one-hour volume	666	1073	745	2309	2838	2868	2653	2964	2942	1907	1452	1151	1510	1264	1019	1417	1062	820	1077	758	523
Weighted peak hour factor	0.71	0.78	0.86	0.83	0.88	0.97	0.82	0.92	0.95	69.0	0.73	0.84	0.72	28.0	8.0	0.67	0.76	0.74	0.69	0.63	0.71
CUMULATIVE PROJECT BUILDOUT CONDITION	IT CONDITI	NO																	+		7/01/1620
Total Cumulative Volume	1550	1750	1430	6310	7485	7540	6180	7280	7290	5140	2080	3040	1615	1515	1333	1754	1524	1372	362	748	655
Project Volume	83	æ	15	37	22	Ŧ	32	22	F	158	106	25	8	4	83	108	67	98	113	2	88
Existing Volume	888	1013	713	2219	2784	2839	2500	2868	2892	1630	1280	1080	1223	1085	925	1083	980	743	745	553	414
Net Change Ex to Cumulative	288	707	702	4054	4879	4690	3645	4390	4387	3352	3694	1928	324	386	385	553	297	593	10	125	203
Cumulative trips PHF	0.95	0.85	0.95	98.0	0.95	0.95	0.85	98.0	98.0	0.95	0.95	0.95	98.0	0.85	0.95	0.95	0.95	0.85	0.95	0.95	0.95
% in peak 15 min	56%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	36%	56%	26%	26%	26%	26%	26%	26%	56%	26%
Cumulative trips in peak 15	157	185	185	1067	1231	1234	828	1155	1154	882	972	202	8	102	Ď	146	157	126	22	æ	S
Project Volume	SS	g	12	37	8	7	S	22	11	158	106	25	88	4	23	108	. 67	98	113	02	88
Project PHF	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42
% in peak 15 min	%09	48%	61%	%09	48%	61%	80%	48%	61%	%09	48%	61%	%09	48%	61%	%09	48%	61%	%09	48%	61%
Volume in peak 15	83	9	6	ឧ	1	7	21	11	7	22	21	32	9	21	4	2	32	Ø	29	¥	23
Sum of peak 15 min	481	518	392	1734	2024	1965	1698	1929	1907	1499	1439	827	481	411	375	547	445	402	357	267	194
Peak 15 min as one-hour vol.	1923	2071	1568	8836	9608	7861	6794	77.14	7828	5994	5754	3308	1925	1645	1501	2189	1781	1607	1428	1067	E
Intersection one-hour volume	1550	1750	1430	6310	7485	7540	6180	7280	7290	5140	2080	3040	1615	1515	1333	1754	1524	1372	362	748	655
Weighted peak hour factor	0.81	0.86	0.91	0.91	0.92	96.0	0.91	0.94	96.0	98.0	88.0	0.92	0.84	0.92	0.89	080	98.0	0.86	0.67	0.70	28.0

	Bolk	Bollenbacher/Rogge Intersection 8	одде 8	-) <u>-</u>	Jasper/Rogge Intersection 9	9 a		Jace/Rogge mersection 10	10,00	2 -	ns Untersection 11	11	ı ⊆ 2	HS University 2/Rogge Intersection 12	COMP 12	2	Intersection 13	3	2	Intersection 14	4
	AM	QW.	Md	WY	QM	PM	AM	QW	PM	AM	QM	PM	AM	MD	PM	AM	QW	PM	AM	QW	Md
Evisting Volume	707	614	368	576	48	352	298	540	369	288	528	356	88	528	356	288	278	356	888	. 528	356
Existing PHF	0.68	990	0.86	0.73	0.74	99	0.78	0.71	0.93	0.76	0.71	0.83	92.0	0.71	0.93	9.76	0.71	0.93	9.76	0.71	0.93
% in peak 15 min	37%	38%	29%	34%	34%	27%	33%	32%	27%	33%	35%	27%	33%	35%	27%	33%	35%	27%	33%	32%	27%
Volume in peak 15	280	233	107	197	18	22	197	6	88	183	188	88	193	186	88	193	186	98	183	186	98
Project Volume	337	208	111	342	.211	113	395	246	130	466	284	148	161	87	88	22	97	29	100	8	32
Project PHF	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42
% in peak 15 min	%09	48%	81%	%09	48%	61%	80%	48%	61%	%0g	48%	61%	%09	48%	61%	9609	48%	61%	%09 ************************************	48%	61%
Volume in peak 15	201	400	89	8	101	69	232	118	62	277	136	8	88	42	S	88	47	æ	8	53	20
Sum of neak 15 min	461	332	175	401	285	163	432	308	178	471	322	186	289	228	146	249	232	132	253	215	115
Peak 15 min as one-hour vol.	1842	1330	669	1603	1140	920	1727	1233	714	1883	1289	744	1157	911	583	286	830	527	1012	828	461
Intersection one-hour volume	104	822	479	918	755	465	883	786	488	1054	812	2 6	749	615	438	882	625	415	889	588	388
Welghted peak hour factor	0.67	0.62	0.69	0.67	99.0	0.72	0.67	0.64	0.7	99.0	0.63	0.68	0.66	89.0	0.75	0.68	0.67	0.79	0.68	0.68	0.84
CUMULATIVE PROJECT BUILDOUT CONDITION	OUT CONDIT	NO		5 8		200		200	3												
Total Cumulative Volume	828	782	909	803	745	593	880	9//	929	1395	1083	73	1363	2867	747	1333	1065	1111	1339	1052	385
Project Volume	118	g	8	123	92	42	178	111	28	703	430	177	671	8	8	25	412	223	647	380	211
Existing Volume	707	614	388	9/9	544	352	288	240	369	288	528	356	288	528	326	288	528	326	288	228	326
Net Change Ex to Cumulative	\$	38	198	\$	125	189	호	125	138	104	125	198	4	125	198	\$	125	198	\$	125	28
Cumulative trips PHF	0.95	0.95	0.95	0.95	0.95	0.95	980	0.85	98.0	0.95	0.85	0.95	0.95	98.0	0.95	0.95	0.95	0.95	0.85	0.85	0.95
% in peak 15 min	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	26%	56 %	26%	26%	26%	26%
Cumulative trips in peak 15	27	22	25	27	æ	25	27	ಜ	52	27	æ	25	27	æ	25	27	83	25	22	æ	25
Drainet Volume	118	g	9	123	78	42	178	111	95	203	430	177	671	344	193	641	412	223	647	399	211
Design PHF	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42	0.42
% in peak 15 min	%09	48%	61%	%09	48%	61%	%09	48%	61%	9609	48%	61%	9609	48%	61%	%09	48%	61%	9609	48%	61%
Volume in peak 15	20	35	24	22	98	88	106	æ	88	418	206	108	388	165	118	382	198	136	386	192	129
Sum of peak 15 min	328	283	183	298	253	172	330	278	187	639	425	256	620	384	288	802	417	284	909	410	277
Peak 15 min as one-hour vol.	1430	1170	737	1191	1013	989	1320	1105	749	2557	1701	1023	2481	1536	1062	2409	1666	1135	2424	1841	1106
Intersection one-hour volume	828	782	909	88	745	593	880	9//	626	1385	1083	731	1363	286	747	1333	1065	Ш	1339	1052	765
Weighted peak hour factor	0.66	0.67	0.83	0.67	0.74	0.86	0.67	0.70	0.84	0.55	0.64	0.71	99.0	0.65	0.70	0.55	0.64	0.68	99'0	9. 3.	0.69

APPENDIX S HEAVY VEHICLE PERCENTAGES

SALINAS #5 HIGH SCHOOL TRAFFIC ANALYSIS PERCENTAGE HEAVY VEHICLES

	Interse	ection		AM Pea	k Period		20	MidDay P	eak Period			PM Pea	ak Period	
	North-South Street	East-West Street	North Leg	East Leg	South leg	West Leg	North Leg	East Leg	South leg	West Leg	North Leg	East Leg	South leg	West Leg
1	Natividad Road	Rogge Road	5%	-	2%	3%	4%	-	5%	5%	2%	-	1%	1%
2	Natividad Road	Boronda Road	6%	2%	4%	5%	4%	2%	2%	2%	3%	1%	1%	2%
3	San Juan Grade Road	Boronda Road	5%	3%	5%	7%	3%	2%	2%	3%	3%	2%	1%	2%
4	San Juan Grade Road	Russeii Road	5%	-	3%	4%	4%	-	3%	5%	2%	-	2%	3%
5	San Juan Grade Road	Penzance Street	5%	0%	5%	0%	6%	0%	4%	0%	3%	0%	2%	0%
6	San Juan Grade Road	Rogge Road	9%	4%	4%	4%	6%	7%	6%	0%	4%	1%	3%	2%
7	Kelton Drive	Rogge Road	-	4%	0%	4%	-	7%	0%	7%	-	1%	0%	1%
8	Bollenbacher Drive	Rogge Road		4%	0%	4%	- '	7%	0%	7%	-	1%	0%	1%
9	Jasper Way	Rogge Road	- 11	4%	0%	4%	-	7%	0%	7%	-	1%	0%	1%
10	Jade Drive	Rogge Road		4%	0%	4%	- v	7%	0%	7%	-	1%	0%	1%

APPENDIX T TRAFFIC OPERATIONS AND SAFETY AT SCHOOLS: RECOMMENDED GUIDELINES

1. Report No.	2. Government Accession No.	Technical Report Documentation Pa 3. Recipient's Catalog No.
FHWA/TX-04/4286-2	2. Government Accessing No.	J. Recipients Chang No.
4. Title and Subtitle		5. Report Date
TRAFFIC OPERATIONS AND SAFI	ETY AT SCHOOLS: RECOMMENDED	October 2003
GUIDELINES		Resubmitted: January 2004
		6, Performing Organization Code
7. Author(s)		8. Performing Organization Report No.
Scott A. Cooner, Kay Fitzpatrick, Mar	rk D. Wooldridge, and Garry L. Ford	Report 4286-2
9. Performing Organization Name and Address		10. Work Unit No. (TRAIS)
Texas Transportation Institute		The second secon
The Texas A&M University System		11. Contract or Grant No.
College Station, Texas 77843-3135		Project No. 0-4286
12. Sponsoring Agency Name and Address		13. Type of Report and Period Covered
Texas Department of Transportation		Research:
Research and Technology Implementa	ation Office	September 2001 - August 2003
P. O. Box 5080		14. Sponsoring Agency Code
Austin, Texas 78763-5080		

15. Supplementary Notes

Research performed in cooperation with the Texas Department of Transportation and the U.S. Department of Transportation, Federal Highway Administration.

Research Project Title: Operational and Safety Guidelines for Roadway Facilities around Schools

16. Abstract

The state of Texas, particularly the large urban areas, has experienced considerable population growth in recent years. This growth has produced new schools in areas near highways originally designed for low volumes and relatively high speeds. Another trend is the higher proportion of children being transported to and from schools in private vehicles. These realities, and many of the other issues associated with traffic around schools, make it important to aggressively consider the design of roadways within and around schools to ensure the safest possible traffic environment. Equally important is the consideration of the location and design of the school site, preferably during the planning stages, in order to establish safe and efficient operations.

The Texas Department of Transportation (TxDOT) is currently focusing attention on these issues through its Precious Cargo Program. Precious Cargo allows TxDOT staff to review school site plans and make recommendations before the schools are built. Since the program's inception, more than 180 schools in 70 various school districts statewide have seen traffic safety improvements around their schools or future school sites.

This report contains the recommended school site planning guidelines for transportation-related elements such as site selection, general site requirements and design, bus operations, parent drop-off/pick-up zone, driveways, turn lanes, signing and marking, parking, and pedestrian and bicycle access. The research team based these guidelines on a comprehensive review of existing guidelines and the results of field studies at school sites in Texas. The report provides examples of good practices and also examples of practices to avoid for many of the more prominent guidelines. The guidelines are focused on transportation design, operations and safety within school sites – with a particular focus on the parent and bus drop-off/pick-up zones. The final chapter of the report contains a site plan review checklist that TxDOT engineers, field crews, architects, and school district personnel can use to coordinate efforts and make sure that school access is safe and efficient.

17. Key Words School, Safety, Drop-off, Pick-up, De Bus, Site Plan	sign Guidelines, School	public throu National Tee 5285 Port R	ons. This document is ava gh NTIS: chnical Information Serv	
19. Security Classif.(of this report) Unclassified	20. Security Classif.(of Unclassified	this page)	21. No. of Pages 80	22. Price

Guideline 8: Single-file right wheel to the curb is the preferred staging method for buses. (DESIGN and OPERATIONS)

Best Practice for Application of Guideline 8

Guideline 8 refers to the preferred staging method for school buses while loading or unloading students at school sites. The preferred method of staging buses is single-file right wheel to the curb because students are not required to pass between buses.

Figure 13 shows the different staging methods for buses for loading and unloading students at school sites. The bus-loading zone needs to be designed for the expected number of buses to accommodate Guideline 8.

Example of Good Practice

Many schools where researchers performed field studies staged buses in a single-file right wheel to the curb formation. Figure 14 provides a picture of a site where two of the buses were staged in a single-file formation adjacent to the curb for afternoon loading.

Examples to Avoid

The research team observed several sites where the preferred staging method was not employed. The most likely reason for using other staging methods, such as multiple-lane parallel, was lack of space to accommodate the number of buses serving the school campus. In the opinion of the research team that, if possible, staging methods such as the one shown in Figure 15 should be avoided to minimize the risks of conflicts with buses and students in the loading zone area.

GUIDELINES FOR THE DESIGN AND OPERATION OF PARENT ZONES

The topic of design and operation of parent drop-off/pick-up zones at schools has not received considerable attention until recently. Researchers believe that parent drop-off and pick-up zones are often overlooked in school design, but are very important. The provision of adequate zones minimizes illegal standing or parking near schools and helps prevent problems such as blocking bus driveways and flow on adjacent roadways (44). The research team did find some information for guidelines and recommended practices that is provided in Table 9. Several studies, performed in the states of North and South Carolina, have given significant consideration to design and operation of parent drop-off/pick-up zones.

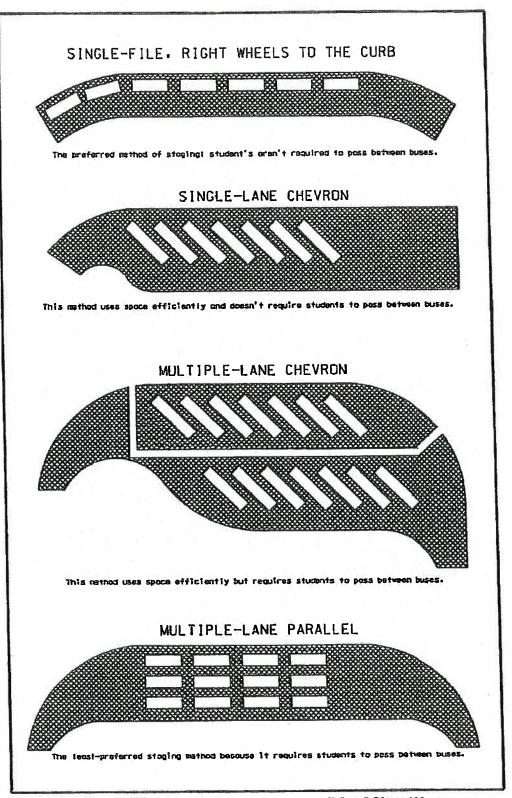


Figure 13. Methods to Stage Buses at School Sites (33).

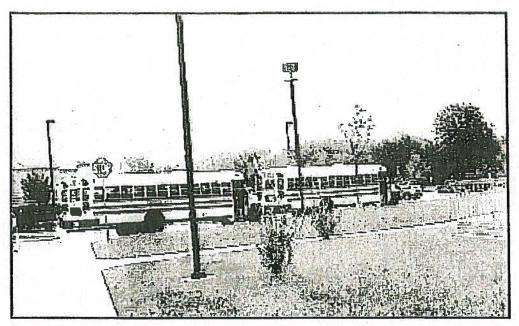


Figure 14. School Buses Staged in Preferred Method - Single-File Right Wheel to the Curb.

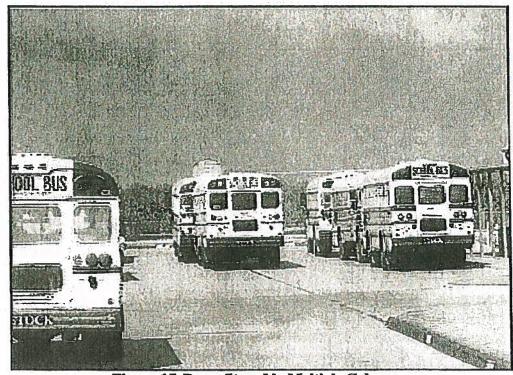


Figure 15. Buses Staged in Multiple Columns – Avoid if Possible to Reduce Potential Conflicts.

Table 9. Guidelines for Design and Operation of Parent Drop-off/Pick-up Zones.

Guideline	Source(s)
Drop-off area design does not require backward movement by vehicles.	Katz, Okitsu, & Associates—California (38), Miami- Dade County—Florida (39), South Carolina DOE (13), Wake County—North Carolina (30), Arizona DOT (25), Douglas County—Colorado (16), North Carolina DOE (9), Missouri DOT (36), Minnesota DOT (23)
★ Parent drop-off/pick-up zones should be one-way in a counterclockwise direction where students are loaded and unloaded directly to the curb/sidewalk.	Miami-Dade County-Florida (39), South Carolina DOE (13), Region of York-Canada (15), ITB Michigan Section (17), North Carolina DOE (9), California DOE (10), Missouri DOT (36), Minnesota DOT (23), Arizona DOT (25)
★ Maximize fronting curb space as loading zone — provide an adequate driveway for lining up cars on site.	Katz, Okitsu, & Associates—California (38), North Carolina DOE (9), Safe School Design Guidelines (45), South Carolina DOT (31), 4286 Research
★ The length of the car pick-up zone can be determined by estimating the maximum number of cars likely to arrive at any one time.	New South Wales-Australia (18), Minnesota DOT (23), North Carolina DOT (46)
Prior to designing and laying out roads and parking, architects should consult with school administrators on: (1) number of cars dropping/picking up students; and (2) type of schedule (staggered or single opening time).	NSC (24), Arizona DOT (25)
Required drop-off and pick-up areas for schools (public or private) shall include at least: (1) 5 auto or (2) one auto space for every 50 students, whichever results in the greater number (no more than 12 spaces required).	City of Henderson-Nevada (43)
Drop-off areas should be at side entrances where site size/frontage permits so that the amount of pavement in front of schools at the street edge is reduced.	City of Mississauga-Canada (12)
★ Do not load or unload students where they have to cross a vehicular path before entering the building.	North Carolina DOE (9)
★ Short-term parking spaces should be identified past the student loading area and near the building entrance.	North Carolina State University (47)
★ Parent loading should occur in designated zones to minimize pedestrian/vehicle conflicts.	4286 Research
★ Student safety patrols and loading supervisors should be well trained and wear reflective safety vests.	4286 Research, North Carolina State University (47)
★ Traffic cones and other channelizing devices can be used to minimize pedestrian/vehicles conflicts.	4286 Research

★ Guidelines with this star symbol also have a best practice section

North Carolina Guidelines for Managing School Carpool Traffic

Some of the most comprehensive studies on the design and operation of drop-off/pick-up zones have occurred in the State of North Carolina. Researchers at the North Carolina State University (NCSU) collected data at 20 elementary schools on the loading process and associated queuing. Based on these studies, NCSU developed a Best Practice for Managing School Carpool Traffic Schematic (47). Figure 16 replicates this schematic and the corresponding guidelines. NCSU also produced a web-based school carpool decision support tool that provides procedural recommendations based on the common problems during school drop-off/pick-up times (48). For example, if the problem is that parent's vehicles are spilling back out of the site onto adjacent roads, implementation of a dual queue lane is recommended for the purpose of increasing storage capacity.

The North Carolina DOT also has the Municipal School and Transportation Assistance (MSTA) group dedicated to addressing safety concerns and traffic operations on school campuses and the surrounding state roadways (49). The MSTA, based on data collected at numerous schools throughout the state, has developed a design tool called the School Traffic Calculator (46). This tool estimates the morning and afternoon traffic loads and the corresponding maximum queue lengths that can be used to size the drop-off/pick-up zone.

South Carolina Guidelines for On-Site Stacking Length

The South Carolina DOT also has a dedicated unit for handling school-related transportation issues. This unit recently published a document entitled *Guidelines for School Transportation Design (31)*. This document contains information, provided in Table 10, regarding recommended on-site stacking lengths ranging from 800 to 1500 ft (244 to 458 m) depending on the school type and student population.

Table 10. South Carolina DOT Recommendations for On-Site Stacking Length (31).

School Type	Student Population	Loop Drive Stacking Length (linear feet) (m)
Elementary	200 - 600 600 - 1400	900 - 1200 (274.5 - 366) 1200 - 1500 (366 - 457.5)
Middle	200 600 600 1200	900 1200 (274.5 366) 1200 1500 (366 457.5)
High	400 - 800 800 - 2500	800 - 1200 (244 - 366) 1200 - 1500 (366 - 457.5)

Note: For high school populations greater than 2500 students, consider two separate student pick-up/drop-off loops.

It should be noted that many of the school sites in South Carolina utilize a single two-way driveway (i.e., driveway serves as the entrance and exit) for the parent zone in order to increase the stacking length. This type of design is not as prominent in Texas schools where most sites have separate entrance and exit points, which can decrease the available stacking space.

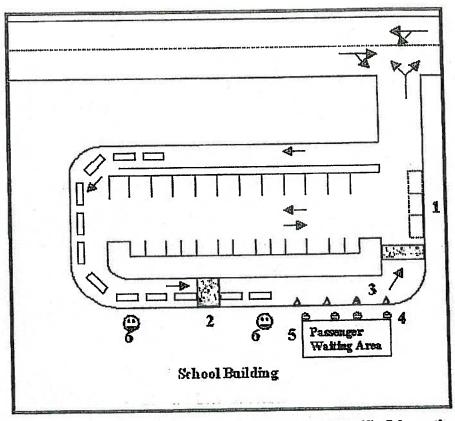


Figure 16. Best Practice for Managing School Carpool Traffic Schematic (47).

- Short-term parking spaces should be identified past the student loading area and near the building entrance.
 These spaces can be identified by installing 'Visitor Parking' signs at the designated spaces and should be used for parents requiring an extended period of time to load or unload.
- Crosswalks should be clearly marked with the first choice location being before the loading area and the second choice location after the loading area.
- 3. Make sure there is clear demarcation of the bays in the loading area.
 - a. Paint the loading area into separate bays by installing 4-inch white solid pavement markings; each bay should be a minimum of 8 feet wide.
 - b. The end bays should be at least a minimum length of 20 feet and the middle bays should be at least a minimum length of 30 feet. There should be a maximum of 4-5 bays.
- 4. Each bay should have its own safety assistant, trained by teachers at the beginning of every school year.
 - a. One safety assistant should be present in each loading bay.
 - b. This safety assistant is responsible for assisting the child(ren) into or out of their vehicle.
 - c. Each safety assistant should wear an orange safety vest to provide visibility and to be easily identified by children and drivers.
- 5. At the end of the school day, have children wait in an organized fashion in the loading area or adjacent to it.
 - a. Organization allows for children to pay attention and hear their name or number called.
 - b. This helps to expedite the loading process by getting children to their vehicles quicker.
 - c. It also helps the carpool time to be safe, as children will not be left to run around unsupervised.
- Implement an Advanced Passenger Identification system using numbers or name cards placed in the windshield of the vehicle waiting in the carpool.
 - a. This will require at least two people. The first person should stand five or six cars before the loading area and call out the names of the children over a walkie-talkie to the second person.
 - b. The second staff member should be standing in the loading area itself relaying the names or numbers with a speaker system and directing students to the appropriate bay.

Guideline 9: Provide an adequate driveway for stacking cars on site. (DESIGN)

The research team found several examples of guidelines similar in nature to Guideline 9. Having adequate on-site stacking length to accommodate parent vehicles during the morning drop-off and afternoon pick-up operations is important. One of the primary focuses of the field studies during the 4286 project was to examine geometric design and operational practices in parent drop-off/pick-up zones. Researchers concentrated on collecting sufficient data at elementary schools in Texas to be able to validate the existing South Carolina (31) and North Carolina (46) guidelines for on-site stacking length.

The data collected during the 4286 field studies validated the School Traffic Calculator (46). It is good practice to use the afternoon pick-up data to predict the maximum queue of vehicles. The maximum queue length is then used to design and appropriate size the length needed in the parent driveway for lining up cars on site. The analysis of the average, maximum, and 95th percentile queue data at Texas schools did not produce any statistically significant models based on a regression analysis. The data did show that the observed maximum queue lengths were often well below the recommended on-site stacking lengths given in Table 10 and those predicted by the School Traffic Calculator (46).

It appears the South Carolina and North Carolina recommended on-site stacking lengths were more conservative compared to the Texas data. Based on this finding, the research team feels that the recommended on-site stacking lengths for Texas schools can be decreased and will still be able to meet the objective of Guideline 9 – providing an adequate driveway for stacking cars on site. Even though no statistically significant models were developed based on queue length, the research team had sufficient data to formulate recommended on-site stacking lengths for Texas elementary and middle schools. Based on the data from this project, researchers recommend the on-site stacking lengths for high schools contained in Table 10 for Texas because no new field data were collected at Texas high schools (7). Table 11 provides the recommended on-site stacking lengths for Texas schools.

Table 11. Recommended Parent Drop-off/Pick-up Zone On-Site Stacking Length for Texas.

School Type	Student Population	Loop Drive Stacking Length (linear feet) (m)
Elementary	Less than 500 500 or more	400 – 750 (122 – 229) 750 – 1500 (229 – 458)
Middle	Less than 600 600 or more	500 - 800 (153 - 244) 800 - 1600 (244 - 488)
High (31)	400 800 800 2500	800 - 1200 (244 - 366) 1200 - 1500 (366 - 458)

Note: For high school populations greater than 2500 students, consider two separate student pick-up/drop-off loops.

Best Practice for Application of Guideline 9

Providing adequate on-site stacking length is important to the safety and operations of traffic within and around the school site.

Examples to Avoid

During the case studies and field studies, the research team observed many sites that did not provide adequate on-site stacking length. The inadequate on-site space to accommodate the queue led to spillback on adjacent roadways. Figure 17 shows an intermediate school site where the both lanes of the northbound direction of the adjacent roadway were blocked by the queue of vehicles that backed up out of the parent drop-off/pick-up zone driveway. Figure 18 shows another example of queue spillback at an elementary school site.

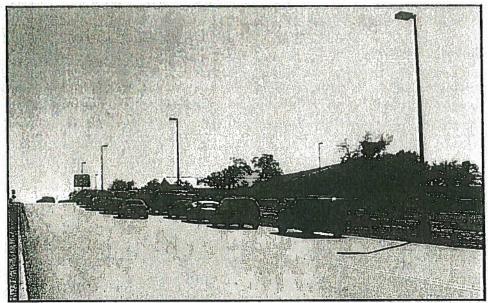


Figure 17. Example of Queue Spillback from the School Site.

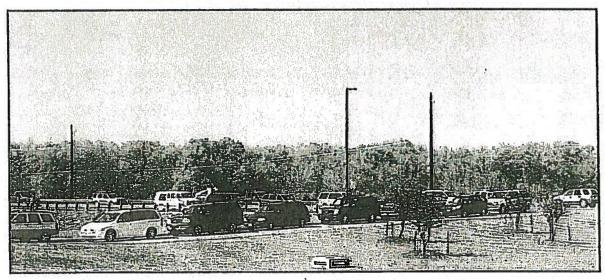


Figure 18. Another Example of Queue Spillback from the School Site.

APPENDIX U INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEET

El Dorado Drive/Rogge Road

Tue Jun 21, 2011 15:14:49 GPBO AM ______ Level Of Service Computation Report 2000 HCM Operations Method (Base Volume Alternative) ******************** Intersection #15 Rogge Rd/El Dorado Dr ******************************* Critical Vol./Cap.(X): Cycle (sec): 100 0.913 Loss Time (sec): 9
Optimal Cycle: 110 Average Delay (sec/veh): 40.0 110 Level Of Service: ***************************** North Bound South Bound Approach: East Bound West Bound L-T-R L-T-R L-T-RMovement: -----||----||----||-----| Split Phase Split Phase Protected Include Include Include Control: Protected Include Include Include 0 0 0 0 6 6 6 6 Rights: Include 6 0 6 Min. Green: 0 Lanes: Volume Module: 410 0 161 0 0 0 332 309 Base Vol: 0 139 288 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 410 0 161 0 0 0 0 332 309 139 288 0 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 PHF Adj: 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 0.62 . 0 260 0 535 PHF Volume: 661 0 0 0 498 224 465 . 0 0 0 0 0 0 0 0 Reduct Vol: · 0 0 0 0 Reduced Vol: 661 0 260 0 0 0 0 535 498 224 465 PCE Adj: 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 661 0 260 0 - 0 0 0 535 498 224 465 n Saturation Flow Module: Sat/Lane: Adjustment: 0.91 1.00 0.82 1.00 1.00 1.00 1.00 0.96 0.82 0.91 0.96 1.00 Final Sat.: 1736 0 1554 0 0 0 0 1828 1554 1736 1828 Capacity Analysis Module: Vol/Sat: 0.38 0.00 0.17 0.00 0.00 0.00 0.00 0.29 0.32 0.13 0.25 0.00 Crit Moves: **** Green/Cycle: 0.42 0.00 0.42 0.00 0.00 0.00 0.00 0.35 0.35 0.14 0.49 0.00 Volume/Cap: 0.91 0.00 0.40 0.00 0.00 0.00 0.00 0.83 0.91 0.91 0.52 Uniform Del: 27.4 0.0 20.4 0.0 0.0 0.0 0.0 29.8 31.0 42.3 17.2 IncremntDel: 15.9 0.0 0.4 0.0 0.0 0.0 9.2 19.8 35.0 0.5 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Delay Adj: 1.00 0.00 1.00 0.00 0.00 0.00 0.00 1.00 1.00 1.00 1.00 Delay/Veh: 43.4 0.0 20.8 0.0 0.0 0.0 0.0 38.9 50.7 77.3 17.8 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 43.4 0.0 20.8 0.0 0.0 0.0 0.0 38.9 50.7 77.3 17.8 0.0 С LOS by Move: D A Α Α A A D D \mathbf{E} В HCM2kAvqQ: 23 0 6 0 0 0 0 18 19 10

Note: Queue reported is the number of cars per lane. ***********************************

Approach:

West Bound

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

South Bound

North Bound

L - T - R L - T - RL - T - R Movement: L - T - RSplit Phase Control: Split Phase Protected Protected Rights: Include Include Include Include Min. Green: 6 0 6 0 0 0 0 6 6 6 6 0 4.0 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 1 0 0 0 1 0 0 0 0 0 Lanes: 0 0 1 0 1 1 0 1 0 0

East Bound

0.26

0.13 0.21 0.00

Volume Module: Base Vol: 207 0 148 0 0 0 0 29.7 283 158 265 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Initial Bse: 207 0 148 0 0 0 0 297 283 158 265 0 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 User Adj: 1.00 1.00 1.00 1.00 PHF Adj: 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 0.71 PHF Volume: 292 0 208 0 0 0 0 399 418 223 373

0 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 208 Reduced Vol: 292 0 0 0 0 418 0 399 223 373 PCE Adi: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 FinalVolume: 292 0 208 0 0 0 0 418 399 223 373

Saturation Flow Module: 1900 1900 1900 1900 1900 1900 1900 Sat/Lane: 1900 1900 1900 1900 1900 1.00 Adjustment: 0.89 1.00 0.79 1.00 1.00 1.00 0.94 0.79 0.89 0.94 1.00 Lanes: 1.00 0.00 1.00 0.00 0.00 0.00 0.00 1.00 1.00 1.00 1.00

Capacity Analysis Module:
Vol/Sat: 0.17 0.00 0.14 0.00 0.00 0.00 0.00 0.24

*** Crit Moves: Green/Cycle: 0.28 0.00 0.28 0.00 0.00 0.00 0.00 0.42 0.42 0.21 0.63 Volume/Cap: 0.62 0.00 0.50 0.00 0.00 0.00 0.00 0.56 0.62 0.62 0.33 0.00 Uniform Del: 31.6 0.0 30.4 0.0 0.0 0.0 0.0 21.8 22.7 35.9 8.5 0.0 IncremntDel: 2.7 0.0 0.9 0.0 0.0 0.0 0.0 0.9 1.9 3.5 0.2 0.0 0.0 0.0 InitQueuDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 1.00 Delay Adj: 1.00 0.00 0.00 0.00 0.00 0.00 1.00 1.00 1.00 1.00 0.00 Delay/Veh: 34.3 0.0 31.3 0.0 0.0 0.0 0.0 22.8 24.6 39.3 0.0

User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 AdjDel/Veh: 34.3 0.0 31.3 0.0 0.0 0.0 0.0 22.8 24.6 39.3 8.7 0.0 C С LOS by Move: Α Α Α Α C С Α D Α Α 9 0 6 0 0 0 0 10 10

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

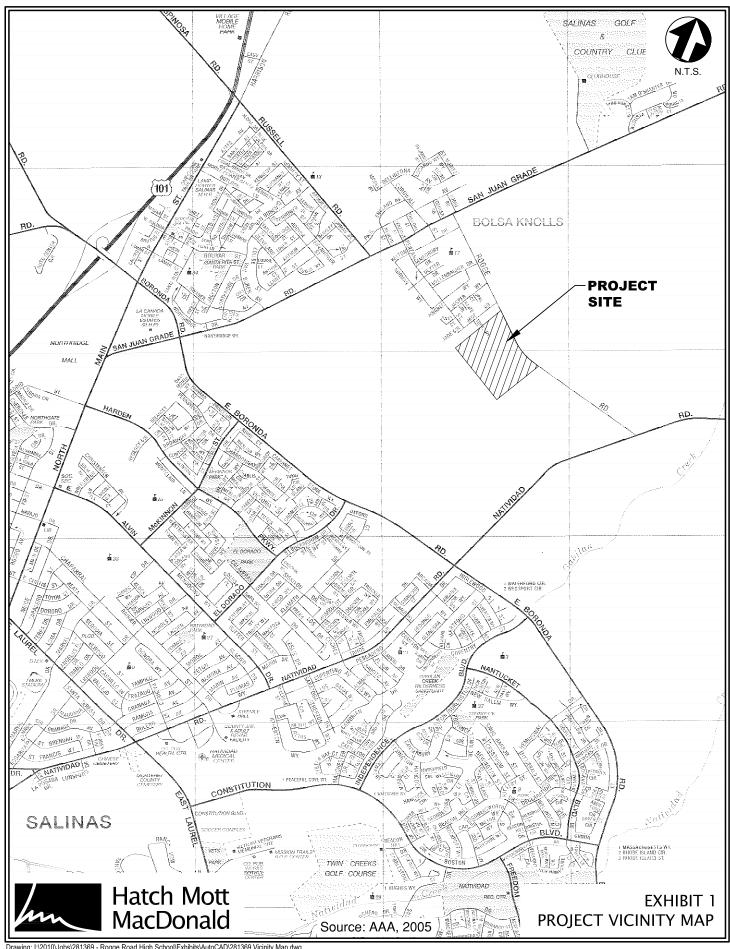
************************** Intersection #15 Rogge / El Dorado ******************

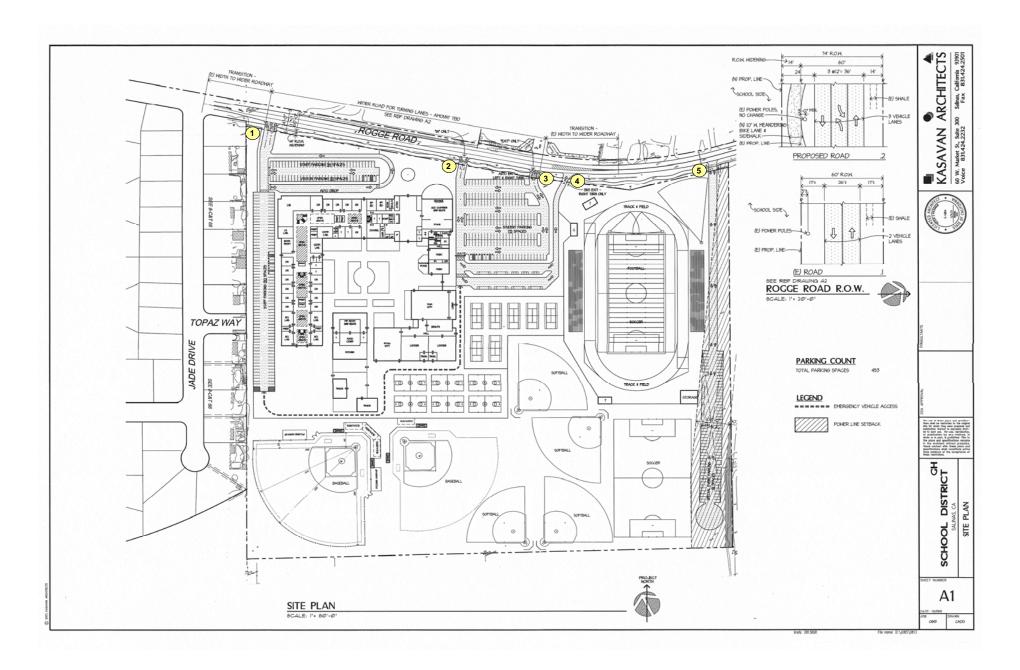
100 Critical Vol./Cap.(X): 0.300 Cycle (sec): Loss Time (sec): 9
Optimal Cycle: 27 Average Delay (sec/veh): 21.1

Level Of Service: **************************

Approach: Movement:	L ·		- R	L -		- R	Ŀ.		- R	L -	est Bo	- R
Control: Rights:	Sp.	lit Ph Inclu O	ase ide 6	Sp.	lit Ph Inclu	ase de	P:	rotect Inclu	ed ide	Pı	otect Inclu	ed de 0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 (0 0	0 1	0 (0 0	0 0	0 (0 1	0 1	1 () 1	0 0
	-					1						
Volume Module Base Vol:	e: 150		07	0	0	0	· · · · · · · ·	195	153	125	267	0
Growth Adj:		1.00			1.00	1.00		1.00	1.00	1.00		1.00
Initial Bse:		0	87	0	0	0	0	195	153	125	267	0
User Adj:		-	1.00	_	1.00	1.00	•	1.00	1.00	1.00		1.00
_	1.00		1.00	1.00		1.00		1.00	1.00	1.00		1.00
_	150		87	0	0	0	0	195	153	125	267	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:		0		0	0	0	0	195	153	125	267	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:		0		0	0	0	0	195	153	125	267	0
Saturation D			•									
Saturation F				1000	1000	1000	1000	1000	1000	1000	1000	1000
Sat/Lane:	0.89	1900	1900 0.79	1.00	1900	1900 1.00		1900 0.94	1900 0.79		1900	1900
Adjustment: Lanes:	1.00		1.00	0.00		0.00		1.00	1.00	0.89		1.00
Final Sat.:		0.00	1510	0.00	0.00	0.00		1777	1510	1688		0.00
						1						1
Capacity Anal							<u> </u>			İ		
Vol/Sat:		0.00	0.06	0.00	0.00	0.00	0.00	0.11	0.10	0.07	0.15	0.00
0110 110 100 1	****							****		****		
Green/Cycle:			0.30	0.00		0.00		0.37	0.37	0.25		0.00
Volume/Cap:			0.19		0.00	0.00		0.30	0.28	0.30		0.00
Uniform Del:		0.0	26.3	0.0	0.0	0.0		22.6	22.3	30.6	8.8	0.0
IncremntDel:	0.3	0.0	0.2	0.0	0.0	0.0	0.0	0.3	0.3		0.1	0.0
InitQueuDel:		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj: Delay/Veh:			1.00 26.5		0.00	0.00		1.00	1.00		1.00	0.00
User DelAdj:		0.0	1.00	0.0	1.00	1.00		22.8	1.00	31.0	8.9	0.0
AdjDel/Veh:	27.5	0.0	26.5	0.0	0.0	0.0		22.8	22.6	31.0	1.00	1.00
LOS by Move:			20.3 C	0.0 A		0.0 A	0.0 A		C		0.9 A	0.0 A
HCM2kAvqQ:			2	Ô	- 0	0	· 0	4	3	3	4	Ô
*****	****	*****	*****	****	*****	****	****	- *****	*****	****	*****	*****

Note: Queue reported is the number of cars per lane. ************************************





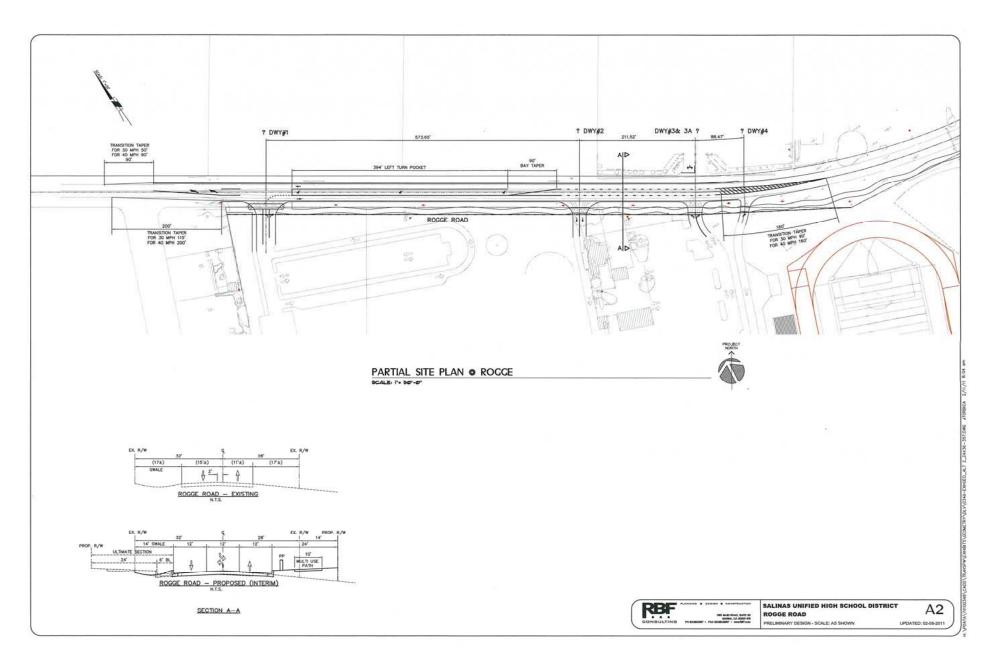
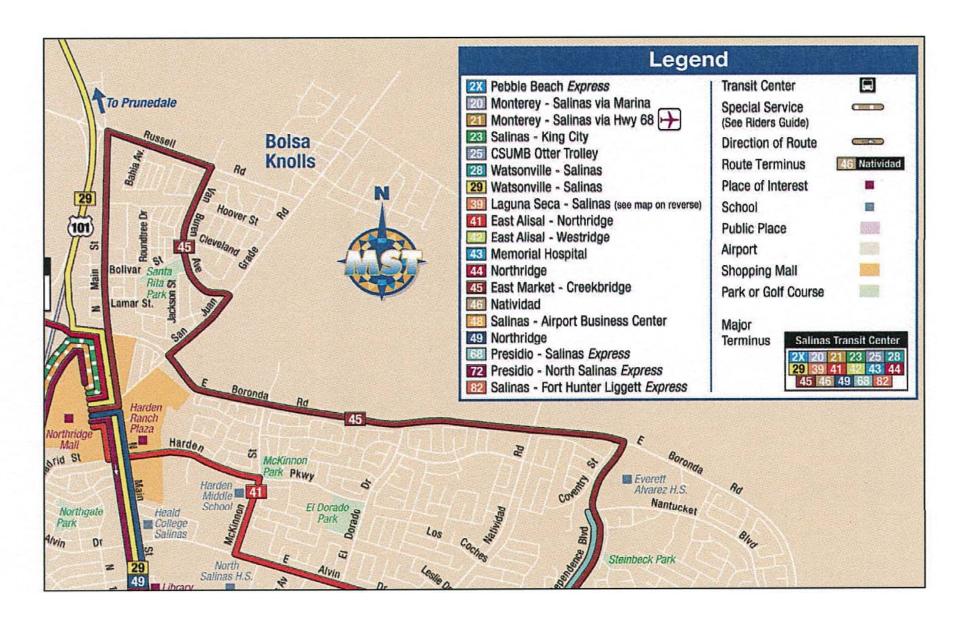
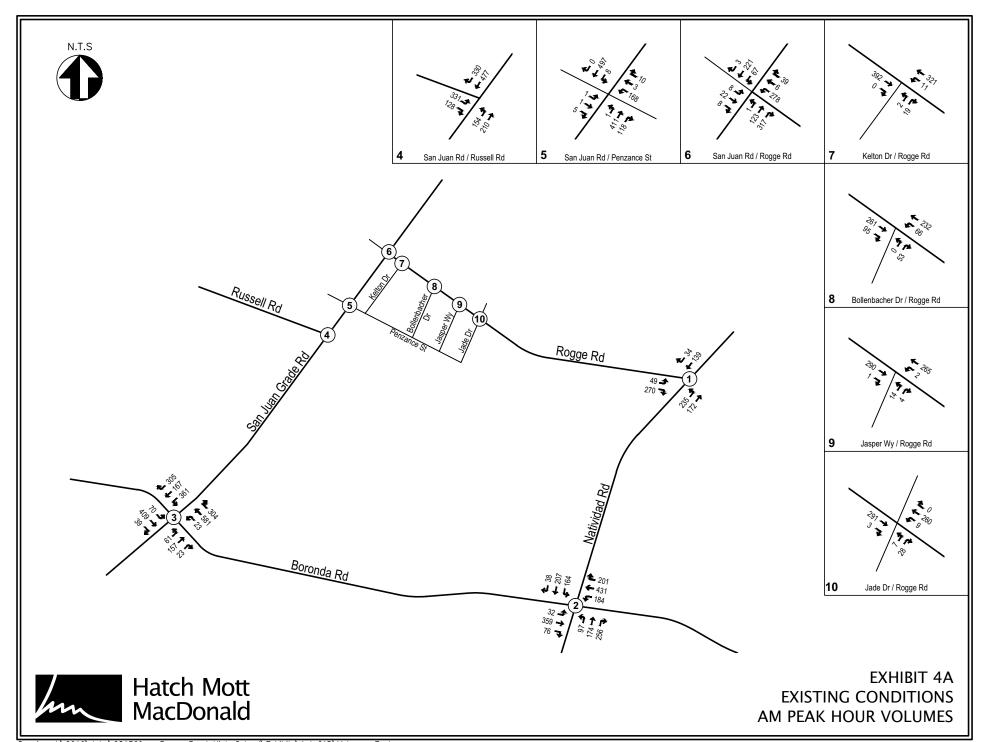
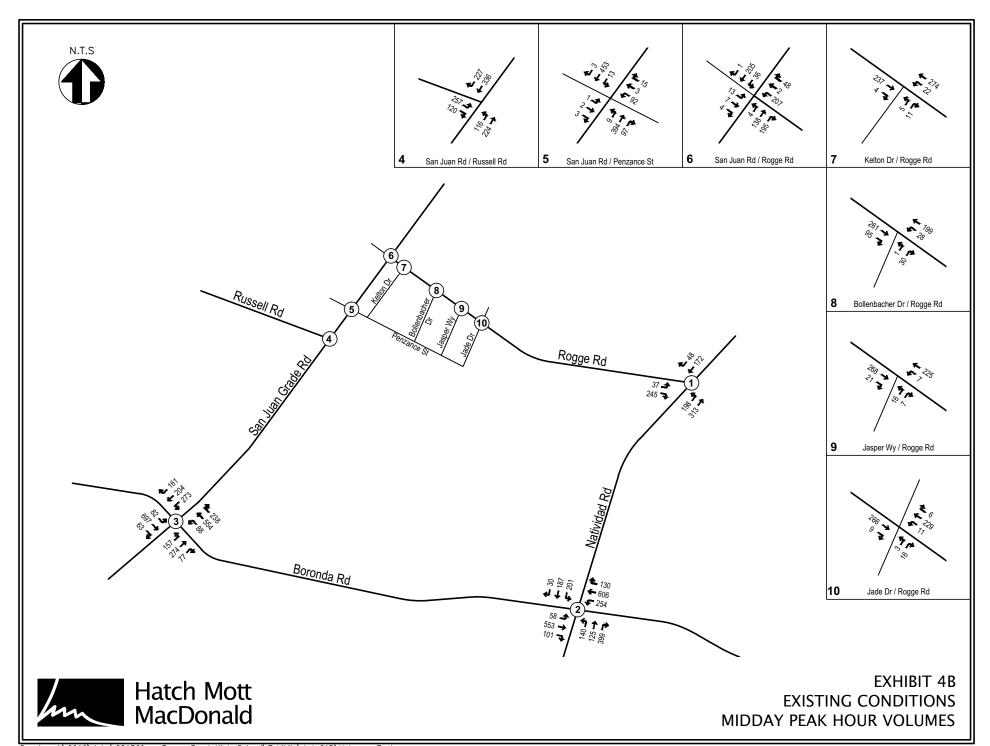
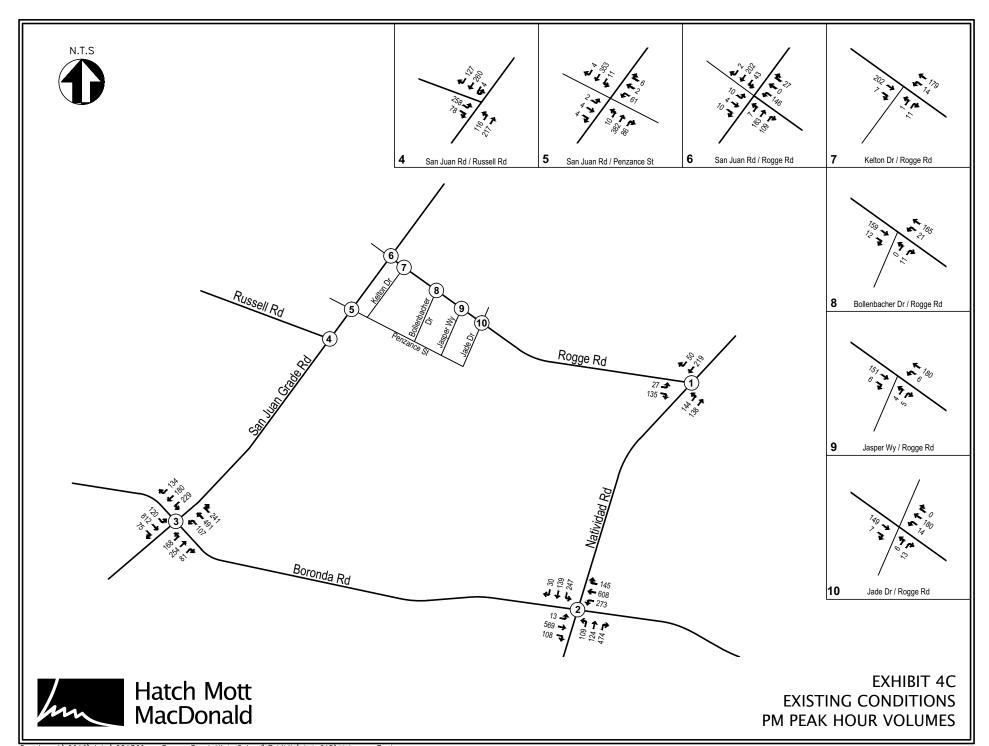


EXHIBIT 2B PROPOSED ROGGE ROAD IMPROVEMENT PLAN









					T																		
w o. 1			Existing Lane	Existing Intersection	LOS	Existing Conditions						Existing + Project Phase 1 Conditions						Cumulative + Project Buildout Conditions					
								MD Pk Hr		PM Pk Hr		AM Peak Hr		MD Pk Hr		PM Pk Hr		AM Peak Hr		MD Pk Hr		PM Pk Hr	
	N-S Street		Configuration	Control	Standard	AM Pe Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS
1 N	latividad Road	Rogge Road	NB 1-L, 1-T SB 1-L/R EB 1-L, 1-R	Two-Way Stop-Overall (Worst Approach) W/Improvement	D (E)	7.5 15.0	A B	5.8 14.8	A B	4.3 11.7	A B	10.1 20.8	B C	6.7 16.5	A C	4.7 12.3	A B	88.7 274.6 24.0	F F C	57.9 224.9 19.2	F F	13.0 59.1 18.3	В F В
2 N	latividad Road	Boronda Road	NB 1-L, 1-T, 1-R SB 1-L, 1-T/R EB 1-L, 1-T, 1-R WB 1-L, 1-T/R	Signal W/Improvement 1 W/Improvement 1 & 2	D	34.0	С	40.4	D	40.1	D	37.5	D	43.2	D	40.9	D	483.2 39.6 34.8	F D C	624.3 66.5 52.8	F E D	571.4 51.4 50.8	F <i>D D</i>
3 San Juar	n Grade Road	Boronda Road		Signal W/Improvement 1 W/Improvement 1 & 2	D	27.8	С	28.1	С	26.9	С	28.5	С	28.5	С	27.3	С	75.7 50.1 37.3	E D D	155.2 74.3 50.0	F E D	196.2 88.8 32.7	F F C
4 San Juar	n Grade Road	Russell Road	NB 1-L, 1-T SB 1-L, 1-T, 1-R 1-L, 1-R	Signal W/Improvement 1 W/Improvement 1 & 2	D	28.4	С	23.7	С	25.1	С	41.6	D	24.6	С	25.0	С	70.7 70.7 46.6	E E D	71.4 38.6 31.4	E D C	31.7 31.6 30.1	c c c
5 San Juar	n Grade Road	Penzance Street	NB 1-L/T/R SB 1-L, 1-T/R EB 1-L/T/R WB 1-L/T/R	Two-Way Stop-Overall (Worst Approach) W/Improvement	D (E)	5.6 36.8	A E	2.2 19.3	A C	1.6 16.8	A C	31.6 255.7 25.1	D F C	3.0 30.4 23.3	A D C	1.8 21.1 23.0	A C C	15.4 132.6 19.9	С F В	2.9 34.8 12.8	A D	1.7 25.6 9.8	A D
6 San Juar	n Grade Road	Rogge Road	NB 1-L/T, 1-R SB 1-L/T, 1-T/R EB 1-L/T/R WB 1-L, 1-T/R	All-Way Stop W/Improvement	D	18.3	С	12.0	В	10.7	В	133.6 23.5	F C	23.0 21.2	C C	13.2 22.0	В	49.2 24.7	E C	23.8 21.6	C C	26.4 19.8	D B
7	Kelton Drive	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop-Overall (Worst Approach)	D (E)	0.5 12.9	A B	0.7 12.1	A B	0.5 9.7	A A	0.6 22.4	A C	0.6 15.6	A C	0.5 10.7	A B	0.5 16.4	A C	0.6 14.2	A B	0.4 10.7	A B
8 Boller	nbacher Drive	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop-Overall (Worst Approach)	D (E)	1.7 11.8	A B	1.0 11.7	A B	0.7 9.3	A A	1.8 19.6	A C	0.9 13.8	A B	0.6 10.0	A A	1.6 14.7	A B	0.9 13.1	A B	0.5 9.9	A A



			Existing	Existing		Existing Conditions							Exis	sting + Pr Cond	oject Pha litions	ise 1		Cumulative + Project Buildout Conditions					
	N-S	E-W	Lane Configuration	Intersection Control	LOS Standard	AM P	AM Peak Hr		MD Pk Hr		PM Pk Hr		AM Peak Hr		MD Pk Hr		PM Pk Hr		AM Peak Hr		Pk Hr	PM Pk Hr	
	Street	Street	J		- Lundard	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS
9	Jasper Way	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop-Overall (Worst Approach)	D (E)	0.5 14.3	A B	0.7 13.3	A B	0.4 9.8	A A	0.9 35.4	A E	0.7 19.3	A C	0.4 11.2	A B	0.6 20.6	A C	0.6 16.8	A C	0.3 11.4	A B
10	Jade Drive	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop-Overall (Worst Approach)	D (E)	0.8 11.8	A B	0.5 9.5	A A	0.8 9.8	A A	2.1 26.9	A D	0.9 13.8	A B	1.0 11.1	A B	1.6 16.6	A C	0.9 13.0	A B	0.8 11.1	A B
11	High School Driveway 1	Rogge Road	EB 1-T WB 1-T	Two-Way Stop-Overall (Worst Approach)	D (E)	-	-	-	-	-	-	18.5 99.8	C F	3.4 20.3	A C	1.5 12.2	A B	200.4 850.2	F F	9.4 40.7	A E	2.2 12.9	A B
12	High School Driveway 2	Rogge Road	EB 1-T WB 1-T	Two-Way Stop-Overall (Worst Approach)	D (E)	-	-	-	-	-	-	0.3 9.0	A A	0.1 8.3	A A	0.1 7.8	A A	1.3 15.0	A B	0.1 9.4	A A	0.6 8.6	A A
13	High School Driveway 3	Rogge Road	EB 1-T WB 1-T	Two-Way Stop-Overall (Worst Approach)	D (E)	-	-	-	-	-	-	0.0 0.0	A A	1.4 17.0	A C	0.9 11.5	A B	0.0 0.0	A A	1.8 22.5	A C	1.0 13.7	A B
14	High School Driveway 4	Rogge Road	EB 1-T WB 1-T	Two-Way Stop-Overall (Worst Approach)	D (E)	- -	- -	-	-	-	-	0.2 11.6	A B	0.2 11.1	A B	0.0 0.0	A A	0.1 20.6	A C	0.2 16.2	A B	0.0 0.0	A A
15	El Dorado Drive	Rogge Road	EB 1-T WB 1-T	Signal (Future)	D	-	-	-	-	-	-	-	-	-	-	-	-	40.0	D	25.0	С	21.1	С

- 1. L, T, R = Left, Through, Right.
- NB, SB, EB, WB = Northbound, Southbound, Eastbound, Westbound.
 Analysis performed using 2000 Highway Capacity Manual methodologies.
- 4. Worst approach level of service standard is generally LOS E. Level of service "F" is the level of service at which improvements would be required.
- 5. Levels of service cited under Mitigations use recommended improvements shown on Exhibit 5B.
- 6. * = Delay is over 300 seconds (5 minutes)
- 7. Operations in **bold** represent significant impacts.
- 8. Modified Access Plan: Levels of Service based on access plan depicted on Exhibit 5C.



			Existing	Existing		
	N-S Street	E-W Street	Lane Configuration	Intersection Control	Existing + Project Phase 1 Conditions	Cumulative + Project Buildout Conditions
_	Olicet	Olicci	Comiguration	Control	Conditions	Conditions
1	Natividad Road	Rogge Road	NB 1-L, 1-T SB 1-L/R EB 1-L, 1-R	Two-Way Stop	Lenghten the NB left turn lane to provide 480 feet for vehicle storage plus additional distance for vehicle deceleration.	Signalize Add free right turn lane on eastbound approach.
2	Natividad Road	Boronda Road	NB 1-L, 1-T, 1-R SB 1-L, 1-T/R EB 1-L, 1-T, 1-R WB 1-L, 1-T/R	Signal	None Required	Construct Salinas Traffic Improvement Program improvements: a. Provide 2 LT lanes, 2 T lanes and 1 RT lane on the NB & SB approaches. b. Provide 2 LT lanes, 3 T lanes and 1 RT lane on the EB & WB approaches. 2. Add a fourth through lane on WB Boronda Rd.
3	San Juan Grade Road	Boronda Road	NB 2-L, 2-T, 1-R SB 2-L, 2-T, 1-R EB 2-L, 1-T, 1-T/R WB 2-L, 3-T, 1-R	Signal	None Required	Construct Salinas Traffic Improvement Program improvements: a. Provide 2 LT lanes, 3 T lanes and 1 RT lane on the EB approach. In addition to #1, construct the following: a. Improve northbound to eastbound right turn to free right turn movement. b. Operate SB right turn with overlap phase.
4	San Juan Grade Road 	Russell Road	NB 1-L, 1-T SB 1-L, 1-T, 1-R EB 1-L, 1-R	Signal	None Required	1. Construct Salinas Traffic Improvement Program improvements: a. Extend Russell Road to the east. b. Provide 2 LT lanes, 1 T lane and 1 shared T/R lane on the NB approach. c. Provide 2 LT lanes, 2 T lanes and 1 RT lane on the SB, EB & WB approaches. 2. In addition to #1, construct the following: a. Add a right turn lane to the northbound San Juan Grade approach.
5	San Juan Grade Road	Penzance Street	NB 1-L/T/R SB 1-L, 1-T/R EB 1-L/T/R WB 1-L/T/R	Two-Way Stop	Signalize Add left turn lane to northbound San Juan Grade Rd approach to Penzance St.	Signalize Add left turn lane to northbound San Juan Grade Rd approach to Penzance St.
6	San Juan Grade Road	Rogge Road	NB 1-L/T, 1-R SB 1-L/T, 1-T/R EB 1-L/T/R WB 1-L, 1-T/R	All-Way Stop	Signalize Add NB LT lane. Modify SB approach to provide LT lane and 1 shared T/R lane. Restripe WB lanes to provide LT lane and 1 shared LT/R lane. Operate E & W approaches with split signal phasing. Operate NB SJG right turn with overlap phase.	1. Signalize 2. Add NB LT lane. 3. Modify SB approach to provide 1 LT lane and 1 shared T/R lane. 4. Restripe WB lanes to provide 1 LT lane and 1 shared LT/R lane. 5. Operate E & W approaches with split signal phasing. 6. Operate NB SJG right turn with overlap phase.
7	Kelton Drive	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop	None Required	None Required
8	Bollenbacher Drive	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop	None Required	None Required
9	Jasper Way	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop	None Required	None Required
10	Jade Drive	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L/R	Two-Way Stop	None Required	None Required
11	High School Driveway 1	Rogge Road	EB 1-T WB 1-T	Future Intersection	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.
12	High School Driveway 2	Rogge Road	EB 1-T WB 1-T	Future Intersection	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.
13	High School Driveway 3	Rogge Road	EB 1-T WB 1-T	Future Intersection	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.
14	High School Driveway 4	Rogge Road	EB 1-T WB 1-T	Future Intersection	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.	Modify the school access plan to provide adequate traffic operations at the Rogge Road intersections with the school driveways.
15	El Dorado Drive	Rogge Road	EB 1-T WB 1-T	Future Intersection	Not Applicable	Provide WB left turn lane. Provide EB right turn lane. Provide separate left turn lane and right turn lane on the NB approach. Signalize.

- NOTES:
 1. L, T, R = Left, Through, Right.
 2. NB, SB, EB, WB = Northbound, Southbound, Eastbound, Westbound.



		Student Population		Traffic Volumes											Trip	Genera	tion Ra	ates (tr	ips/stι	ident)			
School	Date of Counts		Dailv	AM Peak Hour		ur	PM School Peak Hour				M Stree		Daily	AM Peak Hour			PM School Peak Hour			PM Street Peak Hour			Notes
		.,		1 1 1		ОВ	Total		ОВ	Total				Total		ОВ	Total		"OB	Total		"OB	
Gilroy High School	May 2006	2,250	_	1,503	923	580	869	357	512	-	-	-	-	0.67	0.41	0.26	0.39	0.16	0.23	-	-		40% of students eligible to be bused to school; actual percentage riding bus closer to 35%
Aptos High School	March 2004	1,825	-	1,007	607	400	644	257	387	1	-	-	-	0.55	0.33	0.22	0.35	0.14	0.21	-	-	-	38% of students are bused to school
Seaside High School	March 2008	1,355	3,246	667	411	256	455	207	248	371	169	202	2.40	0.49	0.30	0.19	0.33	0.15	0.18	0.27	0.12		Wednesdays are half-days (school ends at 1:20 PM); 27% of students in Monterey Peninsula Unif. Sch. Dist. are bused
Everett Alvarez High School	March 2008	2,140	3,827	1,143	669	474	729	285	444	361	162	199	1.79	0.53	0.31	0.22	0.34	0.13	0.21	0.17	0.08	0.09	24% of students are bused to school
Pajaro Valley High School	March 2008	1,525	2,188	667	388	279	405	152	253	168	62	106	1.43	0.43	0.25	0.18	0.27	0.10	0.17	0.11	0.04		Early closures on 3/6 and 3/10; PM activity reduces sharply after school; 20% of students are bused to school
School Survey Database Weighted Average Trip Rate			3,172	1,054	634	420	651	263	388	305	134	172	1.85	0.55	0.33	0.22	0.34	0.14	0.20	0.18	0.08	0.10	
Institute of Transportation Engineers Trip Rate	1	-	-	-	-	-	-	-	-	-	-	-	1.71	0.41	0.28	0.13	0.28	0.09	0.19	0.14	0.07	0.07	
Inbound/Outbound Split	School Survey Database										60%	40%		40%	60%		44%	56%					
						Institute of Transportation Engineers						69%		31%	32% 68%		68%	47% 53%					
Comparison of Survey Trip Generation Rate Versus ITE Rate										8%	33%			22%			28%						

Notes:

- 1. Student enrollments/population were at time of traffic count at each school.
- 2. PM School Peak Hour is the peak one-hour traffic period of school between 1:00 4:00 PM.
- 3. PM Street Peak Hour is the peak one-hour traffic period of school between 4:00 PM and 6:00 PM, which would coincide with peak hour of adjacent street system.
- 4. Sources of Data:

Gilroy High School: Christopher High School Traffic Analysis, Fehr & Peers Transportation Consultants, May 30, 2006.

Aptos High School: Aptos High School Access and Circulation Evaluation, Higgins Associates, May 7, 2004.

Seaside High School: Hatch Mott MacDonald (formerly Higgins Associates) traffic counts, March 2008.

Everett Alvarez High School: Hatch Mott MacDonald (formerly Higgins Associates) traffic counts, March 2008.

Pajaro Valley High School: Hatch Mott MacDonald (formerly Higgins Associates) traffic counts, March 2008.

Institute of Transportation Engineers: *Trip Generation*, 8th Edition, Institute of Transportation Engineers, 2008.

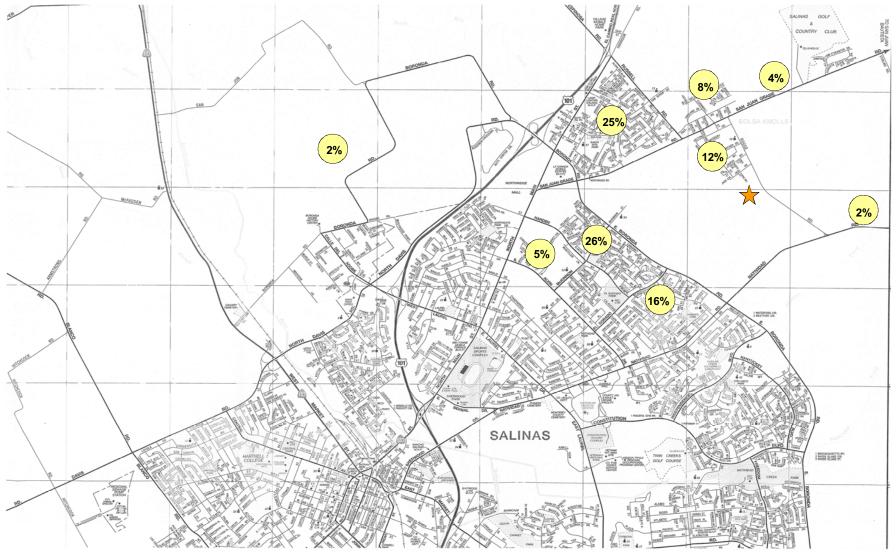
EXHIBIT 6A TRIP GENERATION RATE COMPARISON (AREA HIGH SCHOOLS VS. ITE)

PROJECT TRIP GENERATION

			AM	I PEAK	HOUF	₹	PM SC	HOOL	PEAK	HOUR	PM ST	HOUR		
	PROJECT SIZE	WEEKDAY DAILY TRIPS	TOTAL PEAK HOUR	% OF ADT	IN	OUT	TOTAL PEAK HOUR	% OF ADT	IN	OUT	TOTAL PEAK HOUR	OF	IN	OUT
TRIP GENERATION RATES ¹														
High School (per student)	1,500 students	1.85	0.55	30%	60%	/ 40%	0.34	18%	40%	/ 60%	0.18	10%	44%	/ 56%
Rogge Road High School														
High School	1,500 students	2,768	825	30%	495	/ 330	510	18%	204	/ 306	270	10%	119	/ 151
Trip Generation By Area														
Santa Rita District (Project Phase 1)	900 students	1,661	495	30%	297	/ 198	306	18%	122	/ 184	162	10%	71	/ 91
Future Growth Area (Remainder of Project Buildout)	600 students	1,107	330	30%	198	/ 132	204	18%	82	/ 122	108	10%	48	/ 60
TOTAL	1,500 students	2,768	825	30%	495	/ 330	510	18%	204	/ 306	270	10%	119	/ 151

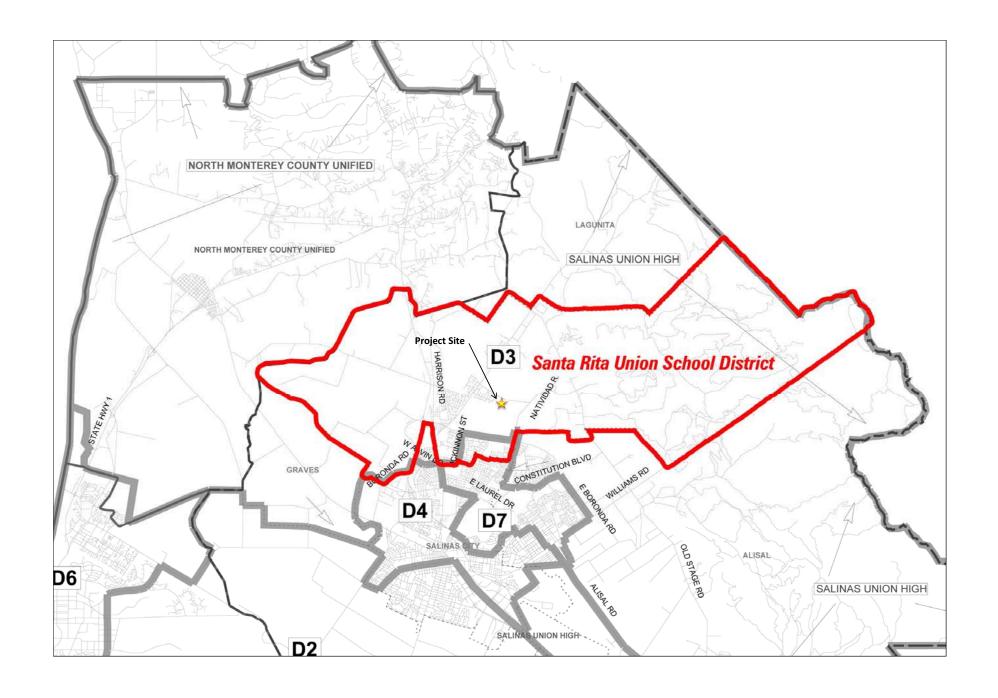
Notes:

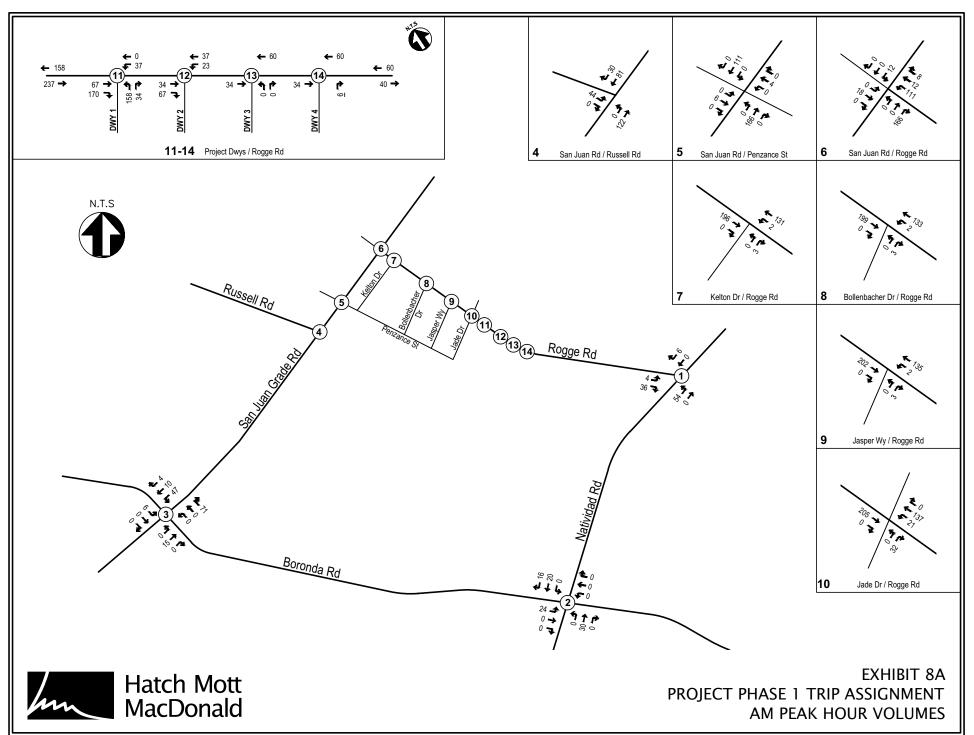
1. Trip generation rates for High School derived from traffic counts performed at five area high schools. See Exhibit 6A for more information.

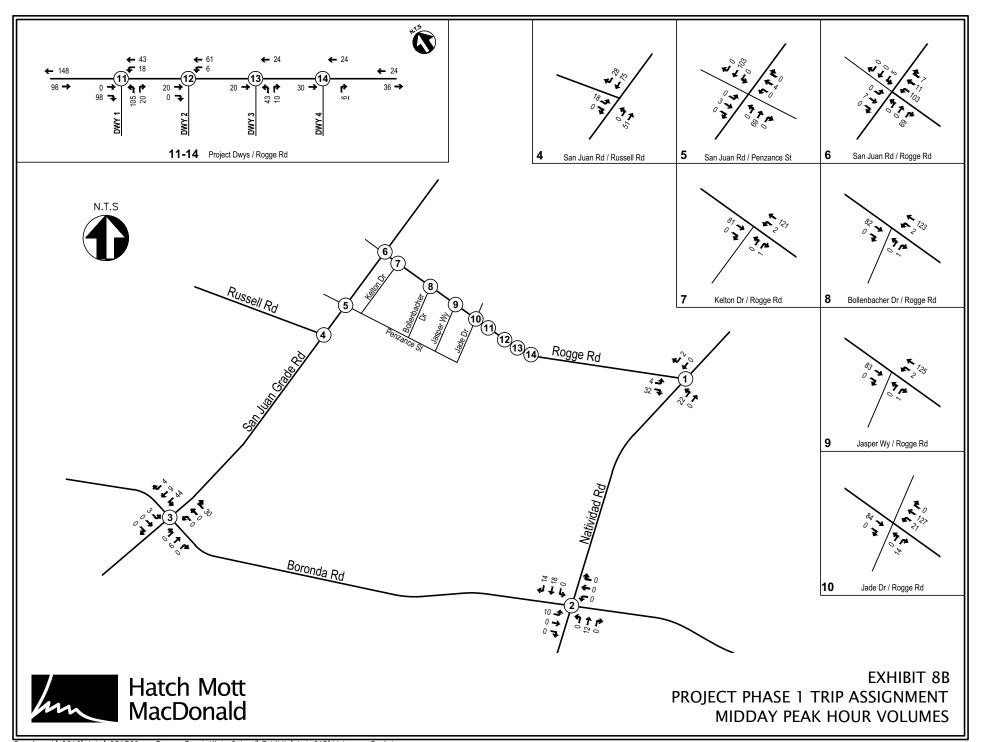


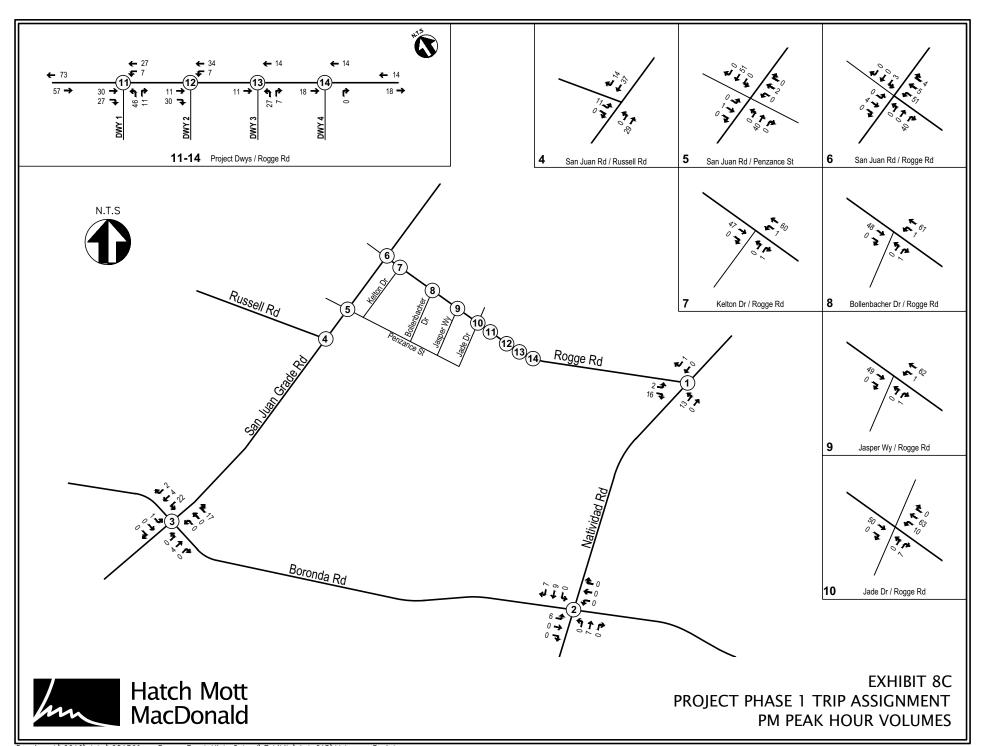
Santa Rita School District enrollment = 60% of the total #5 High School attendance.

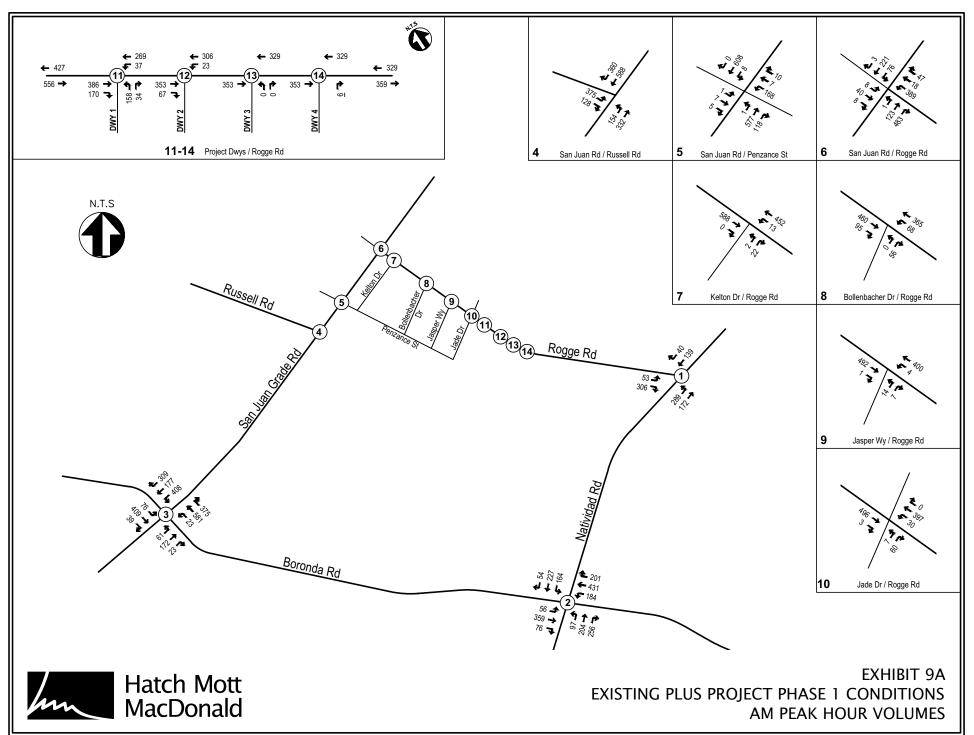
EXHIBIT 7A
PROJECT
TRIP DISTRIBUTION
(SANTA RITA SCHOOL
DISTRICT TRIPS - PROJECT PHASE 1)

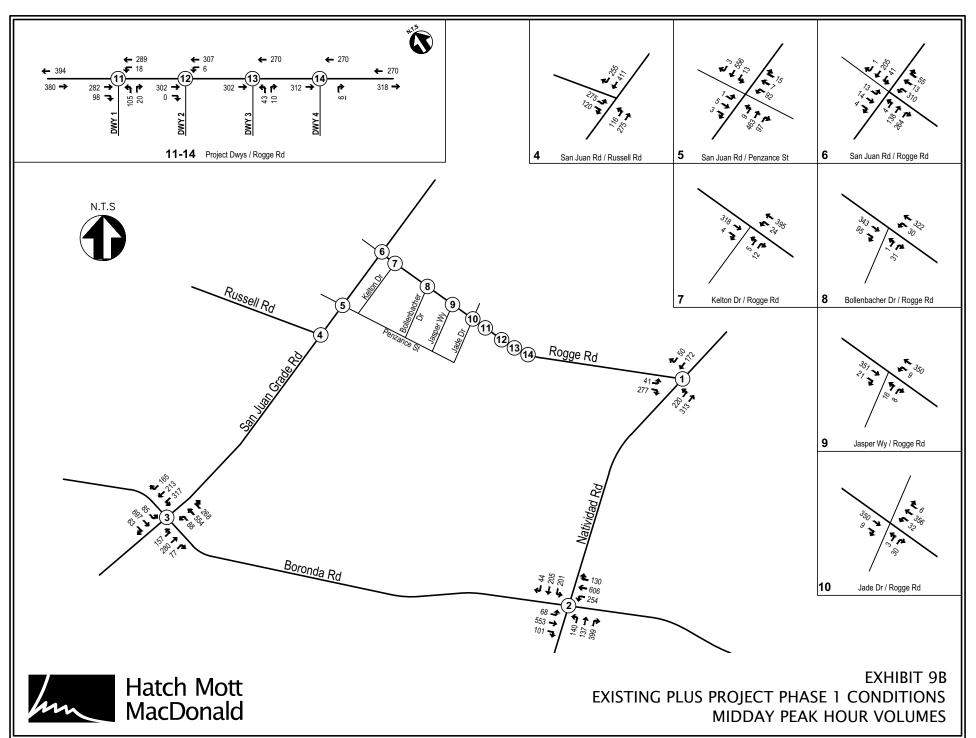


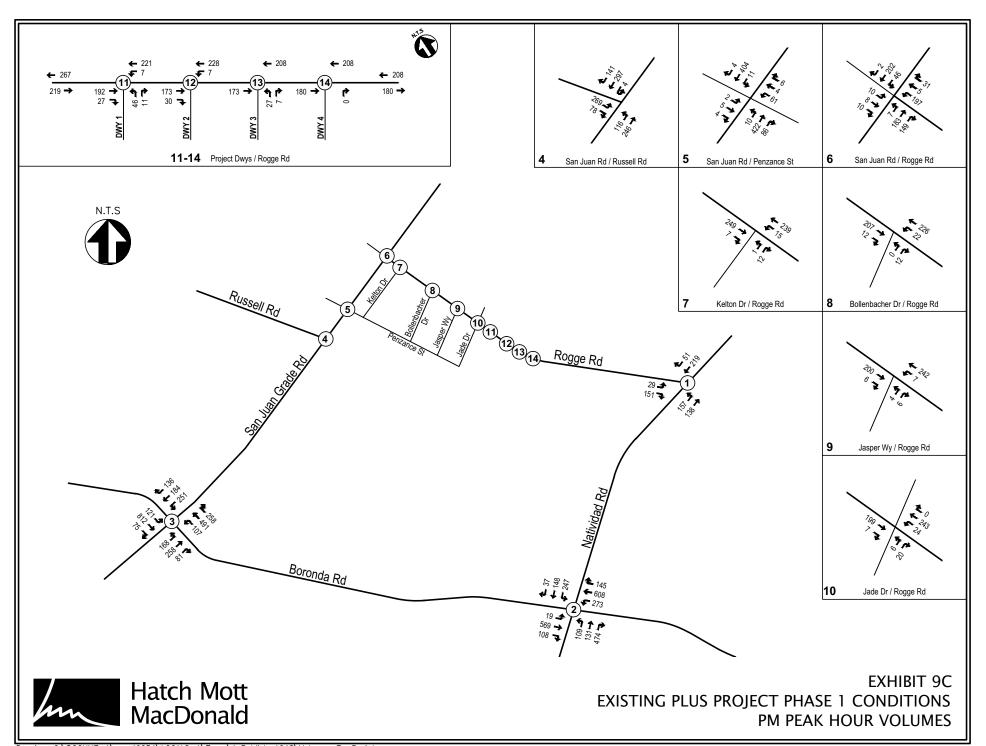












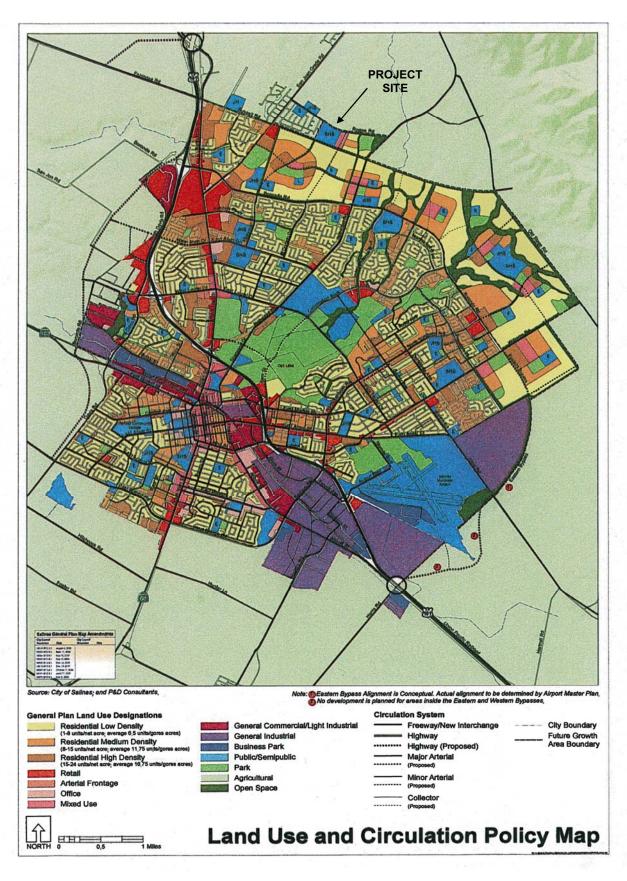
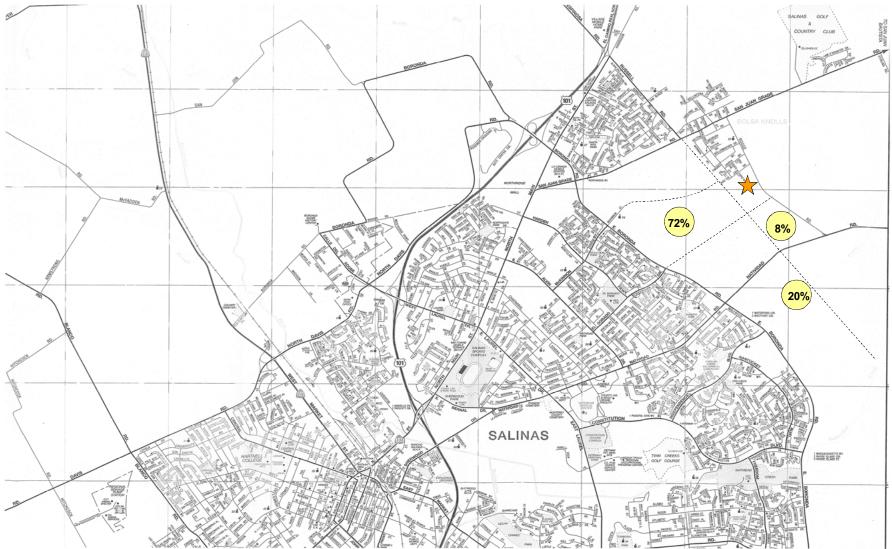


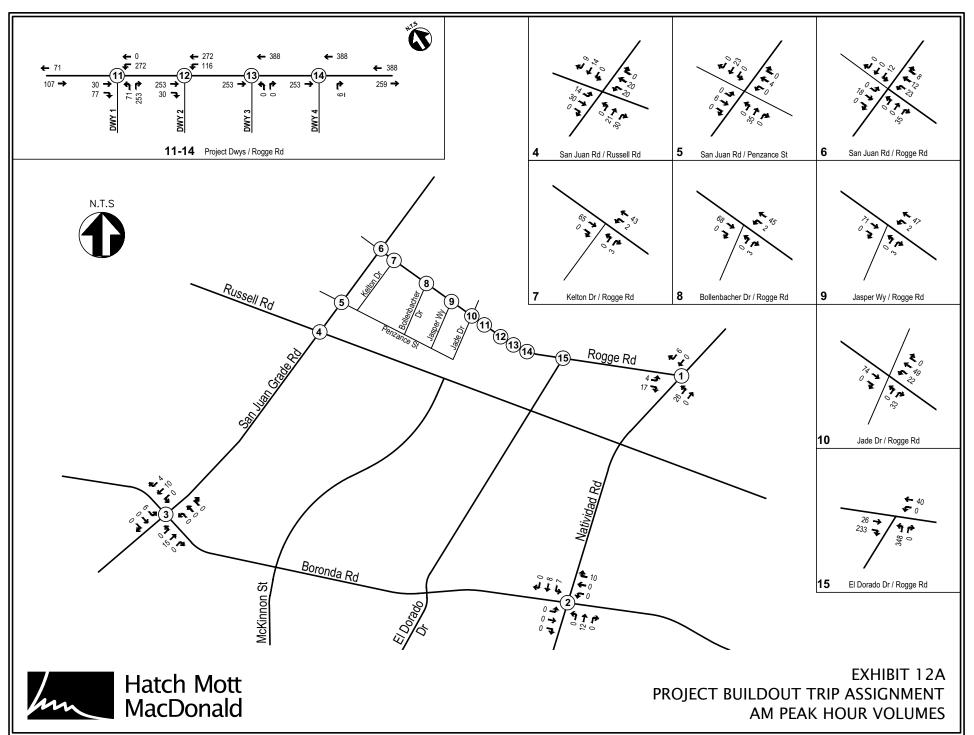
EXHIBIT 10 CITY OF SALINAS GENERAL PLAN ROAD NETWORK

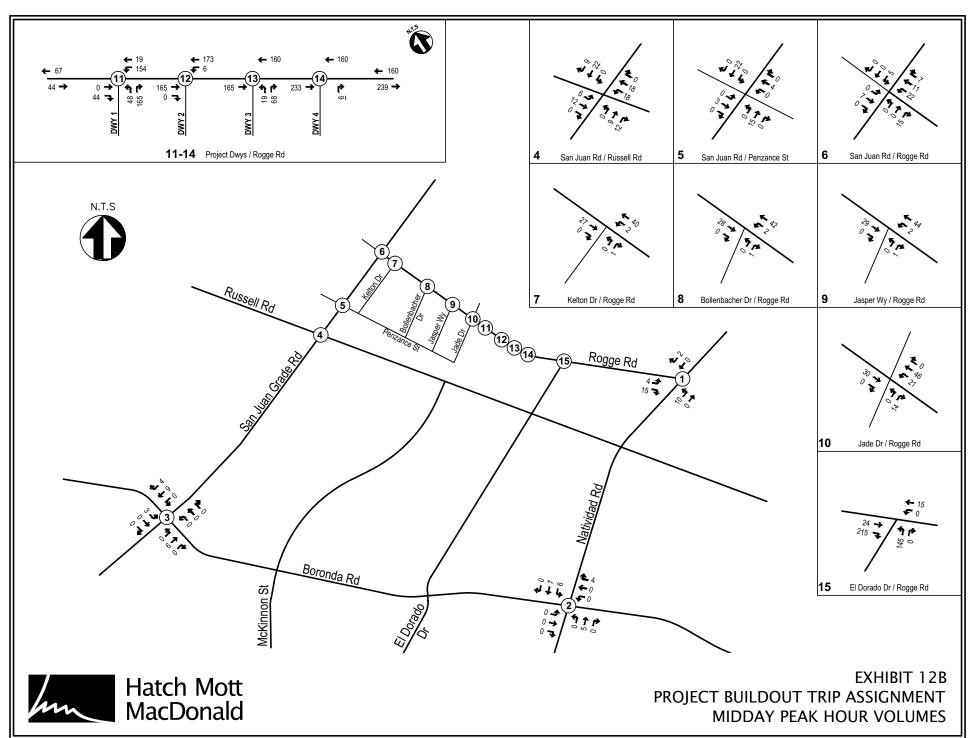


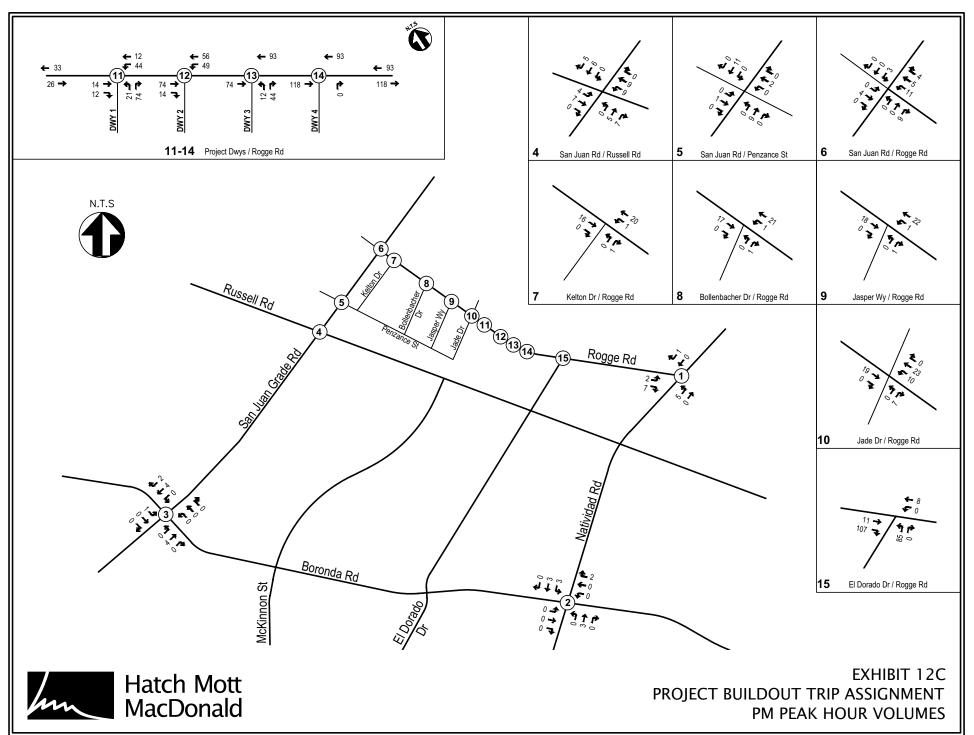
Future Growth Area enrollment = 40% of the total #5 High School attendance.

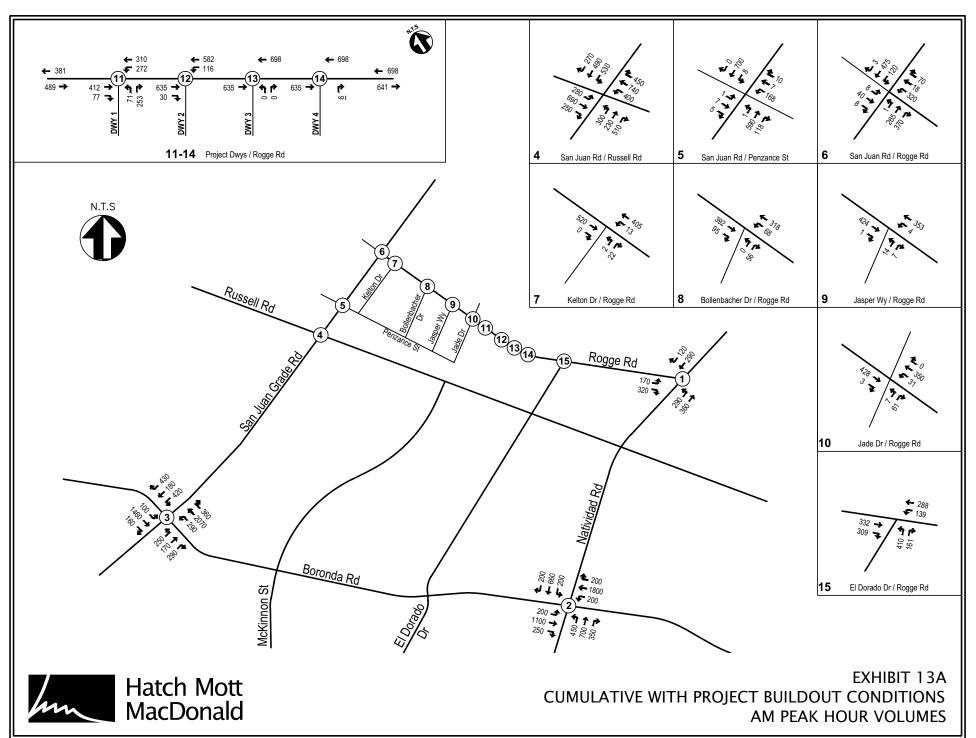
EXHIBIT 11
PROJECT TRIP DISTRIBUTION
(FUTURE GROWTH AREA REMAINDER OF PROJECT BUILDOUT)

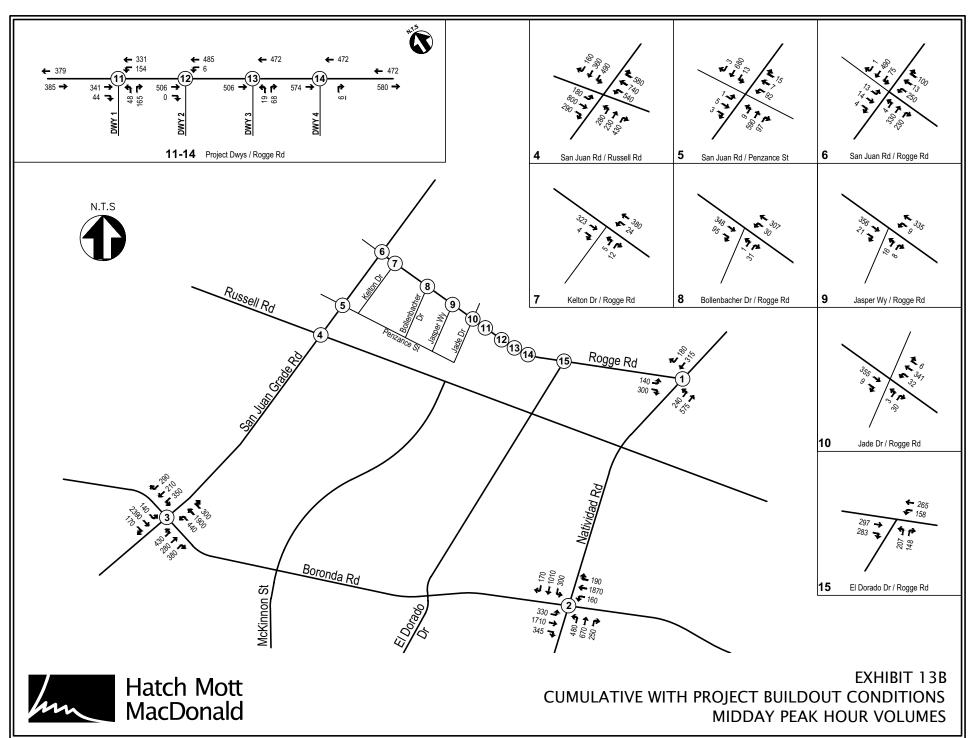
Hatch Mott MacDonald TripDistribution Ex 7.x/s

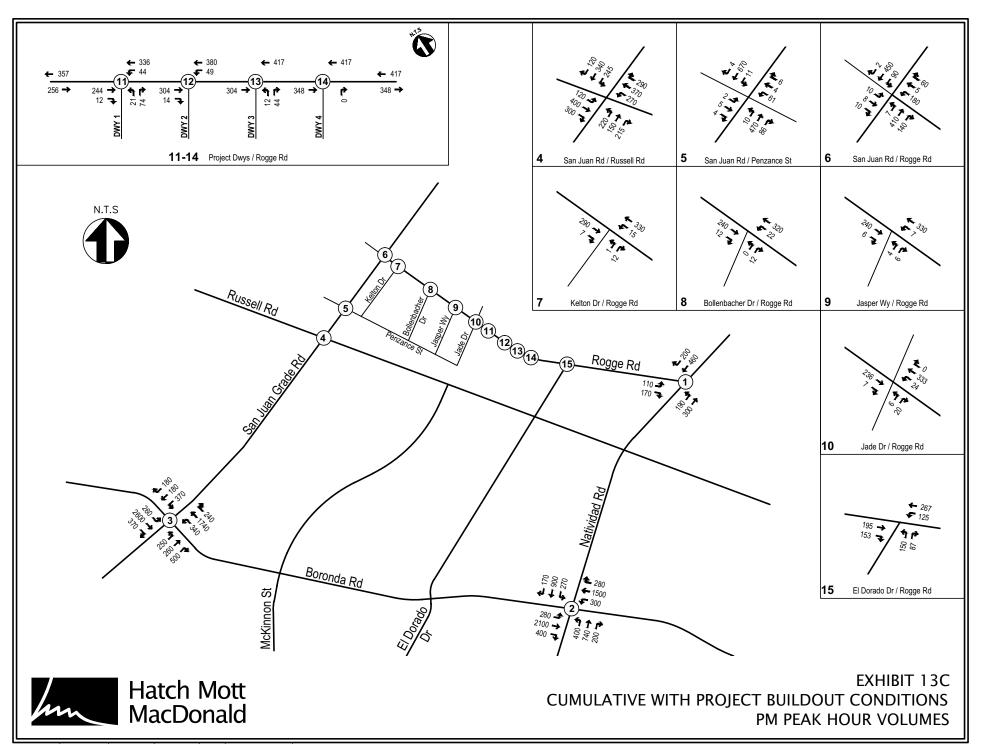


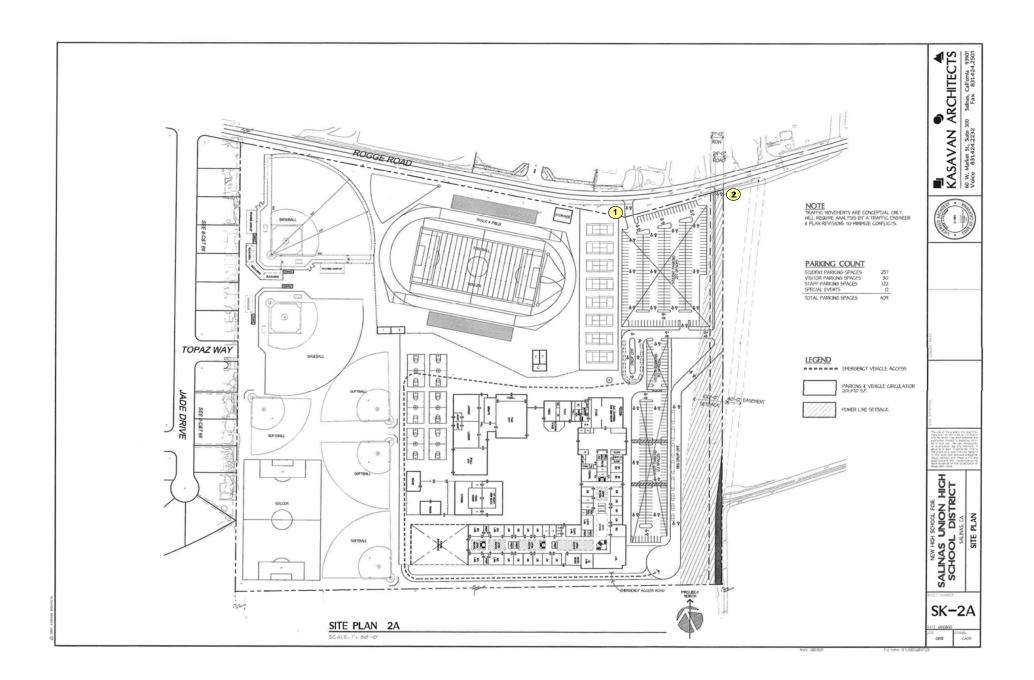


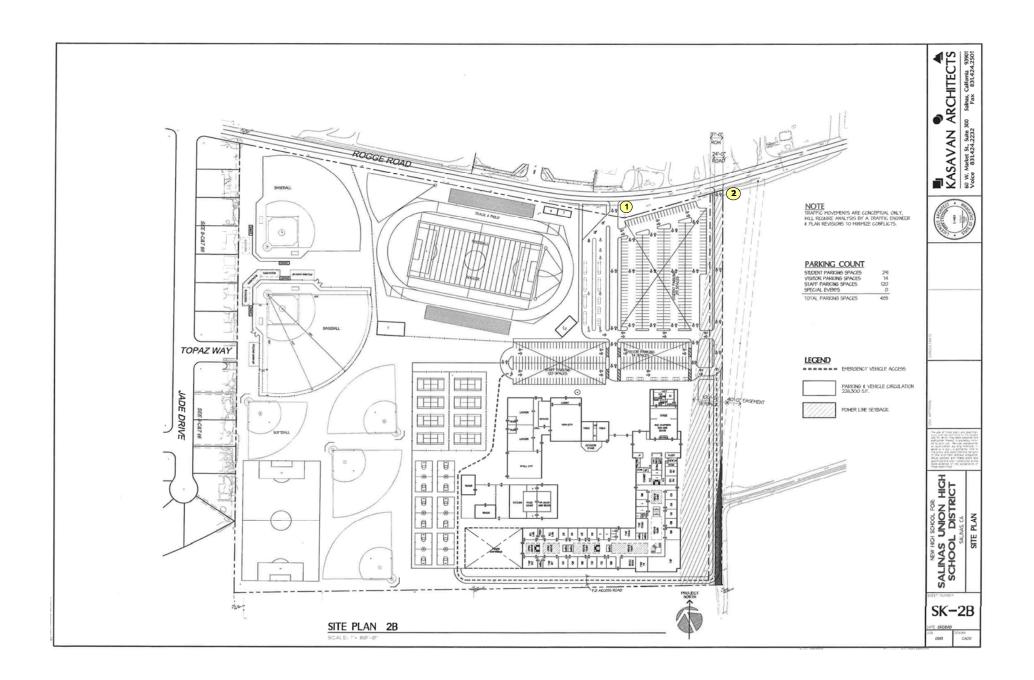






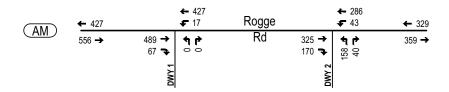


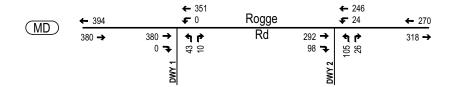


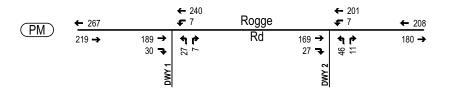


EXISTING PLUS PROJECT PHASE 1

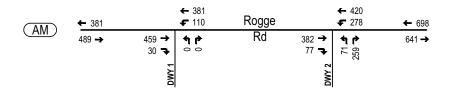


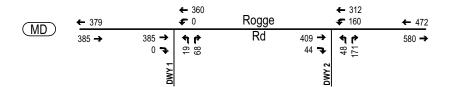






GENERAL PLAN BUILDOUT PLUS PROJECT BUILDOUT





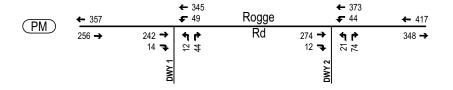
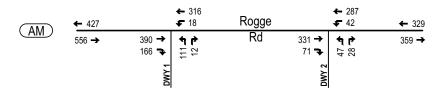


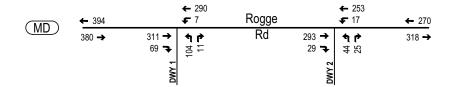


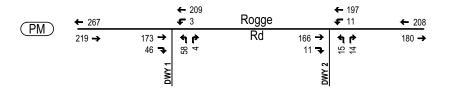
EXHIBIT 15A HIGH SCHOOL #5 ROGGE ROAD DRIVEWAY VOLUMES PROJECT ALTERNATIVE 1

EXISTING PLUS PROJECT PHASE 1

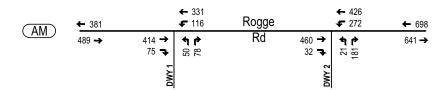


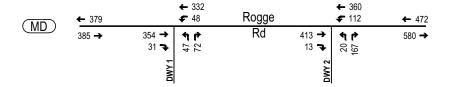






GENERAL PLAN BUILDOUT PLUS PROJECT BUILDOUT





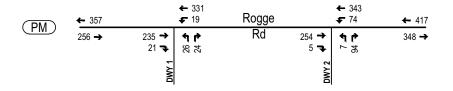




EXHIBIT 15B HIGH SCHOOL #5 ROGGE ROAD DRIVEWAY VOLUMES PROJECT ALTERNATIVE 2

						Fxis	ting + Pro	iect Pha	ise 1	Cumulative + Project Buildout							
			Existing				Condi			Conditions							
N-S E	-W	Lane Configuration	Intersection Control	LOS Standard	AM Peak Hr		MD Pk Hr		Sat Mid Pk Hr		AM Pe	eak Hr	MD F	Pk Hr	Sat Mid	d Pk Hr	
Street S	Street				Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	Delay (sec)	LOS	
	1 Rogge Road	NB 1-L/R EB 1-T/R WB 1-L, 1-T	Two-Way Stop (Worst Approach)	D (E)	0.2 9.8	A A	1.1 16.1	A C	0.9 11.7	A B	1.3 11.5	A B	1.9 18.6	A C	1.6 13.3	A B	
	Rogge Road	EB 1-T/R WB 1-L, 1-T	Two-Way Stop (Worst Approach)	D (E)	110.2 566.8	F F	7.4 43.3	A E	1.8 13.4	A B	*	F F	53.8 273.3	F F	2.3 15.4	A C	
			W/ TWLTL Two-Way Stop (Worst Approach)	D (E)	28.3 143.9	D F					308.0 1375.0	F F					
			W/ Siginalization W/ Siginalization & NB free right turn	D D	24.8 -	C -					- 31.5	- C	- 15.9	- В			
PROJECT ALTERNATIVE	2																
	Rogge Road	EB 1-T/R WB 1-L, 1-T	Two-Way Stop (Worst Approach)	D (E)	4.6 36.5	A E	2.9 19.7	A C	1.6 12.3	A B	16.4 125.5	C F	3.4 21.6	A C	1.4 14.9	A B	
			W/ TWLTL Two-Way Stop (Worst Approach)	D (E)	-	:	-	-	- -	-	4.7 28.4	A D					
	Rogge Road	NB 1-L/R EB 1-T/R WB 1-L, 1-T	Two-Way Stop (Worst Approach)	D (E)	3.5 32.6	A D	2.1 18.4	A C	1.0 11.3	A B	211.9	F F	9.4 48.8	A E	2.5 12.7	A B	
			W/ TWLTL Two-Way Stop (Worst Approach)	D (E)							63.5 412.0	F F					
			W/ Siginalization	D							27.4	С					

NOTES

- 1. L, T, R = Left, Through, Right.
- 2. NB, SB, EB, WB = Northbound, Southbound, Eastbound, Westbound.
- 3. Analysis performed using 2000 Highway Capacity Manual methodologies.
- 4. Worst approach level of service standard is generally LOS E. Level of service "F" is the level of service at which improvements would be required.
- 5. Levels of service cited under *Mitigations* use recommended improvements shown on *Exhibit 5B*.
- 6. * = Delay is over 300 seconds (5 minutes)
- 7. Operations in **bold** represent significant impacts.
- 8. Rogge Rd TWLTL: Widen Rogge Road to provide a two-way left turn lane (TWLTL) in the median.

EXHIBIT 16
INTERSECTION
LEVELS OF SERVICE
ROGGE ROAD/PROJECT DRIVEWAY
INTERSECTIONS
SITE PLAN ALTERNATIVES 1 AND 2

